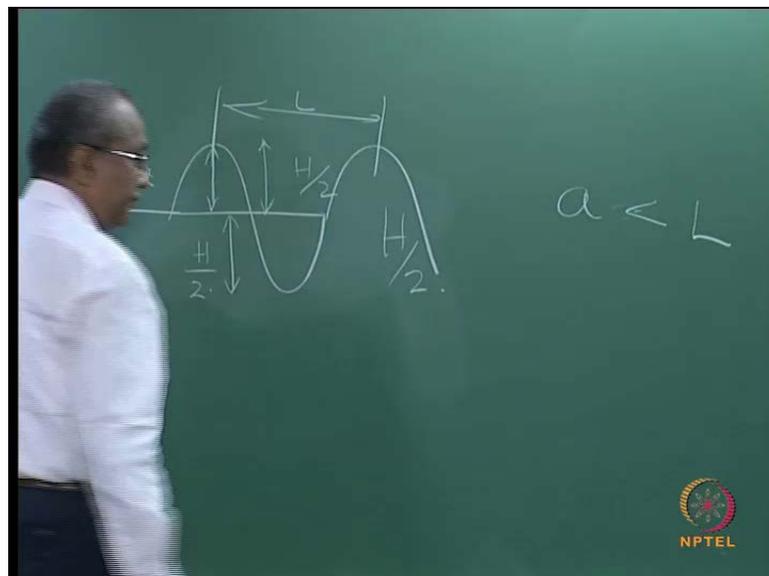


**Wave Hydro Dynamics**  
**Prof. V. Sundar**  
**Department of Ocean Engineering**  
**Indian Institute of Technology, Madras**

**Module No. # 06**  
**Wave Theories and Testing Facilities**  
**Lecture No. # 01**  
**Finite Amplitude Wave Theories**

Having seen, the aspects of small amplitude theory, where in the basic assumption was that the amplitude of the wave is much less compared to the wavelength, which can be most cases for the waves, which are in deeper waters. But when the waves travel propagate in the coastal waters, then you will not you cannot really, have linear waves. That is, when I say linear waves crest and the trough portion should be equal. That is, we define amplitude as equal to wave height by 2.

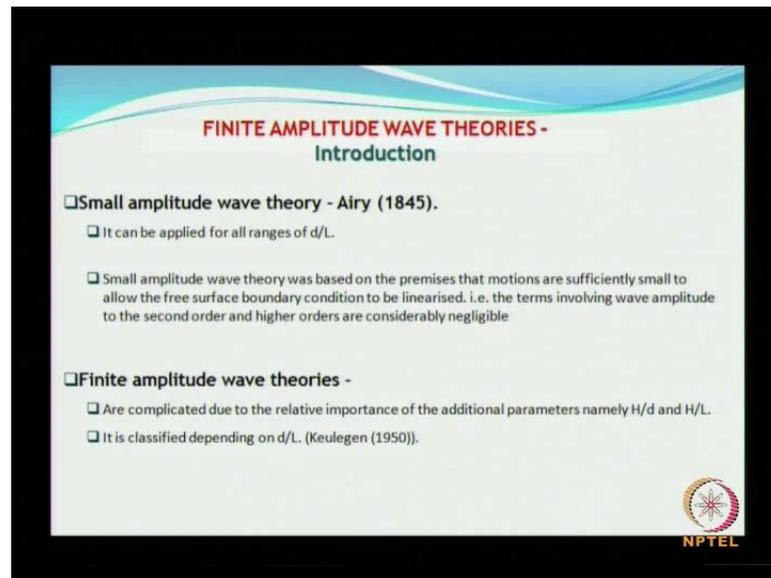
(Refer Slide Time: 00:56)



So, in the small amplitude wave theory, we say that  $H$  is equal to  $2a$ . So, this is your  $H$  by 2. So, this portion about water level which is either high tide line or a mean sea level or a low tide line. This will be the about which the wave surface is oscillating; so you see that this is  $H$  by 2 and this is  $H$  by 2, such a wave is called as a linear wave. When you want

to have the linear wave, the assumption is that, what we saw that amplitude? This is amplitude is much less than wavelength. That is, this is your wavelength, but this may not be the case. As, we have already seen a number of other kinds of waves, or when I gave some of the other examples like Tsunami etcetera.

(Refer Slide Time: 02:15)



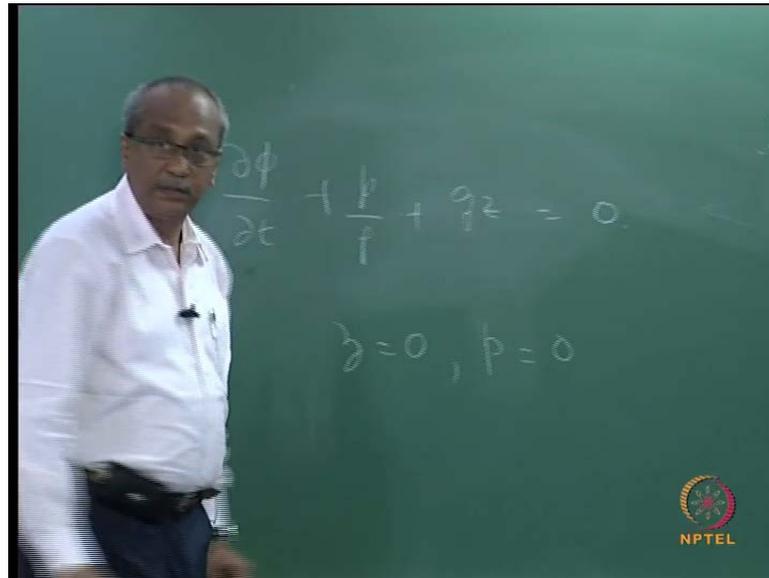
**FINITE AMPLITUDE WAVE THEORIES - Introduction**

- **Small amplitude wave theory - Airy (1845).**
  - It can be applied for all ranges of  $d/L$ .
  - Small amplitude wave theory was based on the premises that motions are sufficiently small to allow the free surface boundary condition to be linearised. i.e. the terms involving wave amplitude to the second order and higher orders are considerably negligible
- **Finite amplitude wave theories -**
  - Are complicated due to the relative importance of the additional parameters namely  $H/d$  and  $H/L$ .
  - It is classified depending on  $d/L$ . (Keulegan (1950)).

  
NPTEL

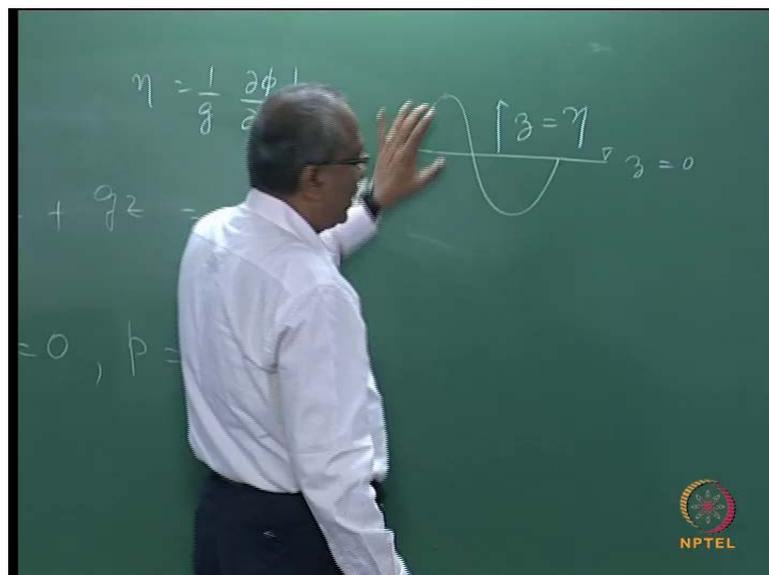
The waves can not be linear and hence we resort to, what is called as the Finite Amplitude Wave Theories. Small amplitude wave theory can be applied for all ranges of  $d$  by  $L$ , can be applied. There is no hard and fast rule. Small amplitude wave theory was based on the premises that the motions are sufficiently small, to allow the brief free surface boundary condition to be linearised. Please recollect the derivation of velocity potential.

(Refer Slide Time: 02:58)



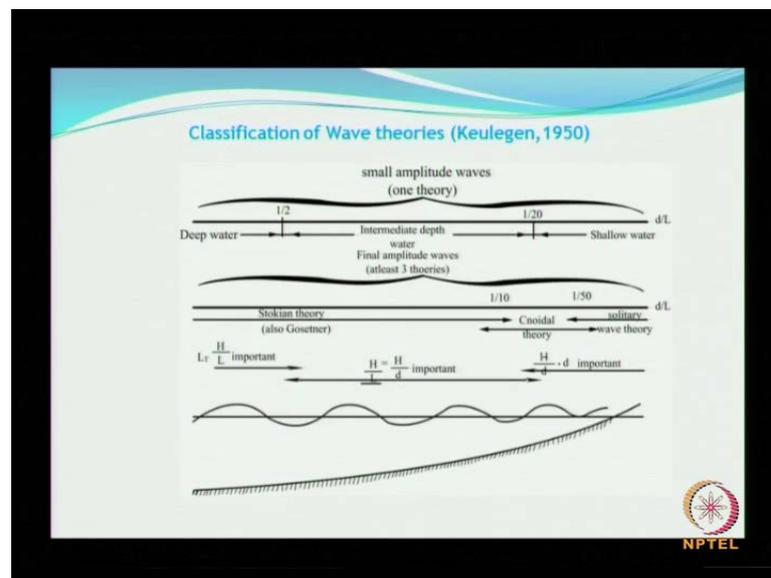
What did we do? We linearised the Bernoulli Equation,  $\frac{\partial \phi}{\partial t} + \frac{p}{\rho} + gz = 0$  by  $\eta$  and, what we did? We use this Bernoulli Equation at a free surface. When we have the free surface, this is the free surface  $z = 0$ , which is free surface, free surface is, this is the free surface. But instead of using  $z = \eta$ , we had adopted the simplicity in order to simplify the problem. We adopted this for the dynamic free surface boundary condition. That is at  $z = 0$ ,  $p = 0$  and this is called as the dynamic free surface boundary condition.

(Refer Slide Time: 04:04)



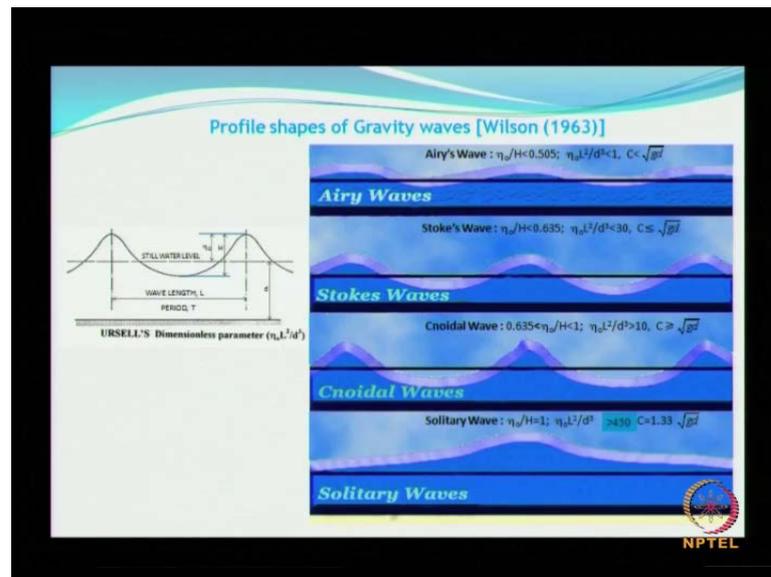
So, from which, remember we get  $\frac{1}{g} \frac{d^2 \phi}{dt^2}$  at  $z = 0$ . For the simple reason, we stated that the amplitude is small compare to the  $(\epsilon)$  that was the basic assumption, which was made. So, that is, what is illustrated here, as so, what we have done here? Is conveniently, we have avoided the higher order terms. Strictly speaking, those higher order terms have to be considered, and thus we have, what is called as the Finite Amplitude Wave Theories? A quite complicated due to the relative importance of additional parameters namely  $\frac{H}{d}$  and  $\frac{d}{L}$ .

(Refer Slide Time: 05:08)



So, based on the variation in  $\frac{d}{L}$  Keulegan Carpenter, it is Keulegan in 1950, he classified the waves for the wave theories, as per the  $\frac{d}{L}$ . That is,  $d$  is the water depth and  $L$  is the wavelength so, for the entire water depth situation, that is  $\frac{d}{L}$  is equal covering the entire range. You see, there is a single theory, which is the small amplitude Airys Theory. But, for other cases, so as indicated here, you have  $\frac{d}{L}$  and beyond this up to this, you have, what is called as your Stokian Theory and within this range, we have, what is called as the Cnoidal Theory. And then  $\frac{d}{L}$  up to  $1/50$ , that is less than  $1/50$ , we have solitary wave. So, here  $\frac{H}{L}$  is important,  $L$  is also important so,  $\frac{H}{d}$  and  $\frac{H}{L}$  is important. And all these things are quite important, they plays important role in defining the wave characteristics.

(Refer Slide Time: 06:37)



So, Airys Theory  $\eta_0$  by  $H$  is less than 0.5 and here, you see that what is called as the Ursells parameter. Ursells parameter is this is  $\eta_0$  and this is the wave length and  $d$  is small  $d$  so, this is the Ursells parameter, Ursells parameter is indicated here. What should be the Ursells parameter for the different types of waves that are being considered? For example, Stokes Wave Theory. So, here the celerity is less than root of  $g d$ , less than or equal to root of  $g d$ . So, here it is greater than root of  $g d$  that is in cnoidal waves and, then in the case of solitary waves, the speed of the waves is equal to 1.33 into root of  $g d$ . As we have already seen, the Shallow water conditions, in Shallow water the celerity or the speed of the wave is root of  $g d$ .

