

Advanced Strength of Materials
Prof. S. K. Maiti
Department of Mechanical Engineering
Indian Institute of Technology, Bombay

Lecture – 2

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STRESS IS A TENSOR			
Scalar	Mass	Density	Temp.
Vector	Forces	Moments	
Tensor			
Order / Rank			
1. Zero rank	—	Scalar	m, ρ, T
2. 1st	—	Vector	F_i, M_i
3. 2nd	—	Stress, Strain	$\sigma_{ij}, \epsilon_{ij}$
4. 4th	—	material property	
$\sigma_{ij} = E_{ijkl} \epsilon_{kl}$			

We have heard now that stress is a tensor I would like to now tell you what is a tensor? You have come across quantities like scalar you remember the examples are mass, density, temperature, they have simply magnitude. We have also come across vector examples are: forces, moments, in this case you have magnitude and also a direction. So, they have 1 more attribute in fact all quantities that engineer and physicist come across they are nothing but, tensor scalar, vector, they are also tensor.

So, tensor is a more general quantity. Example of tensor can be of different order, let us look into how we define the order or the rank of a tensor. In fact, this order or rank is defined by the number of attributes. We have 0 rank tensor and they are nothing but, scalar and examples I have already given you mass, density, temperature, and we can represent them by symbols like: mass let's say m , density by ρ and temperature by T .

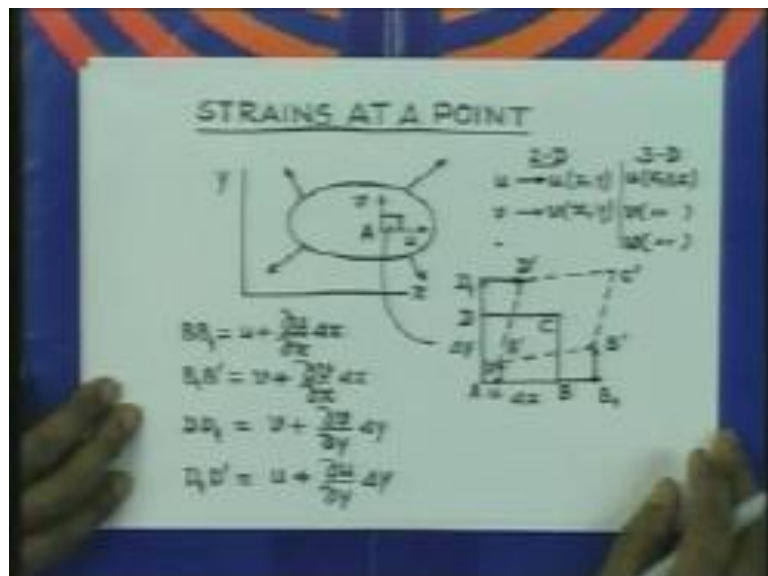
Then, we have first rank tensor they are nothing but, vector you can represent them by symbols like: F_i, M_i F_i in the case of 3 dimensions I can have 3 components: 1 in the x

then in the y and z direction therefore, there are 3 component. So, they have magnitude plus the directions and there is 1 index to indicate the first rank tensor.

Similarly, we have second rank tensor examples are: stress, strain. In the case of stress, we have seen that, we have magnitude of stress it has a direction of action and at the same time it has also a plane of association. So therefore, there is 1 more attribute and we generally represent the stress by symbols σ_{ij} , so there are 2 attribute therefore, there are 2 indices.

Similarly, in the case of strain we represent by symbol ϵ and there are also 2 indices use to indicate the rank of the tensor. In fact, in phase analysis and strength of materials we will have 1 more tensor which is fourth rank tensor and this is material property tensor. In fact, the stresses and strains are related and they are related by a relationship of the type, which you can write σ_{ij} is equal to some property tensors E_{ijkl} ϵ_{kl} . So, there are 4 indices and it is a fourth rank tensor. So, you should be now on be careful in trying to differentiate between stress and a vector. Now, we would like to configure how we can quantify the deformation at a point or define the strains at a point.