So, the shape of the waves are given here. So, you see this is more or less sinusoidal or it has to be sinusoidal, then we call it as Airys Theory. If the crest is slightly steeper and the trough is slightly flatter, then it enters into the Stokian Wave Theories. Under Stokes Wave Theories, you have several orders that involves. That is, because you have the number of orders, higher order terms you include based on that, you have the order theory. So, you have the 2 order, 3 order, 4<sup>th</sup> order, 5<sup>th</sup> order Stokes Theory etcetera. Then, we have the Cnoidal waves, where in the profile is defined by  $\cos^2$  cosine elliptical function, which we will see later. Finally, we have the Solitary wave, so in the case of  $\text{sech}^2$  cosin, Cnoidal Waves, you see the steepening of the waves.

The trough is quite flat, and long. And these are basically called as long waves, Cnoidal and Solitary Waves. The importance of Cnoidal and Solitary Wave Theories, as kind rapid importance people are working a lot, in these two waves. Because, the tsunami characteristics can be, to some extent. You cannot call it directly, as a Solitary wave to a certain extent. The characteristics of a tsunami, can be represented to the characteristics of Solitary waves in the case of Solitary waves, you see that, there is no trough. Now, the waves are going to hit like this, at regular intervals. So, it is something like, your wall of water, as we usually call it, in terms of as we usually tsunami is termed as wall of water. So, again I would like to reiterate that Cnoidal and Stokes Solitary Waves are basically, Shallow water waves.

(Refer Slide Time: 10:28)

**STOKE - WAVE THEORIES**

- The small amplitude airy theory was developed for waves of somewhat greater amplitude to be called as Stokes higher order theory developed by Stokes (1847).
- The form was expressed as a series of terms of  $\text{Cos} \left[ 2\pi n \left( \frac{x}{L} - \frac{t}{T} \right) \right]$  Where  $n = 0, 1, 2, \dots$
- Such waves are irrotational.
- In deep waters, if terms involving  $(H/L)^3$  and higher powers are neglected form the Fourier series relating to such an analysis.

$$C_o = \left\{ \frac{gL_o}{2\pi} \left[ 1 + \frac{\pi^2 H_o^2}{L_o^2} \right] \right\}^{1/2} \quad (1)$$

- As the ratio  $H/L$  increases, the crest becomes sharper and the trough flatter than Airy wave. This has the effect of raising the median height relative to the SWL.
- The regions of validity for the various wave theories are given in Fig.2.

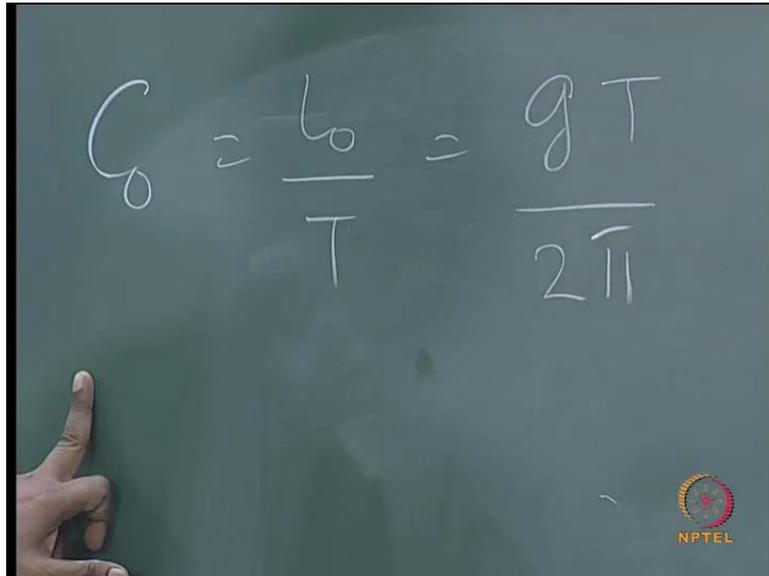
  
NPTEL

So, let us see Stokes Wave Theories again. I am just giving only the highlight of some of these theories, but you need to go through books, or the references indicated in the lecture. Small amplitude Airys Theory was developed for waves, a somewhat greater amplitude to be called as Stokes higher order theory, developed by Stokes in 1847, quite old theory. The form was expressed as a series of term of cos, cos of, so, you have a n standing, where n equal to 0 1, 0 1 2 etcetera. So, it can go to higher numbers of course, the waves are, such waves are considered to be irrotational.

In deep waters, if the terms involving  $H$  by  $L$  cube and higher powers are neglected then, you can have a solution, as given here. Under equation 1, as  $H$  by  $L$ , the rate of  $H$  by  $L$

increases, the crest become sharper and sharper. Whereas, the trough becomes flatter, and flattered than that of a Airys Theory. So, airys theory, you know, how to calculate the C naught, how do you calculate the C naught in Airys Theory,  $g L$  naught by  $2 \pi$ . Whereas, here you have that additional term coming into picture, is that clear?

(Refer Slide Time: 12:39)

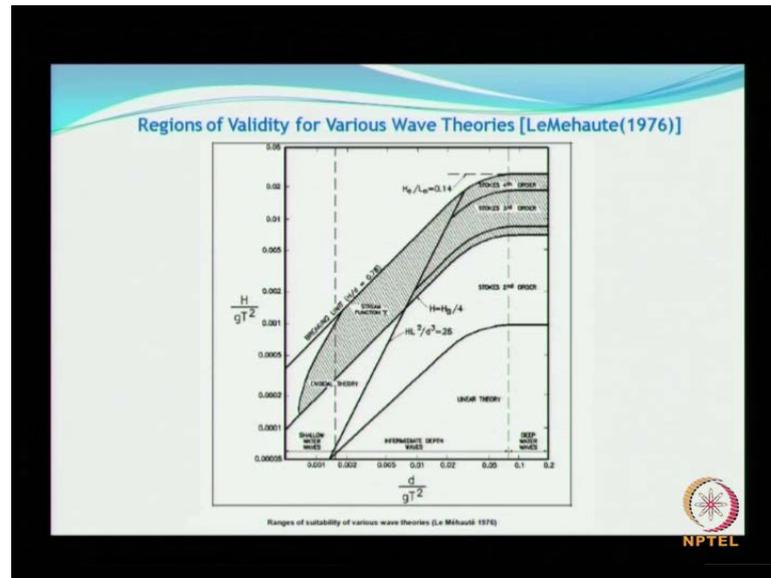

$$C_0 = \frac{L_0}{T} = \frac{gT}{2\pi}$$

In Airys Theory, C naught equal to L naught by T, this is

(No Audio from 12:43 to 13:03)

so, the same thing here ,it is represented, you have an higher order terms coming into picture.

(Refer Slide Time: 13:11)



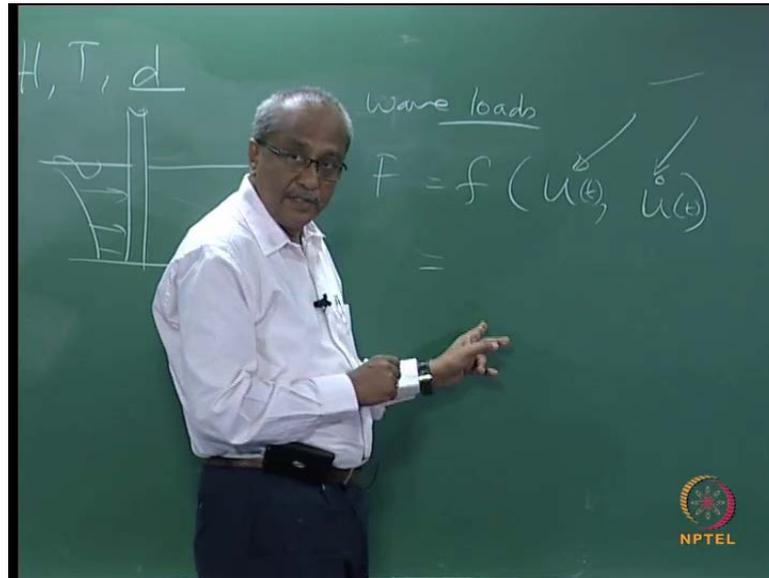
So, this shows, the regions of validity of various theories, and this is after lemehaute in the year 1976. The ranges of the validity of different theories, you see this is the area, where you can adopt linear theory without any problem. This is the region, where you can adopt second order theory. And this is the region, where you can adopt the third order theory, and this is the region, where you can adopt 4<sup>th</sup> order Stokes Theory. This is the region, where you can adopt your Stream Function Theory and Cnoidal Wave Theory comes in here, and this is the breaking limit.

(No Audio from 14:09 to 14:17)

So, this is the area, where you have the Shallow water waves. Shallow water waves and this is the area, where you have the Intermediate water waves. And this is the area, where you have the deep water (O). I am sure, you are able to recollect the characteristics of shallow water deep water and intermediate waters, based on the displacement. Under the water particle displacement, under different water depth condition, we have seen enough.

So, on the y axis, you have achieved H by g d square, and on the X axis, you have d by g d square. So, this shows that, the relationship between wave height, wave period, as well as the water depth are the main factors. That dictate, which theory has to be applied for, and why are, we talking about this theory? Where in, you need to consider all these theories.

(Refer Slide Time: 15:41)



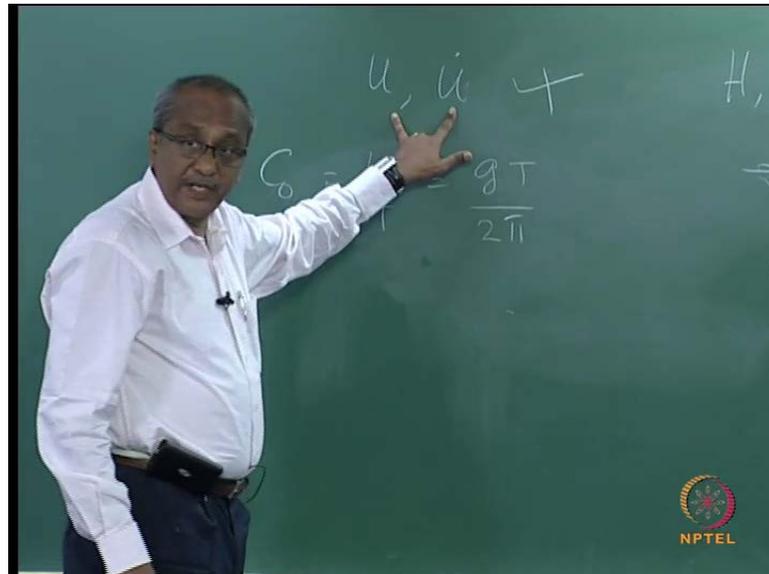
So, you know that, all structures, or pile structure, this is a small example. All structures are supported on pile, and you need to evaluate the force exerted on this structure. Later on, my lecture on wave forces, or wave loads on structures, you will see that the force is going to be a function of orbital velocity and acceleration. So, this is going to be function of  $t$  and also this  $u$  and  $\dot{u}$ , that is particle velocities and accelerations are going to vary with respect to along the depth. (No Audio from 16:29 to 16:35) So, you need a theory to describe the velocity profile for a given wave height and wave period, for the pile in a particular water depth at  $t$ .

So, straight away, if nothing is given, you will be proceeding to calculate these two using linear theory, because it is quite simple and straight forward without, even bothering to check, whether it is applicable. Although, it is the first step, by chance if your parameter that is these two parameters fall in this range, then there is absolutely no problem in using or in proceeding with your linear theory for calculation of the particle kinematics without calculating anything. You have proceeded with Airys Theory, and then finally, if one looks into the variation, and your data falls somewhere in this region, or somewhere, in this region.

Now, you are using a wrong theory for your data, data consisting of wave height period and depth. So, this is a basic step, which you need to do, because you are using a Linear Theory in a zone, where the waves are no longer linear and in fact, it is lying in the

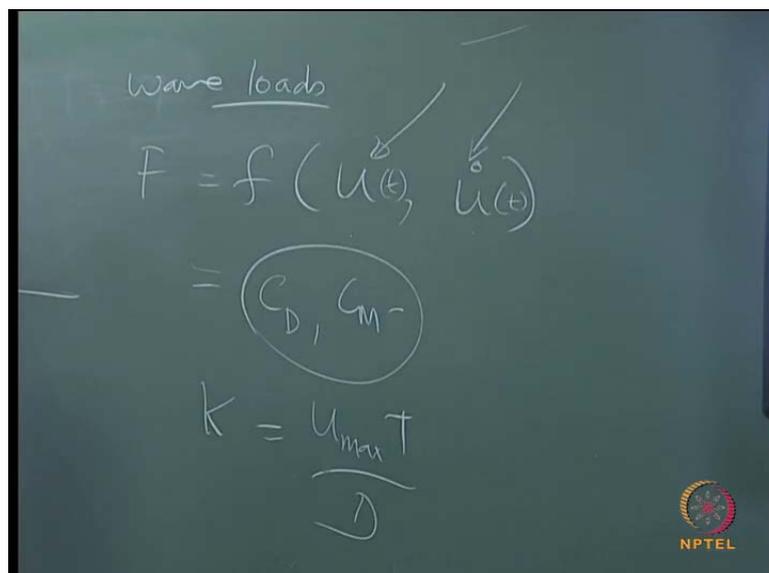
shallow water condition. Shallow water situation ,or may be in the deep water situation somewhere here, the particle Kinematics evaluation of the particle kinematics is incorrect.

(Refer Slide Time: 18:47)



So, you will see that  $u$  and  $\dot{u}$  evaluation is not wrong. it is, you will still have some values, but it is not 100 percent correct. Now, this is only in the estimation of particle velocity and acceleration. Now, the force is going to be a function of velocity and acceleration so, certain degree of the error involved, now multiplies.

(Refer Slide Time: 19:23)



So, for the evaluation of the force, you will also, further see that you need to look at the selection of hydrodynamic coefficients  $C_D$  and  $C_M$ . These are again based on a parameter called Keulegan Carpenter number, which is  $u_{max} T$  by  $D$ . So, here again you see the evaluation is based on this so, your error multiplies. So, the first step is, to make sure, whether your problem falls under Airys Theory. If not under which theory, it follows and only after getting that information, proceed with the calculation of particle kinematics and then with your forces, I hope this is clear.

(Refer Slide Time: 20:37)

>The expressions for wave elevation, orbit velocities, displacements, mass transport velocity and pressure based on Stokes's Second order Wave Theory.

>For first order

$$\eta = \frac{H}{2} \cos\left(\frac{2\pi}{L}x - \frac{2\pi}{T}t\right) = \frac{H}{2} \cos\theta = a \cos\theta$$

>A more general expression would be

$$\eta = a \cos(\theta) + a^2 B_2(L, d) \cos(2\theta) + a^3 B_3(L, d) \cos(3\theta) + \dots + a^n B_n(L, d) \cos(n\theta)$$

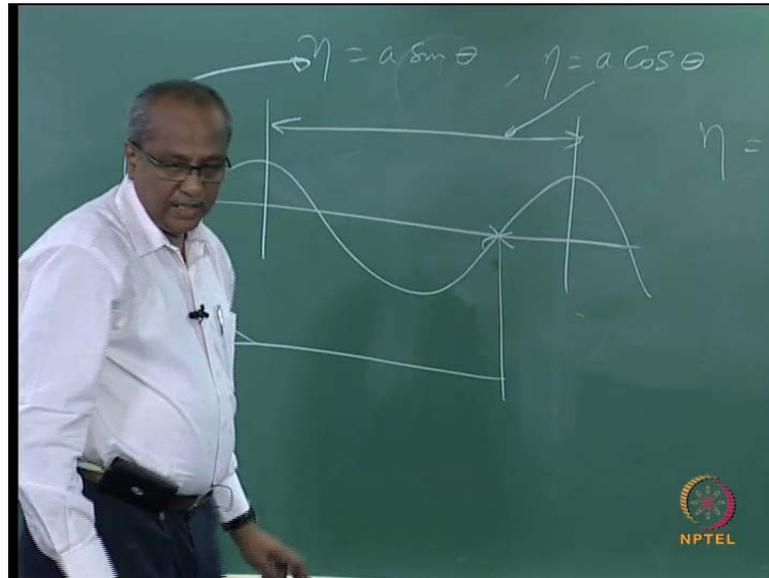
$a < \frac{H}{2}$  for orders higher than 2<sup>nd</sup>, and  $B^2, B^3$ , etc. are specified functions of  $L$  and  $d$

$$\eta = \frac{H}{2} \cos(kx - \sigma t) + \left(\frac{\pi H^2}{8L}\right) \frac{\cosh kd}{\sinh^3(kd)} [2 + \cosh 2kd] \cos 2\theta \quad (2)$$



So, expressions for wave elevation, orbital velocities, displacements, mass transport velocity and pressure all these things, based on Stokes second order theory is provided here.

(Refer Slide Time: 20:53)



Note that earlier lectures, I have used,  $a \sin \theta$  for purposefully. I am considering  $a \cos \theta$ , the main reason is, you should not get confused. Because certain books consider  $\eta$  as a  $\sin \theta$ , certain books consider does not matter at all, because the properties are going to be same. But only thing is, you should not keep on changing it. When I am discussing about the second order theory, I am not going to change anything so, for the second order theory, what I am trying to do is, I am considering only this portion of the wave.

So, this is going to be represented in this form, this is a  $\sin$  curve and this is represented by; is that clear? This is very important, because I see some people have, some kind of problem, some of the students have some problem. So, this is very clear, I do not see anykind of confusion here. A more general expression for representing  $\eta$  is given their, as  $s$  square into  $\beta b^2$  etcetera. It continues, as you can see here square cube to the power  $n$ , and also within the argument of cosine. You have the factor  $n$  so, that  $n$  takes care of the all of the wave theory, that is going to be considered.

So, the second order theory for the wave elevation is shown here, is given by this expression. You can simply derive, I am not going into the derivation part, for the derivation part. There are some books, which do give you the details, but the procedure is as out lined in this small amplitude theory only thing is, you need to considered the higher order terms.

(Refer Slide Time: 23:44)

For deep water  $\frac{d}{L} > \frac{1}{2}$  the above equation becomes

$$\eta = \frac{H_0}{2} \cos\left(\frac{2\pi x}{L_0} - \frac{2\pi t}{T}\right) + \frac{\pi H_0^2}{4L_0} \cos\left(\frac{4\pi x}{L_0} - \frac{4\pi t}{T}\right) \quad (3)$$

**Particle Kinematics**

$$u = \frac{HgT}{2L} \frac{\cosh[2\pi(z+d)/L]}{\cosh kd} \cos(kx - \sigma t) + \frac{3}{4} \left(\frac{\pi H}{L}\right)^2 C \frac{\cosh[2k(z+d)]}{\sinh^4 kd} \cos\left(\frac{4\pi x}{L} - \frac{4\pi t}{T}\right) \quad (4)$$

$$w = \frac{\pi H}{L} C \frac{\sinh[2\pi(z+d)/L]}{\sinh kd} \sin(kx - \sigma t) + \frac{3}{4} \left(\frac{\pi H}{L}\right)^2 C \frac{\sinh[2k(z+d)]}{\sinh^4 kd} \sin\left(\frac{4\pi x}{L} - \frac{4\pi t}{T}\right) \quad (5)$$


So, for deep waters the above expression will reduce as shown here. Similarly, for the second order theory, the particle velocity is given as shown here. Whereas, I mean for the horizontal direction and this is the vertical direction. So, use the appropriate values for the variable concerned, and then evaluate the particle velocities, which naturally will be different from, what you estimate through the linear wave theory.

(Refer Slide Time: 24:19)

**Horizontal water particle displacement**

$$\xi = \frac{-HgT^2}{4\pi L} \frac{\cosh k(d+z)}{\cosh kd} \sin(kx - \sigma t) + \frac{\pi H^2}{8L} \frac{1}{\sinh^2 kd} \left\{ 1 - \frac{3}{2} \frac{\cosh 2k(d+z)}{\sinh^2 kd} \right\} \sin 2(kx - \sigma t) + \left(\frac{\pi H}{L}\right)^2 \frac{Ct}{2} \frac{\cosh 2k(d+z)}{\sinh^4 kd} \quad (6)$$

**Vertical displacement is**

$$\zeta = \frac{HgT^2}{4\pi L} \frac{\sinh[k(d+z)]}{\cosh kd} \cos(kx - \sigma t) + \frac{3}{16} \frac{\pi H^2}{L} \frac{\sinh[2k(d+z)]}{\sinh^4 kd} \cos 2(kx - \sigma t) \quad (7)$$

**Mass transport velocity**

$$\bar{u}(z) = \left(\frac{\pi H}{L}\right)^2 \frac{C}{2} \frac{\cosh[2k(z+d)]}{\sinh^2 kd} \quad (8)$$


How much is that going to be the deviation, it depends on how much is your crest is steeper than the trough. So, here is the horizontal water particle displacement, vertical

particle displacement and the mass transport velocity, all these things, we have seen in the small amplitude wave theory with examples, worked out examples. Is that clear?

(Refer Slide Time: 24:50)

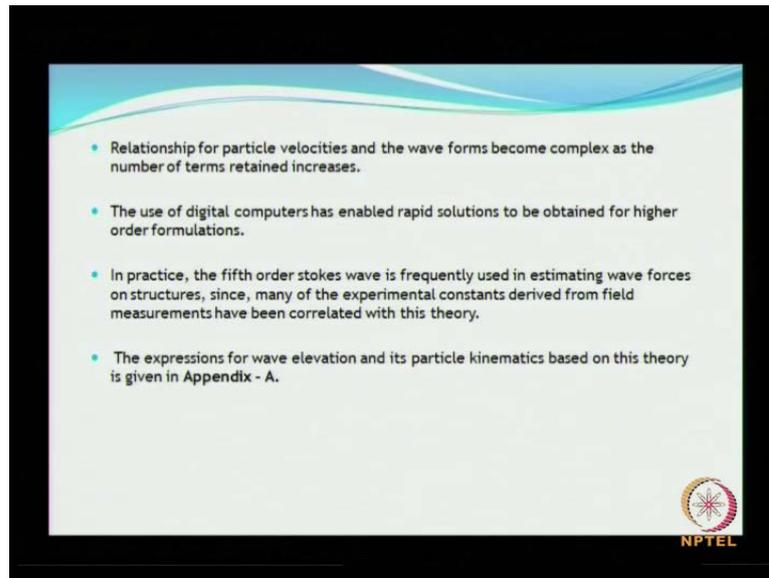
Subsurface pressure

$$\rho = \rho g \cdot \frac{H}{2} \cdot \frac{\cosh [k(d+z)]}{\cosh kd} \cdot \cos(kx - \sigma t) - \rho g z$$

$$+ \frac{3}{8} \rho g \cdot \frac{\pi H^2}{L} \cdot \frac{\tanh kd}{\sinh^2 kd} \left\{ \frac{\cosh 2k(z+d)}{\sinh^2 kd} - \frac{1}{3} \right\} \cdot \cos^2(kx - \sigma t) - \frac{1}{8} \rho g \cdot \frac{\pi H^2}{L} \cdot \frac{\tanh kd}{\sinh^2 kd} \cdot \{ \cosh 2k(d+z) - 1 \} \quad (9)$$

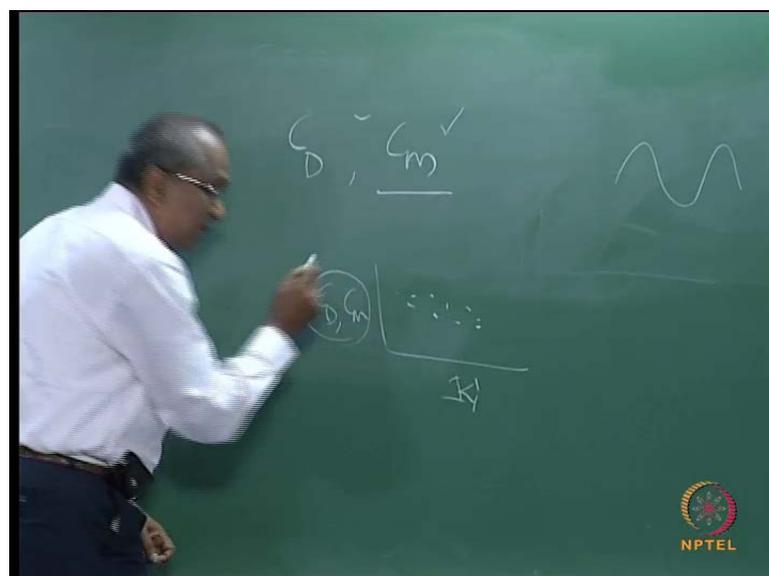

And this gives, the expression for the surf surface pressure all this variables, are known to you, like the k is the wave number, d is the water depth, z is the point of defining the particle velocities, and then sigma is 2 pi by t and load frequency. So, once you have these expressions so, what you need to know is, why you use higher order theories? What happens, if you do not use higher order theories at situations for the conditions. For which, you are suppose to use the higher order theories, that I have already illustrated with the example, that is very important. Now, how to use it is quite straight forward? Because all these variables are already known to you, only thing is the expression is quite long. Is that clear?

(Refer Slide Time: 25:46)



So, relationship for particle velocities and the wave forms, become complex as the number of terms retained in physics. So, usually they go they use 5<sup>th</sup> order theory, once you have the everything programmed, it becomes very simple, is that clear? So, the use of digital computers, has enabled rapid solutions to obtain for higher order solutions. So, usually 5<sup>th</sup> order theory is frequently used in estimating wave forces on structures. Since many experimental constants derived from field measurements have been correlated with this theory. What does that mean? See, as I said anyway this protest to the wave forces on the structures.