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In order to define the strain at a point, I would like to go in for considering a 2 dimensional case for convenience, but it can easily extended to 3 dimensions. So let us, consider a thin disk subjected to forces which are like this and the body is in equilibrium

and you can just deforming. Under the action of these forces each of the point in the body is going to have some moment in the x direction.

Let us represent by this moment by symbol u and it is also going to have some displacement under the action of the loading in the y direction which is equal to v . This value of the displacement is going to vary from point to point and therefore, this u displacement is certainly a function of position of the point. So, u is a function of x, y so also v displacement is a function of the position xy .

And in case of 3 dimension so this is the picture in 2 dimension, if we consider 3 dimension then you are going to have the displacements like: u, v and 1 displacement in the z direction which is generally indicated by w . So therefore, in the case of 3 dimensions you are going to have displacements u it is a function of x, y, z so also v which is going to be function of x, y, z . And then will have displacement w which is also function of x, y, z .

Now, will try to see that the summation at a point let us say point A or rather we would like to calculate the strain so the quantify the strain at the point a after the body has undergone a deformation. In order to do that, you consider a very small element like this and if we draw to a larger scale, let us draw to a larger scale this particular element so this is the element a let us say this is A B and then C and D.

Now, after deformation this particular element is going to change position and it's going to have a shape let us, say that displaced position of this element will indicate by this. So, this is the deformed position of the element therefore, A has shifted to A dash, B to B dash and this is shifted to C dash and this is shifted to D dash. Let us, consider that the distance between AB is Δx which is small distance between AD is Δy .

Now, point A has shifted by a distance u in the x direction and it has shifted by some distance v in the y direction therefore, this is nothing but u and this distance is nothing but v . So, these are the displacement of the point A since it is a continuous body you find that this function u and v they are continuous functions there is no jump in the function as we moved from point over these area of the body.

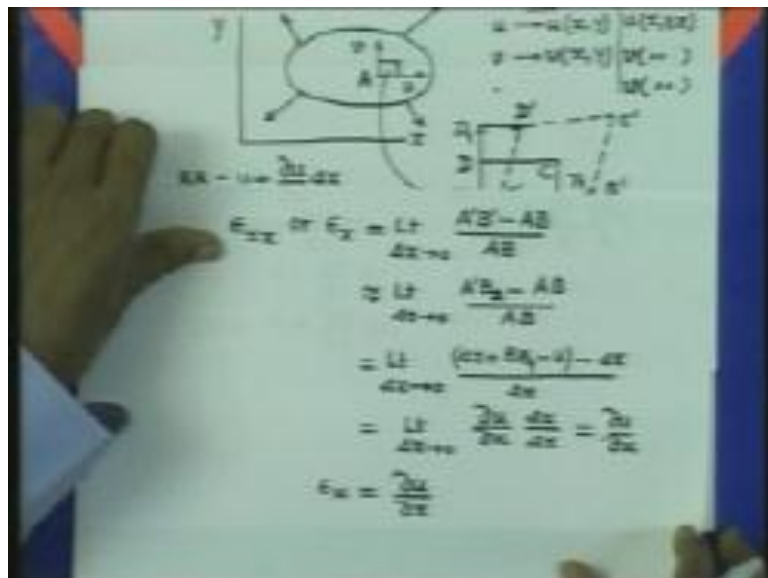
Now, if I want to calculate the displacement at the point B I can calculate it from the displacement of the point A, from the consideration that they 2 are at a distance Δx

which is very small. And you can consider expanding this u in scalar standards, and then you can write that the displacement of the point B in the x direction. Let us, say that is the displacement of the point in the x direction B to B1.

So this B to B1 you can write this thing as BB1 is equal to u of the point A plus rate of change of u with respect to x multiplied by Δx so that, is the displacement BB1. Similarly, if we calculate this distance B1 B dash it can be obtained from the displacement v of the point A. So this would be nothing but v plus rate of change of v with respect to x multiplied by Δx so that is again.

Similarly, if you consider the point D it has shifted to D1 in the y direction and then it has shifted to D dash in the x direction. So therefore, by similar configuration we can write that DD1 is nothing but $v \Delta y$ and this distance D1 D dash is equal to it is again obtainable from $u \Delta x$, therefore these are the displacements.