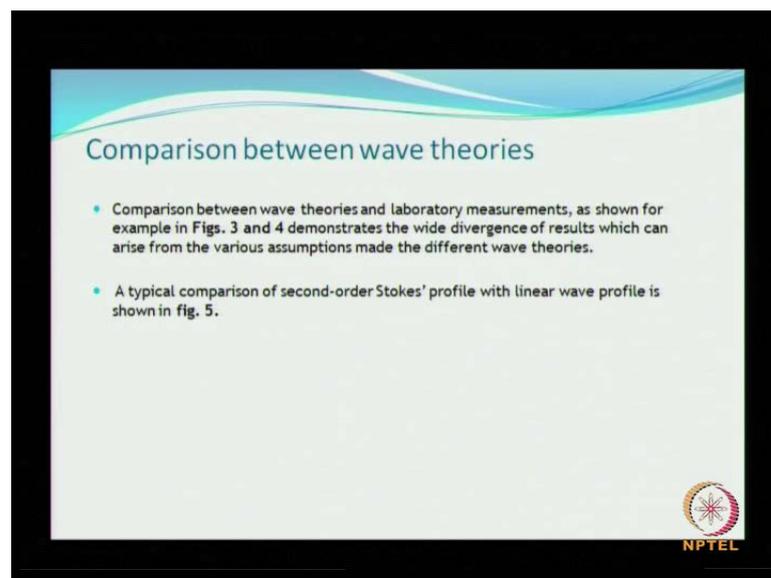
(Refer Slide Time: 26:49)



When we talk about the experimental constants, some of the use have two coefficients mainly, which are called as a hydrodynamics coefficients of drag on inertia. These are derived only from experiments, or from field. So, you generate a wave measure, the force and then from the force, and the wave time history, there is a possibility or a method to get the  $C_M$  and  $C_D$ .  $C_M$  and  $C_D$  are then given as a function of Keulegan Carpenter number, refer to my lecture on wave loads on structures.

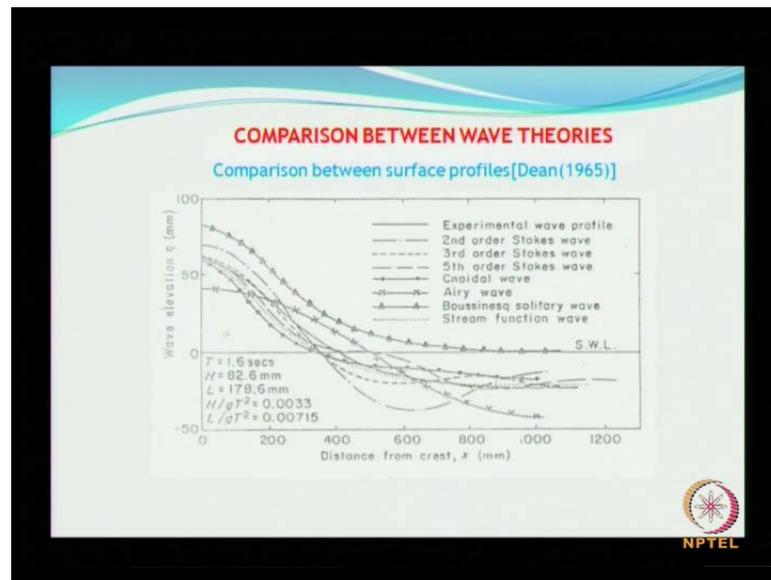
Now, you see that these coefficients, where are you getting it from? You are getting it from measurements either in the lab, or in the field. So, it is always, and these waves are not going to be sinusoidal at least the lab. You can try to generate sinusoidal so, that is the reason why they use the 5<sup>th</sup> order theory the expressions of all these theories is given in appendix a, that will come as a part of the lecture.

(Refer Slide Time: 28:16)



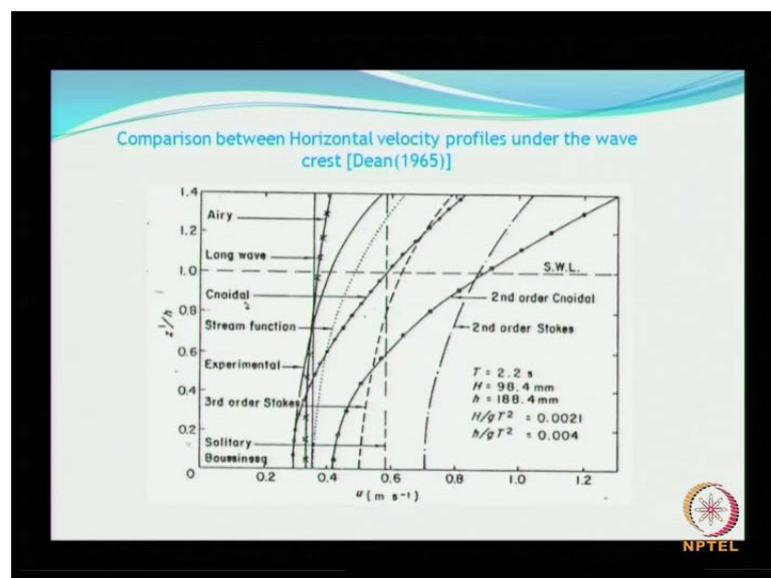
Comparison between the wave theories and lab measurements as shown in example. Demonstrates the wide divergence of the results, which can arise from various assumptions made in the different theories. I just simply highlighted that, there is going to be some difference, but we will see how, what is the kind of difference, we have?

(Refer Slide Time: 28:43)



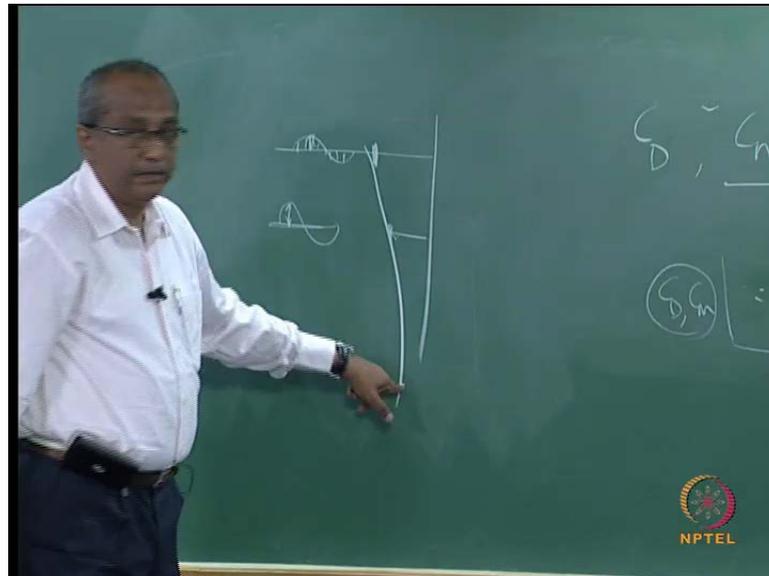
So, this is the comparison between profiles, surface profiles as given by professor Dean in 1965. So, you see the experimental profile, experimental profile is the thick line here. The second order Stokes line is here then stream function theory is here, then you see the Airys Theory here. So, you look at the kind of variation and remember that this is for a given wave height, wave period etcetera. All these data are given here so, this is from the distance from the crest so, the deviation is quite large, somewhere near the crest and this is what really matters.

(Refer Slide Time: 29:49)



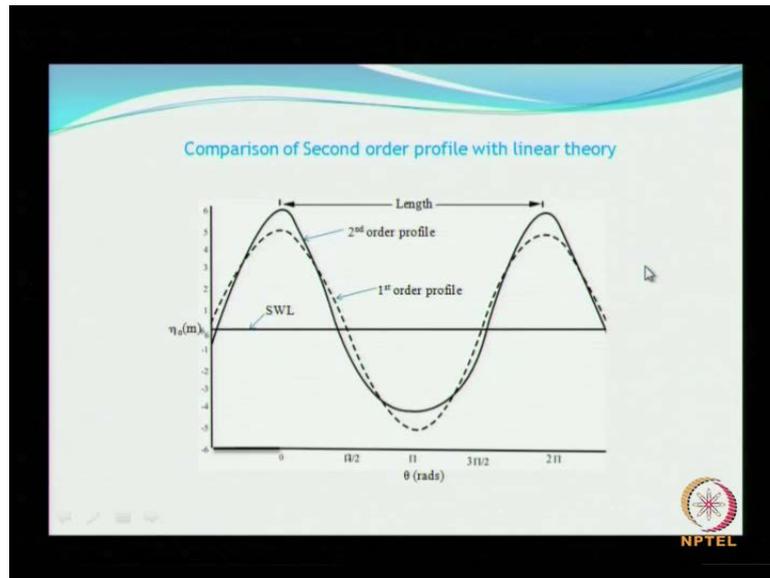
So, this explains, why higher order theories of finite amplitude theories are needed? Yet another comparison here, you compare the comparison of profiles are presented again by Dean, the maximum velocities at each elevation.

(Refer Slide Time: 30:11)



So, at each elevation, in the sense so, you have a time history here. You take only the maximum velocity and that is represented here so, next you have this, and you take only the maximum velocity and that is measured. So, it is plotted along the depth so, the some of the other theories are also included here, like second order Cnoidal Theory, which I am not going to cover here. But you see the Solitary wave theory, Cnoidal wave theory, Airys theory, all these things, how it really differs. The variation is, as from something like around 0.3 and it goes up to about 1 point, and the deviation is significant near the free surface.

(Refer Slide Time: 31:14)



(No Audio from 31:10 to 31:15)

So, look at this variation, simple variation, the difference between a second order profile and the Airys Theory.

(Refer Slide Time: 31:31)

### SOLITARY WAVE THEORY

- In very shallow waters wave crests become peaked and trough flattened. The surface profile is entirely above the SWL in the case of solitary waves.
- The wave is therefore not periodic and has no definite wave length.
- Boussinesq (1872) derived the characteristics of the solitary wave, in shallow water depth, directly from the general equation for steady flow.
- Solitary wave has proved useful in Engineering problems such as the study of very long waves like tsunamis and in determining wave properties near breaking in shallow water and for studying waves of maximum steepness in deep waters.
- Under such conditions the wave characteristics are independent of L and T and depends only on H and d.

$$H_b = 0.78d \quad \dots\dots\dots(10)$$

$$C = \sqrt{2g(H+d)} \quad \dots\dots\dots(11)$$

$$\eta = H_0 \operatorname{sech}^2 \sqrt{\frac{3}{4} \frac{H}{d^3}} (x - Ct) \quad \dots\dots\dots(12) \quad \text{x being origin at the crest.}$$

NPTEL

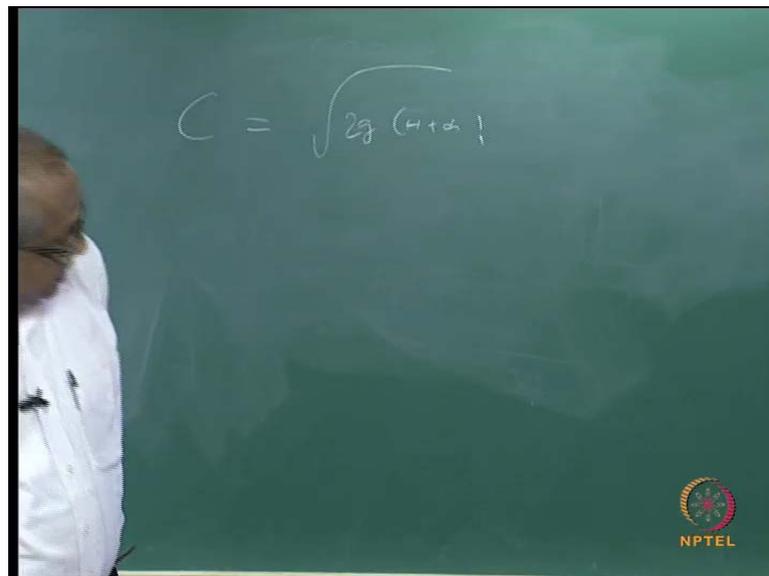
(No Audio from 31:28 to 31:34)

That explains something about, that has given as brief description, description about the finite amplitude theories. Now, we move on to solitary wave theory, as I said earlier,

there are some researches, who are of the opinion that Solitary wave theory. Solitary wave, I mean the tsunami can be the characteristics of the tsunami, can be represented approximately as that of a solitary wave. But there is another set of researches would differ from it. In very shallow waters, wave crest become peaked and trough flattened, the surface profile is entirely above the still water line in the case of a solitary wave.

So, the wave is therefore, not periodic and has no definite wavelength. This is the major difference between Solitary wave and the other waves. Boussinesq derived the characteristics of the Solitary waves in Shallow water directly from general equation of a steady flow, the derivation was quite straight forward. Solitary wave has proved useful in engineering problems, such as study of very long waves like tsunamis. And in determining wave properties near breaking in Shallow water, and for study of waves of maximum steepness in deep waters. Under such conditions the wave characteristics are independent of wavelength  $t$ , and will depends only on  $H$ .

(Refer Slide Time: 33:36)



In this case, the celerity the speed of a solitary wave is given as root of, which we will see later. The profile given by, represented by the solitary wave can be represented as shown here, this is  $\sec$  and  $H$ . And here, all other parameters known to you  $X$  is being the origin at the crest, based on this only, you can draw the profile, is that clear? A different values for a given  $C$ , given characteristics of  $H$ , and  $d$  use these expressions and

traverse the profile from X. (()) X is starting from the crest of the profile, a crest of the Solitary wave.