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Now, we would like to calculate the strains at the point A and the strains are defined like this strains at the point A in the x direction, which is represented by ϵ_{xx} or in sort ϵ_{ax} which is nothing but rate of change of deformation of ab in the x direction. So therefore, it is given by and also that rate of change of deformation has Δx tends to 0 that gives us the strain attribute.

So therefore, it is defined by Δx tending to 0 it is the deformed length is $A'B'$ dash original length is AB divided by the original length. Therefore, this is that is the rate of change of length and as Δx tends to 0 that gives us the strain at A in the x direction. In fact, this $A'B'$ dash since this angle they are least angle is let us, say this angle is γ_1 is very small we can approximate this distance $A'B'$ dash by this distance $A'B$.

So therefore, we can write that this limit is given by it is $A'B^2$ minus ab by AB . Now, we can write this thing very easily it was given by limit Δx tending to 0 and this $A'B^2$ is nothing but Δx plus BB_1 . So, Δx plus BB_1 minus the distance u minus u that is what is this minus AB which is nothing but Δx and this is Δx .

Now, BB_1 is nothing but, u plus Δu Δx del x therefore, will cancel u and then will be also haven calculation of this Δx . So this finally, will give us limit Δx tending to 0 Δu Δx Δx by Δx and therefore, this is Δu Δx . So, the strain in the x direction ϵ_x is nothing but partial derivative of u with respect to x that is the strain in the x direction.

Now, let us calculate the strain in the y direction so we will again confine our self to the deformation here and the strain in the y direction is defined as the deformation of AB in the y direction as this length Δy tend to 0. That is the strain in the y direction at point A.

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$$\begin{aligned}\epsilon_{yy} \text{ or } \epsilon_y &= \lim_{\Delta y \rightarrow 0} \frac{A'B' - AB}{AB} \\ &= \lim_{\Delta y \rightarrow 0} \frac{A'D_2 - AD}{AD} \\ &= \lim_{\Delta y \rightarrow 0} \frac{(\Delta y + v(D_2) - v) - \Delta y}{\Delta y} \\ \epsilon_y &= \lim_{\Delta y \rightarrow 0} \frac{\partial v}{\partial y} \frac{\Delta y}{\Delta y} = \frac{\partial v}{\partial y} \\ \epsilon_y &= \frac{\partial v}{\partial y}\end{aligned}$$

And it is given by ϵ_{yy} or in sort ϵ_y and that is equal to $\lim_{\Delta y \rightarrow 0} \frac{A'D - AD}{AD}$. Now, you can write this thing again as we can approximate $A'B$ again by consider some like this. We can draw this parallel to y axis and if we consider point to the D_2 since these angle. Let us, say this γ_2 is very very small we can now approximate $A'B$ by $A'D_2$.

So, it is $A'D_2 - AD$ by AD and since DD_1 is known and this displacement v is known DD_1 is nothing but $v + \Delta y \frac{\partial v}{\partial y}$. Then, you can write this thing as suppose this is I think you should write this thing approximate not in fact equal to and therefore, which is now substitute the value we get $\lim_{\Delta y \rightarrow 0}$ and this is $\Delta y + A' \frac{\partial v}{\partial y}$ this will be $DD_1 - v$ minus the original length is Δy divided by Δy . And this DD_1 is equal to $v + \Delta y \frac{\partial v}{\partial y}$.

So, this v and this v will cancel this Δy this Δy will cancel therefore, DD_1 only this part will be remaining. And on simplification what we get is that, $\epsilon_{yy} = \lim_{\Delta y \rightarrow 0} \frac{\Delta y + A' \frac{\partial v}{\partial y} - v}{\Delta y}$ and therefore, this is equal to $\frac{\partial v}{\partial y}$. So, the strain in the y direction is nothing but partial derivative of v in the y dash of slope of the v surface in the y direction.

Similarly, your strain in the x direction which is nothing but slope of the u surface in the x direction. Now, you would like to consider the shear strain shear strain is defined as the change in the angle between 2 orthogonal directions under the action of external loading. Here in, we had AD and AB and AD 2 orthogonal directions and upper deformation they have become $A'B$ and $A'D$. And therefore, had been a change in the angle with original 90 degree by the amount γ_1 and γ_2 .