(Refer Slide Time: 34:33)

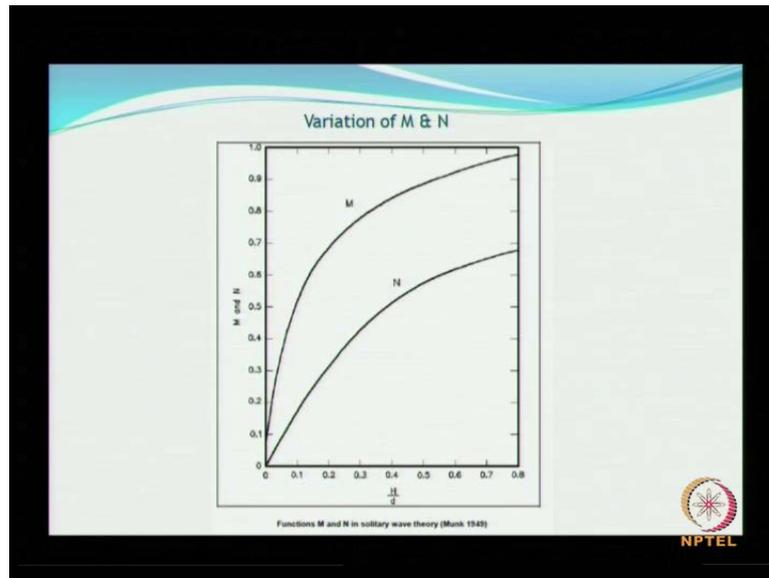
$$u = \frac{CN \left[ 1 + \cos \left( M \left[ \frac{z+d}{d} \right] \right) \cdot \cosh \left( M \frac{x}{d} \right) \right]}{\left\{ \cos \left( M \left[ \frac{z+d}{d} \right] \right) + \cosh \left( M \frac{x}{d} \right) \right\}^2} \quad \dots\dots\dots (13)$$

$$w = \frac{CN \left[ \sin \left( M \left[ \frac{z+d}{d} \right] \right) \cdot \sinh \left( M \frac{x}{d} \right) \right]}{\left\{ \cos \left( M \left[ \frac{z+d}{d} \right] \right) + \cosh \left( M \frac{x}{d} \right) \right\}^2} \quad \dots\dots\dots (14)$$

Where M, N are functions of H/d shown in Fig. 6

The horizontal and vertical, particle velocities are given by this expression, as C is the celerity as disguise, as discussed earlier. And M, and N are values or functions, which can be obtained from a figure, which represented as a function of H by d, which will be shown soon. All other parameters are known to you, because X is starting from the crest of the wave, as said earlier d is the water depth, s is below the still water line, m and n are obtained from:

(Refer Slide Time: 35:17)



The figure as shown here, once you have H by d for the given wave condition. You can derive the values for M and N, use that expression, earlier expression to get u and w.

(Refer Slide Time: 35:33)

$u_{\max}$  will occur when  $x = t = 0$  given as

$$u_{\max} = \left[ CN / \left( 1 + \cos \left\{ M \left( \frac{z+d}{d} \right) \right\} \right) \right] \quad \dots(15)$$

>The total energy in a solitary wave is about evenly divided between kinetic and potential energy.

Total wave energy per unit crest width is

$$E = \frac{8}{3\sqrt{3}} \rho g H^3 / 2 d^3 / 2 \quad \dots(16)$$

the pressure beneath a solitary wave depends on local fluid velocity, as does the pressure under a cnoidal wave, however, it may be approximated by

$$p = \rho g (y_s - y) \quad \dots(17)$$

So,  $u_{\max}$  will occur, when X and t equal to 0 and that is given, as shown here. The total energy in a Solitary wave is about, evenly divided between the kinetic energy and the potential energy. I will not go into the detail of deriving the kinetic energy, although it is quite straight forward. Again use the same principle, as we have done for on a similar procedure. What we have done for linear waves, and you see that the total energy can be

obtained per unit crest, is given as this expression whereas, the pressure is given by this expression.

(Refer Slide Time: 36:19)

Consider the steady flow case where the wave form remains solitary and water enters with a velocity  $V_1 = C$ . Applying Bernoulli's equation.

$$d_1 + \frac{v_1^2}{2g} = d_2 + \frac{v_2^2}{2g} \quad \dots(18)$$

From continuity Eq.  $d_1 V_1 = d_2 V_2$  for width perpendicular to screen. Substituting and rearranging  $V_1 = V = C$

$$\frac{V^2}{2g} = \frac{d_2 + d_1}{1 - \left(\frac{d_2}{d_1}\right)^2} \quad \dots(19)$$

If  $d_1 = d_1$ ,  $d_2 = d_1 + H$  and dropping higher terms

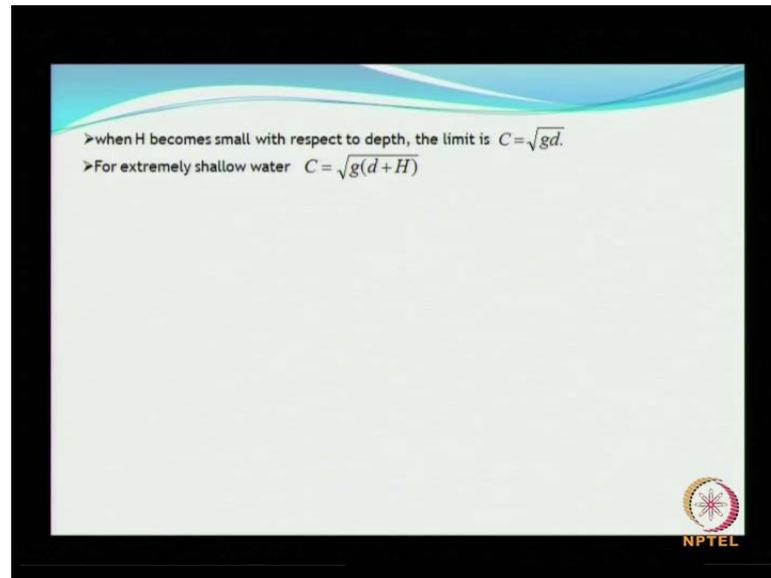
$$C = \sqrt{gd} \left[ 1 + \frac{3H}{2d} \right]^{1/2} \quad \dots(20)$$

The diagram shows a solitary wave profile on a horizontal bed. The water depth is  $d_1$  on the left and  $d_2$  on the right. The wave height is  $H$ . The velocity is  $V_1 = C$  on the left. The NPTEL logo is in the bottom right corner.

So, considering steady flow case, where the wave form remains solitary and wave enters with a velocity  $V_1$  equal to  $V = C$ .  $V_1$  equal to  $C$ , then you consider these two locations so, this is  $d_2$  and this is  $d_1$ , and here  $V_1$  is equal to  $C$ . Use the continuity equation as shown here, and then continuity equation is like, applying the Bernoulli Equation as given here, and then use the continuity equation. From which, I am rearranging  $V_1$  equal to  $V = C$ , you can obtain an expression as shown here.

But then,  $d_1$  equal to  $d_1$  and whereas  $d_2$  is going to be  $d_1$  plus, this is height of the wave. So, substitute in this equation then you get, an expression for the speed of the Solitary Wave. The speed of the Solitary Wave is nothing, but the Shallow Water Wave Condition, multiplied by this value. You understood? So, if  $H$  is large then this component is going to be higher then, you consider just a linear theory propagating in the Shallow water, you understood?

(Refer Slide Time: 38:09)



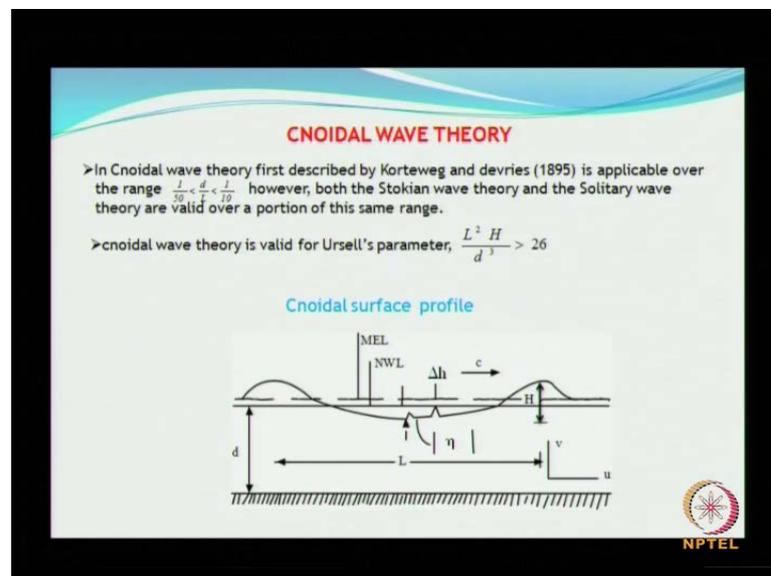
Slide content for slide 38:09:

- > when H becomes small with respect to depth, the limit is  $C = \sqrt{gd}$ .
- > For extremely shallow water  $C = \sqrt{g(d+H)}$

The slide features a blue header with a wavy pattern and an NPTEL logo in the bottom right corner.

So, when H becomes small with respect to depth the limit, as I have told for extremely Shallow waters, you can also assume it as equal to root of g into d plus H.

(Refer Slide Time: 38:28)

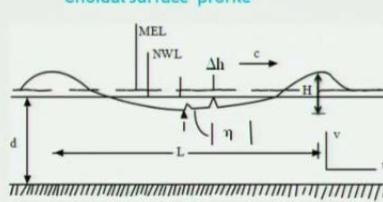


Slide content for slide 38:28:

### CNOIDAL WAVE THEORY

- > In Cnoidal wave theory first described by Korteweg and devries (1895) is applicable over the range  $\frac{1}{10} < \frac{d}{L} < \frac{1}{10}$  however, both the Stokian wave theory and the Solitary wave theory are valid over a portion of this same range.
- > cnoidal wave theory is valid for Ursell's parameter,  $\frac{L^3 H}{d^3} > 26$

**Cnoidal surface profile**



The diagram shows a cross-section of a cnoidal wave. The water depth is labeled 'd'. The wavelength is labeled 'L'. The wave height is labeled 'H'. The surface elevation is labeled 'η'. The velocity is labeled 'u'. The diagram also shows the Mean Envelope Line (MEL) and the Nonlinear Wave Line (NWL). The wave is moving to the right with velocity 'c'. The Ursell's parameter is labeled 'Δh'.

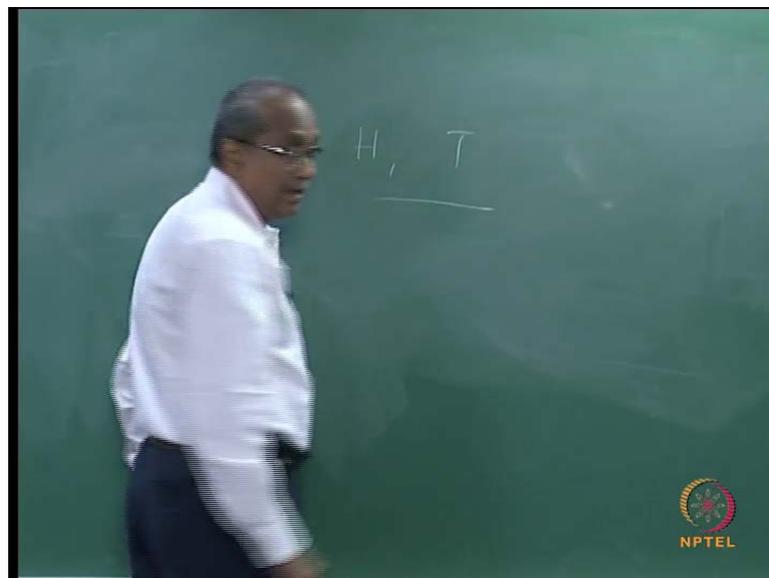
The slide features a blue header with a wavy pattern and an NPTEL logo in the bottom right corner.

So, that is about Solitary Wave Theory, then we move on to the Cnoidal waves, where in the cnoidal wave? It was first described by Korteweg and Devries in 1895. Applicable over a wide range that is d by L, ranging between 1 by 10 and 150, but however, both Stokian Wave Theory, and the Solitary Wave Theory are valid over. The over a portion of this same range, as we have seen earlier. So, Cnoidal Wave Theory is valid for the

Ursells parameter, greater than 26. In our department, in the department of ocean engineering, our wave maker can generate Solitary Wave Theory.

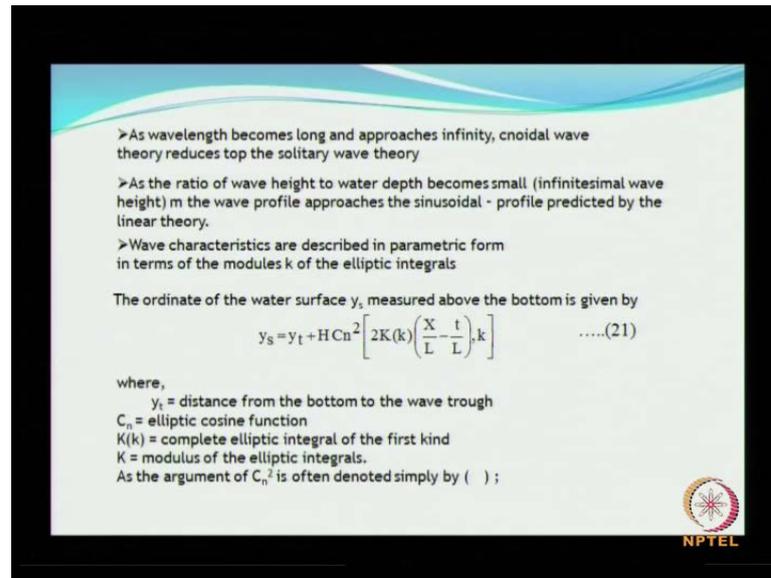
Solitary wave as well as, Cnoidal waves, leave alone other waves, because other waves any normal wave maker can generate, knowingly or unknowingly or unknowingly you may be generating higher order wave theories. It may be difficult for you to generate a linear wave theory, linear wave whatever, you generate in the lab you assume it, as linear wave which is not the case.

(Refer Slide Time: 40:04)



So, unknowingly what I am trying to say is, when you want to use normally fix your wave height and wave period. You do not even bother whether, which theory is going to follow. But in the case of Cnoidal and Solitary, when you want to use your wave maker to generate such waves, all these informations are built in. All these conditions are satisfied only after, which you will see that, such kind of waves are generated in the flume.

(Refer Slide Time: 40:39)



>As wavelength becomes long and approaches infinity, cnoidal wave theory reduces to the solitary wave theory

>As the ratio of wave height to water depth becomes small (infinitesimal wave height) the wave profile approaches the sinusoidal - profile predicted by the linear theory.

>Wave characteristics are described in parametric form in terms of the modulus  $k$  of the elliptic integrals

The ordinate of the water surface  $y_s$ , measured above the bottom is given by

$$y_s = y_t + H Cn^2 \left[ 2K(k) \left( \frac{x-t}{L} \right), k \right] \quad \dots(21)$$

where,

- $y_t$  = distance from the bottom to the wave trough
- $Cn$  = elliptic cosine function
- $K(k)$  = complete elliptic integral of the first kind
- $k$  = modulus of the elliptic integrals.

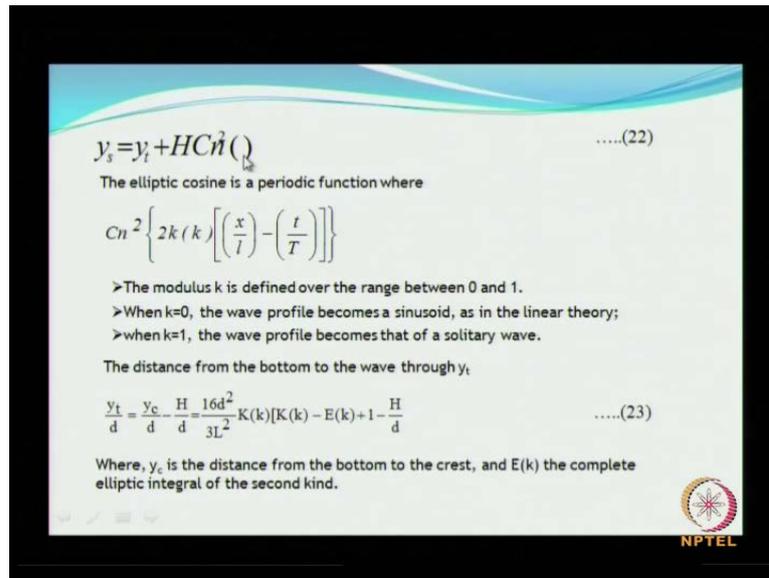
As the argument of  $Cn^2$  is often denoted simply by  $( \quad )$  ;



So, the profile looks something like this, it has a very flat, trough and a steep crest, a photograph of, which was already shown to you. As wavelength becomes long and approaches infinity Cnoidal Theory reduces to the Solitary Wave Theory. As the ratio of wave height to water depth become small, infinitised wave. I mean, the wave profile approaches the sinusoidal wave profile, predicted by the linear theory. These are all some of the statements, then wave characteristics are described by what is called as modulus  $k$  of elliptical interval.

So, the profile is measured from the bottom sea bed and this is the profile, this is the water surface. I mean, the wave profile and this is given by  $y_t$ ,  $y_t$  is the distance from the bottom to the trough and  $Cn$  is the elliptical cosine mathematical function and then  $k$  is complete elliptical integral of the first kind.  $k$  is,  $k$  of  $k$  is complete elliptical integral of first kind and  $K$  is the modulus of elliptical integrals as the argument  $C$  of  $n$  square is often denoted simply by this.

(Refer Slide Time: 42:00)



$$y_s = y_t + H C n^2 \left( \frac{x}{l} - \left( \frac{t}{T} \right) \right) \quad \dots(22)$$

The elliptic cosine is a periodic function where

$$C n^2 \left\{ 2k(k) \left[ \left( \frac{x}{l} \right) - \left( \frac{t}{T} \right) \right] \right\}$$

- >The modulus k is defined over the range between 0 and 1.
- >When k=0, the wave profile becomes a sinusoid, as in the linear theory;
- >when k=1, the wave profile becomes that of a solitary wave.

The distance from the bottom to the wave trough  $y_t$

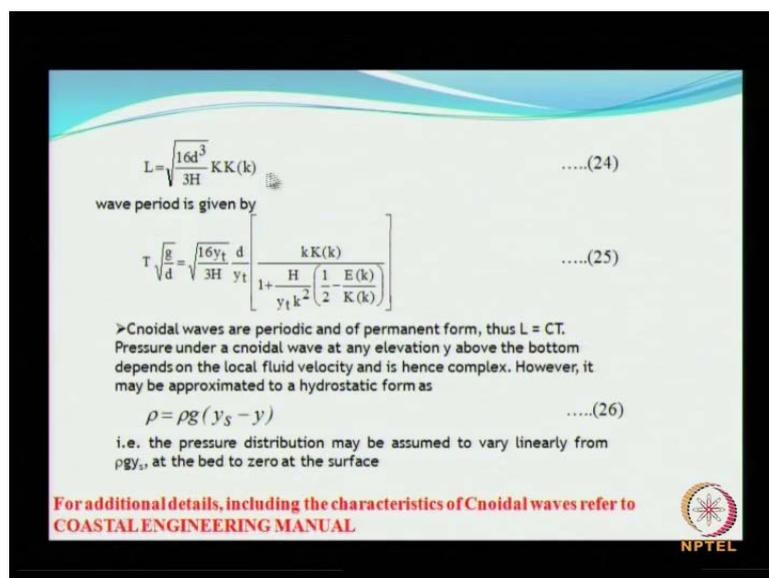
$$\frac{y_t}{d} = \frac{y_c}{d} - \frac{H}{d} = \frac{16d^2}{3L^2} K(k)[K(k) - E(k) + 1] - \frac{H}{d} \quad \dots(23)$$

Where,  $y_c$  is the distance from the bottom to the crest, and  $E(k)$  the complete elliptic integral of the second kind.



As you can see here, the elliptical cosine function is periodic, where  $C n$  is shown as shown here. It is periodic in space and time so, the modulus  $K$ , small  $k$  as we have seen is defined over range between 0 and 1. And when  $K$  is equal to small  $k$  is equal to 0, the wave profile becomes, linear and in the sinusoidal. And when  $k$  is equal to 1, the wave profile becomes that of a Solitary wave. The distance from the bottom to the wave trough  $y_t$ , this can be obtained by this expression as shown here for details reference refer, I would suggest for this to refer Coastal Engineering Manual.

(Refer Slide Time: 42:52)



$$L = \sqrt{\frac{16d^3}{3H} K(k)} \quad \dots(24)$$

wave period is given by

$$T \sqrt{\frac{g}{d}} = \sqrt{\frac{16y_t}{3H} \frac{d}{y_t} \left[ \frac{kK(k)}{1 + \frac{H}{y_t k^2} \left( \frac{1}{2} \frac{E(k)}{K(k)} \right)} \right]} \quad \dots(25)$$

- >Cnoidal waves are periodic and of permanent form, thus  $L = CT$ .
- Pressure under a cnoidal wave at any elevation  $y$  above the bottom depends on the local fluid velocity and is hence complex. However, it may be approximated to a hydrostatic form as

$$\rho = \rho g (y_s - y) \quad \dots(26)$$

i.e. the pressure distribution may be assumed to vary linearly from  $\rho g y_s$  at the bed to zero at the surface



**For additional details, including the characteristics of Cnoidal waves refer to COASTAL ENGINEERING MANUAL**

So, the L is calculated as shown here, and these parameters already been explained and the wave period can be given like this. The pressure is provided by this expression, and as I have suggested including the characteristics of the waves, refer Coastal Engineering Manual, or even the book from is also quite good.

(Refer Slide Time: 43:17)

**STREAM FUNCTION THEORY**

- Although the Stokes waves satisfy the basic Laplace equation and the sea bed boundary conditions, the free surface boundary conditions are not fully satisfied.
- The components of flow at the surface are not necessarily in accordance with the shape of the surface and its motions, nor is a restriction placed on the pressure immediately below the free surface.
- The dynamic free surface boundary condition is

$$\eta + \frac{1}{2g} [(u-C)^2 + w^2] - \frac{C^2}{2g} = \text{const.} \quad \dots(27)$$

The stream function solution may be expressed as

$$\phi(x, z) = \frac{L}{T} z + \sum_{m=1}^N X(m) \sinh \left[ \frac{2m}{L} (d + \eta) \right] \cos \left[ \frac{2m}{L} x \right] \quad \dots(28)$$



So, that explains the salient features of Cnoidal Wave Theory. Then we move on to Stream Function Theory, although the Stokes waves satisfy the basic Laplace equation and the sea bed bottom boundary condition. The free surface boundary condition is not fully satisfied. The free surface boundary condition, I have already explained to you, the components of flow at the surface are not necessarily. In accordance, with the shape of the surface and its motion, nor is a restriction placed on the pressure immediately below the surface. The dynamic free surface condition, in this case is provided like this, and the stream function solution may be expressed, as given by this expression equation 28.

(Refer Slide Time: 44:18)

**STREAM FUNCTION THEORY**

and evaluated by setting  $z = \eta$  to give a surface

$$\eta = \frac{T}{L} \phi_n - \frac{T}{L} \sum_{n=1}^N X(n) \sinh \left[ \frac{2\pi n}{L} (d + \eta) \right] \cos \left[ \frac{2\pi n}{L} x \right] \quad \dots(29)$$

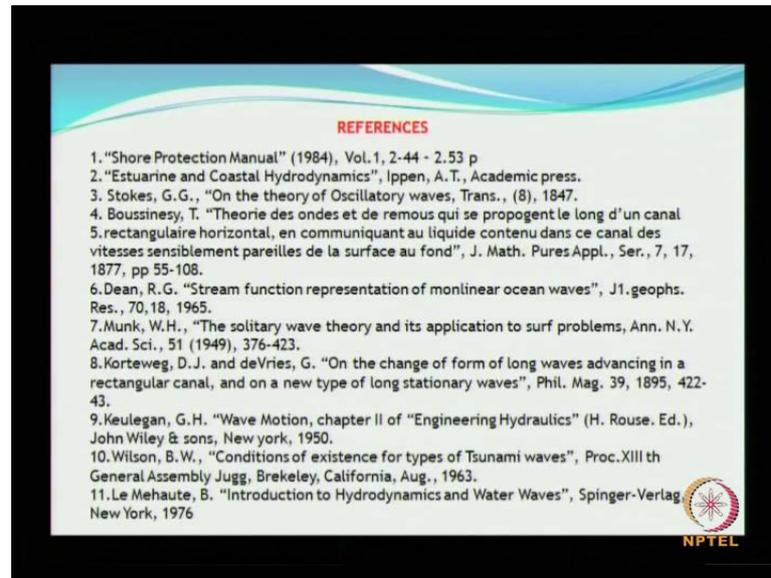
>For a particular water depth, wave height and period the function exactly satisfies the Laplace equation, the sea bed and the surface flow boundary conditions for arbitrary values of the constants  $L$ ,  $\phi_n$  and  $X(n)$ .

>These values can be obtained numerically so that the dynamic free surface boundary condition is have been published by Dean (1965).



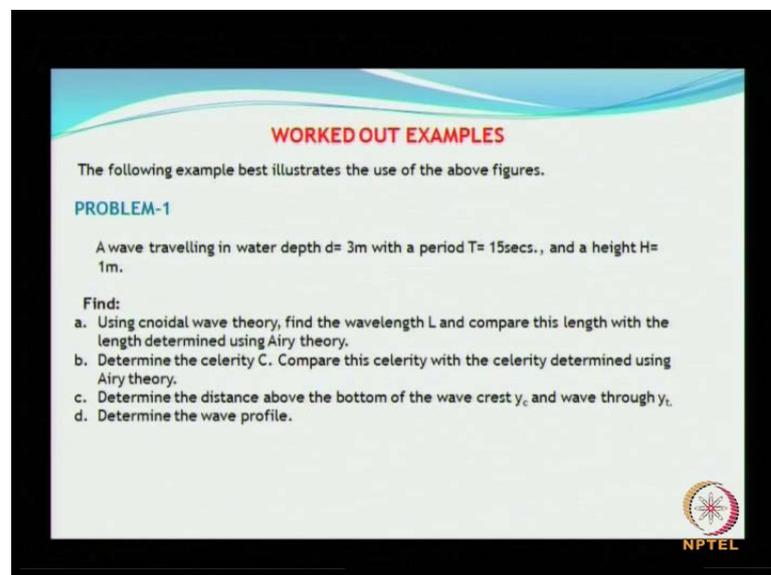
And evaluated at, and evaluating by setting  $z$  equal to  $\eta$ , not  $z$  equal to  $0$ , to give a surface as shown here. This is the surface of a stream function that can be described by stream function theory. For a particular water depth, wave height and wave period the function exactly satisfy the laplace equation. The sea bed and the surface flow boundary conditions for arbitrary values of  $L$   $\phi_n$ , that is capital  $X_n$ . So, these values can be obtained numerically so, that the dynamic free surface boundary condition is can be obtained as published by Dean. So, I would not go into the details of the stream function theory, it is not that widely used (( )) people, there are some cases, where they you need to know the stream function theory.

(Refer Slide Time: 45:21)



I would, this is a series of references that I have collected and there may be some reason the literature also, but for a class like this. I think it is good, you look at some of these references, in order to brush up and then go for the into your research activity.

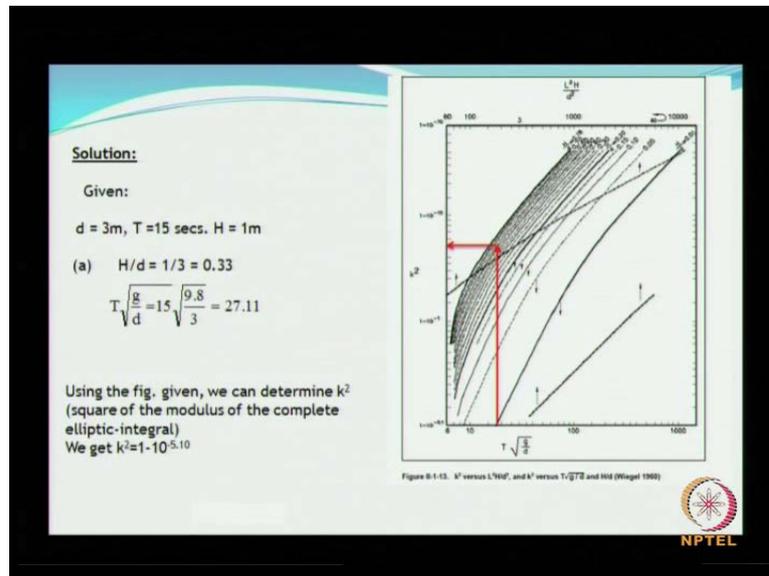
(Refer Slide Time: 45:44)



Now, let us continue with a few worked out examples. So, the Cnoidal Wave Theory I will show, how the celerity is calculated the a wave travelling in a water depth of three meters and wave period fifteen seconds and height one meter. Find, using Cnoidal Theory, find wavelength and compare it is, compare this with the wavelength determined

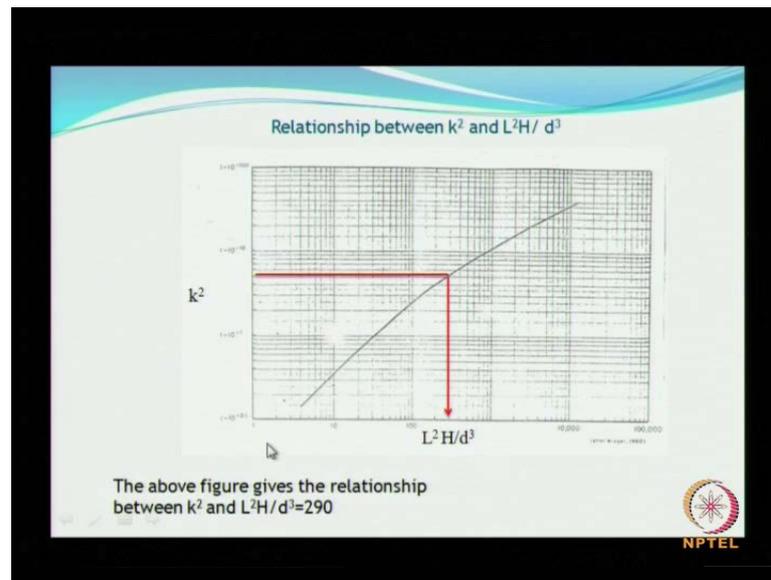
by Airys Theory. Determine celerity compare this, with the celerity of the Airys Theory determine the distance between the above, the bottom of the crest y C and trough y t, and determine the wave profile. This is the problem that is, that we need to look into;

(Refer Slide Time: 46:40)



So, these pictures are given available in the coastal engineering manual, or the shore protection manual. So, for the given problem, d is, this much T so you calculate the value H by d, which is coming to this much, also calculate this parameter T divided by root. I mean, g by d which is working out to 27 point something. So, get into this picture, which shows the determination of square of modulus of complete elliptical integral. So, this becomes handy, this nomograms so, get into this picture and get the value of k and we get k as shown here, one minus ten to the power minus 5.1.

(Refer Slide Time: 47:33)



So, this shows the relationship between this k square, which you have determined from the earlier plot to get you the value for L by L square that is nothing, but the Ursells Parameter. And Ursells parameter is working out to 290, naturally it is greater than 26 so, it is following Cnoidal Wave Theory.

(Refer Slide Time: 48:00)

i.e.  $L = 290 \frac{d^3}{H}$

$$L = \frac{290(3)^3}{1}$$

i.e. **L=88.5m**

By Airy Theory,

$$L = \frac{gT^2}{2} \tanh \frac{(2\pi d)}{L} = 80.6 \text{ m}$$

To check the validity of cnoidal wave theory for the given wave condition we have

$$\frac{d}{L} = \frac{3}{88.5} = 0.0339 < 1/8$$

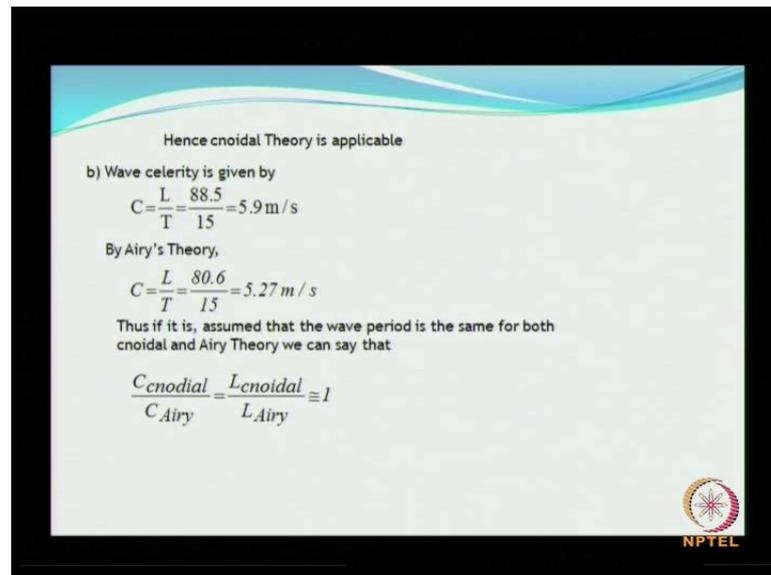
Ursull or stoke's parameter

$$\frac{L^2H}{d^3} = \frac{1}{(d/L)^2} \left( \frac{H}{d} \right) = 290 > 26$$

So, once that is known, this L H L square H divided by d cube, you can arrive at the wave length, and once wavelength so, this is the wavelength, and from the linear theory. I do not want to explain to you, how the linear theory is calculated? And from the linear

theory, you can calculate this as 80.6. So, that is the kind of difference, you have between the wavelengths to check the validity of linear theory, you have already done. So,  $d$  by  $L$  and this one, I have already explained about the Ursells Parameter, you need not go into that.

(Refer Slide Time: 48:42)



Hence cnoidal Theory is applicable

b) Wave celerity is given by

$$C = \frac{L}{T} = \frac{88.5}{15} = 5.9 \text{ m/s}$$

By Airy's Theory,

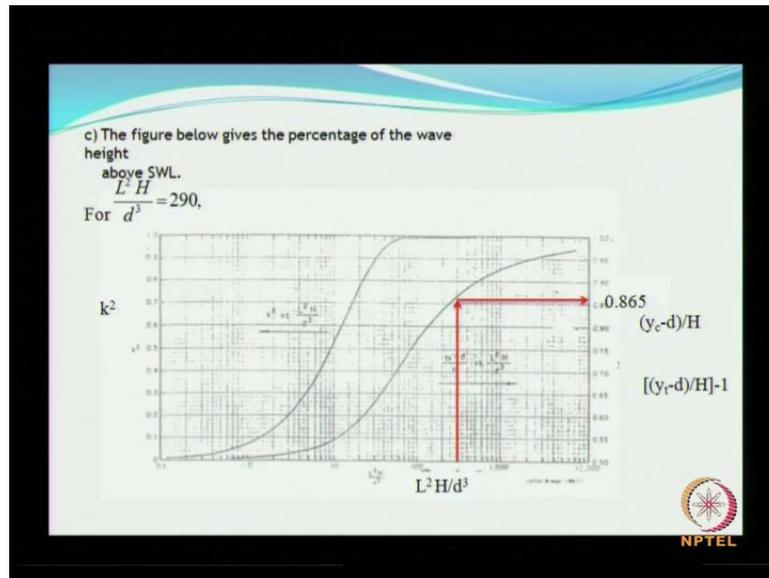
$$C = \frac{L}{T} = \frac{80.6}{15} = 5.27 \text{ m/s}$$

Thus if it is, assumed that the wave period is the same for both cnoidal and Airy Theory we can say that

$$\frac{C_{cnoidal}}{C_{Airy}} = \frac{L_{cnoidal}}{L_{Airy}} \cong 1$$

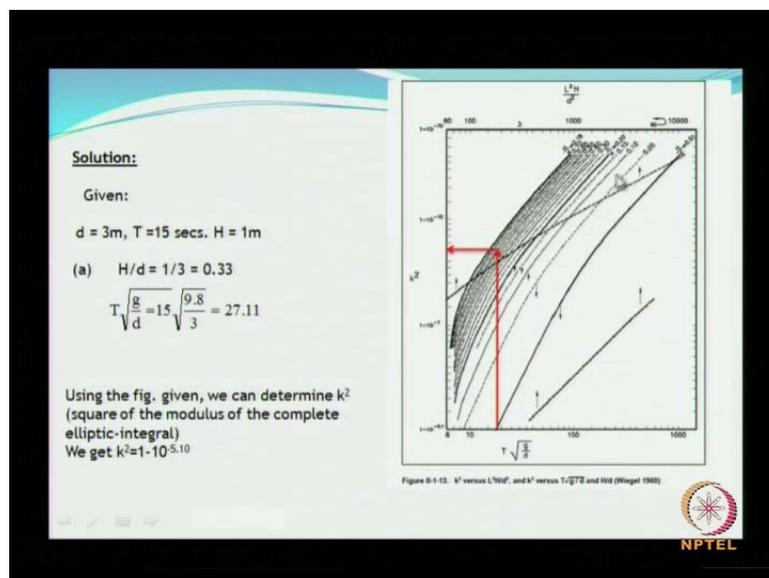

So, once you have calculated your  $L$  is quite straight forward, to calculate the corresponding celebrities. So, in this particular case that is not much of difference between the celebrities, and thus if it assumed that, the wave period is the same for both the Cnoidal, and we can say that, it is equal to 1. But that is not correct, there is still a difference between the Cnoiadal and this one.

(Refer Slide Time: 49:07)



The last one is, about the percentage of crest and trough, so you have already selected this and then you have to go and find out this value, so this is  $y_c$  the portion of the location of the crest.

(Refer Slide Time: 49:31)



So, in this particular picture, I forgot to tell you, that you need to consider this  $H$  by  $d$  while selecting this point so  $H$  by  $d$  here is 0.3, and 0.3 is somewhere here.

(Refer Slide Time: 49:43)

$\frac{Y_c - d}{H}$  is found to be 86.5%

Hence  $Y_c = 0.865H + d$   
 $= 0.865(1) + 3$   
 $Y_c = 3.865 \text{ m}$

Similarly from figure 14

$\frac{(y_t - d)}{H} + 1 = 0.865$

$Y_t = (0.865 - 1)(1) + 3 = 2.865 \text{ m}$



So, from this picture you can arrive at  $y_c$  and  $y_t$ .

(Refer Slide Time: 49:49)

**Problem 2**

A wave of height 1m and length 60m propagates in a water depth of 6m. Which theory could be adopted for the evaluation of the water particle kinematics. For the wave with the same frequency, find the range of wave heights in which case stream function theory could be applied. Take care of the breaking criteria.

**Solution:**

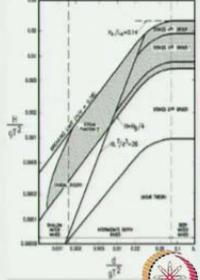
$d = 6\text{m}, H = 1\text{m}, L = 60\text{m}.$   
 $d/L = 6/60 = 0.1$  and corresponding  $d/L_0 = 0.056, L_0 = 107,$

Hence,  $T = 8.2 \text{ sec}$   
 $H/gT^2 = 1/(9.81 * 8.2 * 8.2) = 0.0015, d/gT^2 = 0.009$

Hence Airy theory is valid for kinematics.

For stream function theory  $H/gT^2$  should lie between 0.017 and 0.006

Using these limits  $H = 4$  to  $11.48\text{m}.$   
However, the maximum wave height is  $0.78 * 6 = 4.68\text{m}$   
Hence  $H$  should range from  $4\text{m}$  to  $4.68\text{m}.$



So, we will move on to this second example, a wave height is given as 1 meter, wavelength is given as 60 meters. It is propagating, the wave is propagating in a water depth of 6 meters, which theory could be adopted for the evaluation of water particle kinematics. For the wave, with the same frequency find the range of wave heights in which the Stream Function Theory could be adopted, take care of the breaking criteria. So, the problem is quite simple, only you use this, the problem looks as if it is not

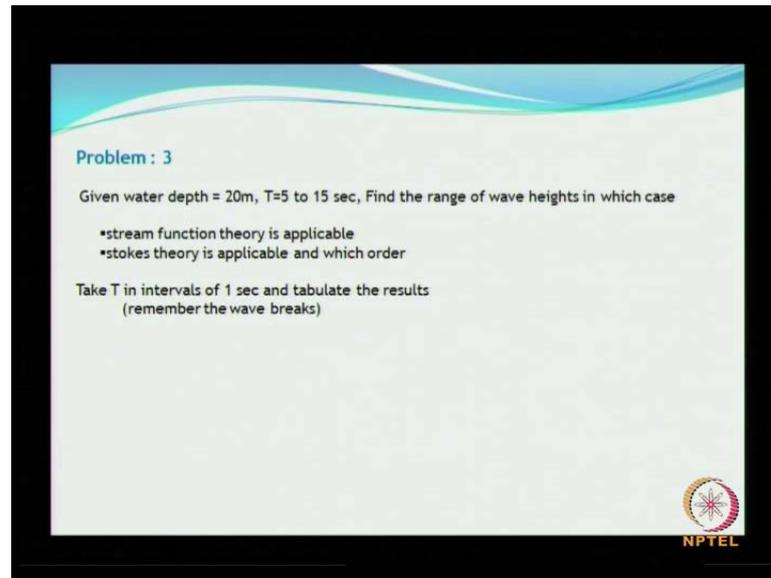
difficult, it is a very simple problem. The only thing is, this is a kind of an exercise to make yourself familiar with this graph that is all, which very often students forget.

So, wave height is given wave, water depth is given so you calculate your  $d$  by  $L$  corresponding  $d$  by  $L$  naught. All these things are known to you  $H$  by  $dt$  square also can be calculated. So, for this picture you need  $H$  by  $dt$  square and  $d$  by  $g T$  square so since wavelength is given to you normally, which is not the case, normally they give directly the wave period, but in this problem, you are given wavelength. So, you use the dispersion relationship and then, and you get the wave period. So, you calculate the wavelength and then calculate your wave period, and then you get these two pictures.

And then for the stream function theory, this one should lie between 0.17 and 0.017 and 0.0 0 6. Unfortunately this picture is not so clear, so you have to take the corresponding this value, and the corresponding this and using these values for the stream function theory. So, in this case, for these two parameters Airys Theory is valid, using this picture. For stream function  $H T$  should be lying in between this value, that is  $H$  by; So, the stream function theory is somewhere, here this is the region where the stream function theory to should follow the values.

So, using these limits,  $H$  will be ranging between four and eleven meters, but you know that the maximum wave height cannot be greater than 0.78 times water depth. That means, here water depth is 6 meters, so the water depth, the wave height cannot **(( ))** greater than 4.8 meters. So, the range of wave heights that could be there, is between four meters and 4.68 meters, is that clear? So, this is just an example, to show you, how you can use this unfortunately the picture is not so clear, because it is quite a big picture. So, you need to do this and probably use the hand out, in case if you want to understand better.

(Refer Slide Time: 53:18)



**Problem : 3**

Given water depth = 20m, T=5 to 15 sec, Find the range of wave heights in which case

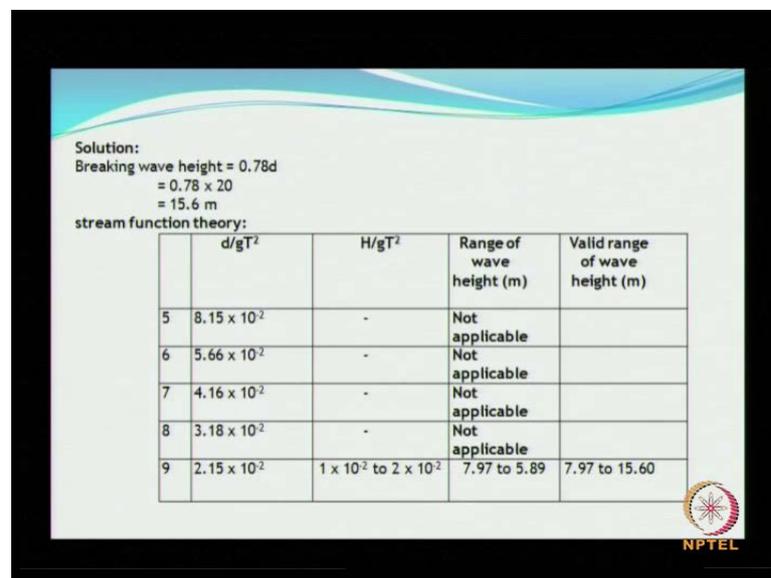
- stream function theory is applicable
- stokes theory is applicable and which order

Take T in intervals of 1 sec and tabulate the results  
(remember the wave breaks)



Given water depth to find the ranges of wave heights in which case Stream Function Theory is applicable, Stokes is applicable and which order. So, you take a wave period in intervals of one second and tabulate the results, remember again the wave breaks.

(Refer Slide Time: 53:42)



**Solution:**  
Breaking wave height =  $0.78d$   
 $= 0.78 \times 20$   
 $= 15.6 \text{ m}$

stream function theory:

	$d/gT^2$	$H/gT^2$	Range of wave height (m)	Valid range of wave height (m)
5	$8.15 \times 10^{-2}$	-	Not applicable	
6	$5.66 \times 10^{-2}$	-	Not applicable	
7	$4.16 \times 10^{-2}$	-	Not applicable	
8	$3.18 \times 10^{-2}$	-	Not applicable	
9	$2.15 \times 10^{-2}$	$1 \times 10^{-2}$ to $2 \times 10^{-2}$	7.97 to 5.89	7.97 to 15.60



So, what we have done is so here given on the first column, I have given the wave heights, wave periods in intervals of one. Calculated the values of  $d$  by  $g T$  square, calculated the values of  $H$  by  $g T$  square and then the range taking into considering the breaking criteria and then range of wave heights, so this is not applicable. Because the

wave is going to break then the range of wave heights, that could be possible for the Stream Function Theory, is that clear? So, use that picture earlier problem, we had that small nomogram, that nomogram need to be used.

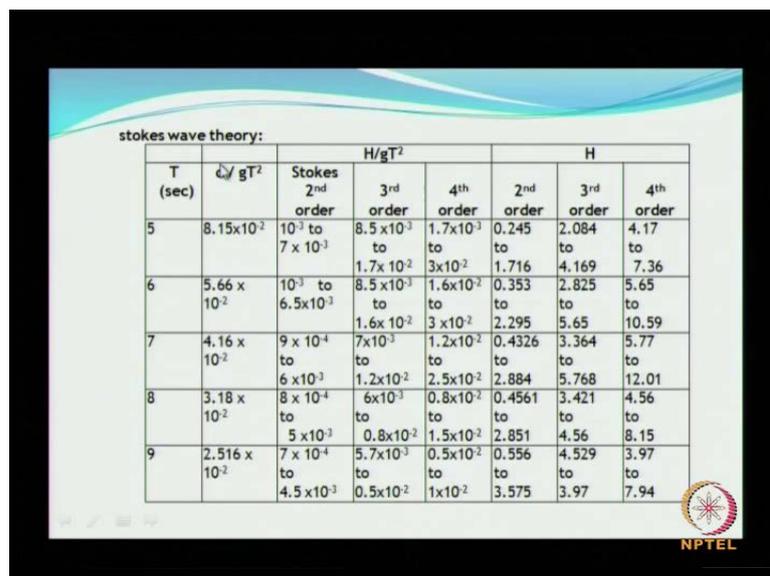
(Refer Slide Time: 54:28)



10	$2.04 \times 10^{-2}$	$9.5 \times 10^{-3}$ to $1.0 \times 10^{-2}$	9.31 to 9.83	9.31 to 9.83
11	$1.68 \times 10^{-2}$	$7 \times 10^{-3}$ to $0.5 \times 10^{-3}$	5.94 to 8.31	5.94 to 8.31
12	$1.415 \times 10^{-2}$	$5 \times 10^{-3}$ to $9 \times 10^{-3}$	7.06 to 12.71	7.06 to 12.71
13	$1.21 \times 10^{-2}$	$5 \times 10^{-3}$ to $1 \times 10^{-2}$	6.63 to 16.58	8.28 to 15.60
14	$1.04 \times 10^{-2}$	$4 \times 10^{-3}$ to $9 \times 10^{-3}$	7.69 to 17.304	7.69 to 15.60
15	$9.06 \times 10^{-3}$	$2.3 \times 10^{-3}$ to $6.8 \times 10^{-3}$	5.08 to 15.01	5.08 to 15.01

So, up to 15 seconds we have given the variation of wave heights.

(Refer Slide Time: 54:36)



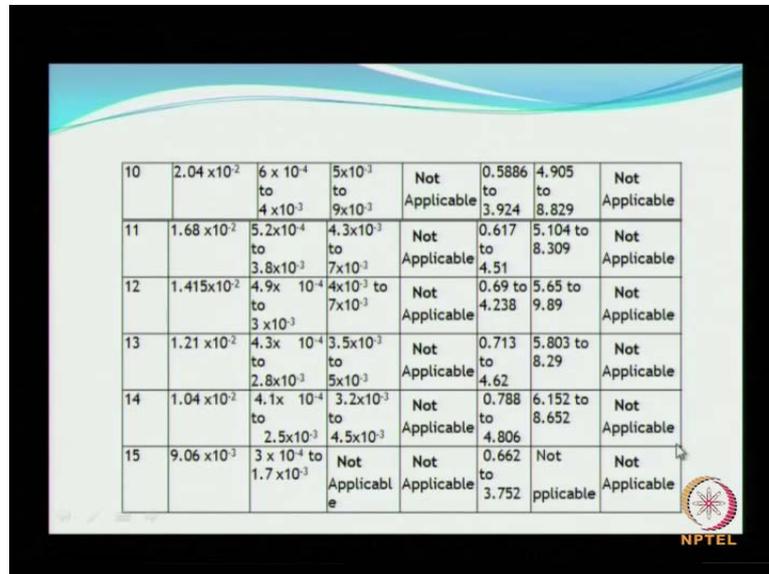
stokes wave theory:

T (sec)	$\frac{H}{gT^2}$	$\frac{H}{gT^2}$			H		
		Stokes 2 <sup>nd</sup> order	3 <sup>rd</sup> order	4 <sup>th</sup> order	2 <sup>nd</sup> order	3 <sup>rd</sup> order	4 <sup>th</sup> order
5	$8.15 \times 10^{-2}$	$10^{-3}$ to $7 \times 10^{-3}$	$8.5 \times 10^{-3}$ to $1.7 \times 10^{-2}$	$1.7 \times 10^{-3}$ to $3 \times 10^{-2}$	0.245 to 1.716	2.084 to 4.169	4.17 to 7.36
6	$5.66 \times 10^{-2}$	$10^{-3}$ to $6.5 \times 10^{-3}$	$8.5 \times 10^{-3}$ to $1.6 \times 10^{-2}$	$1.6 \times 10^{-2}$ to $3 \times 10^{-2}$	0.353 to 2.295	2.825 to 5.65	5.65 to 10.59
7	$4.16 \times 10^{-2}$	$9 \times 10^{-4}$ to $6 \times 10^{-3}$	$7 \times 10^{-3}$ to $1.2 \times 10^{-2}$	$1.2 \times 10^{-2}$ to $2.5 \times 10^{-2}$	0.4326 to 2.884	3.364 to 5.768	5.77 to 12.01
8	$3.18 \times 10^{-2}$	$8 \times 10^{-4}$ to $5 \times 10^{-3}$	$6 \times 10^{-3}$ to $0.8 \times 10^{-2}$	$0.8 \times 10^{-2}$ to $1.5 \times 10^{-2}$	0.4561 to 2.851	3.421 to 4.56	4.56 to 8.15
9	$2.516 \times 10^{-2}$	$7 \times 10^{-4}$ to $4.5 \times 10^{-3}$	$5.7 \times 10^{-3}$ to $0.5 \times 10^{-2}$	$0.5 \times 10^{-2}$ to $1 \times 10^{-2}$	0.556 to 3.575	4.529 to 3.97	3.97 to 7.94

And similarly, Stokes Wave Theory I have worked out for the different, starting from second, third, fourth Order Theory, calculated the H by g T square. This also is

calculated, this calculated then enter into the picture and enter into the nomogram and get the values of the wave height.

(Refer Slide Time: 54:56)



10	$2.04 \times 10^{-2}$	$6 \times 10^{-4}$ to $4 \times 10^{-3}$	$5 \times 10^{-3}$ to $9 \times 10^{-3}$	Not Applicable	0.5886 to 3.924	4.905 to 8.829	Not Applicable
11	$1.68 \times 10^{-2}$	$5.2 \times 10^{-4}$ to $3.8 \times 10^{-3}$	$4.3 \times 10^{-3}$ to $7 \times 10^{-3}$	Not Applicable	0.617 to 4.51	5.104 to 8.309	Not Applicable
12	$1.415 \times 10^{-2}$	$4.9 \times 10^{-4}$ to $3 \times 10^{-3}$	$4 \times 10^{-3}$ to $7 \times 10^{-3}$	Not Applicable	0.69 to 4.238	5.65 to 9.89	Not Applicable
13	$1.21 \times 10^{-2}$	$4.3 \times 10^{-4}$ to $2.8 \times 10^{-3}$	$3.5 \times 10^{-3}$ to $5 \times 10^{-3}$	Not Applicable	0.713 to 4.62	5.803 to 8.29	Not Applicable
14	$1.04 \times 10^{-2}$	$4.1 \times 10^{-4}$ to $2.5 \times 10^{-3}$	$3.2 \times 10^{-3}$ to $4.5 \times 10^{-3}$	Not Applicable	0.788 to 4.806	6.152 to 8.652	Not Applicable
15	$9.06 \times 10^{-3}$	$3 \times 10^{-4}$ to $1.7 \times 10^{-3}$	Not Applicable	Not Applicable	0.662 to 3.752	Not pplicable	Not Applicable

The range of wave height that could possibly cover the different Stokes area waves so with this, I think we have completed the Finite Amplitude Wave Theories. So, I hope this would be useful to you as a beginning so that based on this kind of information, you can just proceed further, by reading additional books.