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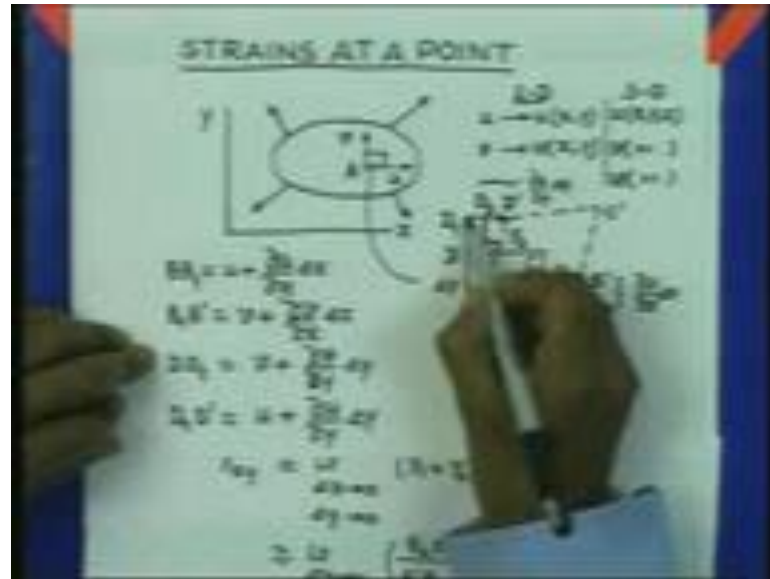
$$\begin{aligned}
 \gamma_{xy} &= \lim_{\substack{\Delta x \rightarrow 0 \\ \Delta y \rightarrow 0}} (\gamma_1 + \gamma_2) \\
 &\approx \lim_{\substack{\Delta x \rightarrow 0 \\ \Delta y \rightarrow 0}} \left(\frac{B_2 B'}{A B_2} + \frac{D_2 D'}{A D_2} \right) \\
 &= \lim_{\substack{\Delta x \rightarrow 0 \\ \Delta y \rightarrow 0}} \left(\frac{\frac{\partial v}{\partial x} \Delta x}{\Delta x + \frac{\partial u}{\partial x} \Delta x} + \frac{\frac{\partial v}{\partial y} \Delta y}{\Delta y + \frac{\partial u}{\partial y} \Delta y} \right) \\
 &= \frac{\frac{\partial v}{\partial x}}{1 + \frac{\partial u}{\partial x}} + \frac{\frac{\partial v}{\partial y}}{1 + \frac{\partial u}{\partial y}} = \frac{\partial v}{\partial x} + \frac{\partial v}{\partial y} \\
 \gamma_{xy} &= \frac{\partial v}{\partial x} + \frac{\partial v}{\partial y}
 \end{aligned}$$

And the shear strain in 2 dimension γ_{xy} is defined by limit Δx tending to 0 so as this length of the element is shrunk to 0 the limit γ_1 plus γ_2 that gives us the value of the shear strain. Now, we can calculate the shear strain as follows you can now write that this is approximately equal to limit Δx tending to 0 Δy tending to 0. That this angle I can approximate it by, that this is $B_2 B'$ divided by this term $A B_2$.

So this is nothing but $B_2 B'$ divided by $A B_2$ that is γ_1 plus we have here this γ_2 is nothing but, $D_2 D'$ divided by the length $A D_2$. So, this is what is the shear strain? Now, if we see that $B_2 B'$ this whole distance this whole distance $B_2 B'$ is nothing but, it is going to be v plus it is nothing but we have this distance is given by $B_1 B'$ minus $B_1 B_2$ which is nothing but, v therefore this distance is nothing but $\Delta v \Delta x \Delta x$.

So therefore, this distance is nothing but $\Delta v \Delta x \Delta x$, therefore this is this distance. And similarly, here this distance we can write this distance which is nothing but this total distance which is $D_1 D'$ minus this distance u therefore, it is $\Delta u \Delta y \Delta y$. So therefore, this is this distance is nothing but, $\Delta u \Delta y \Delta y$.

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So, if you consider that this distance is nothing but this total distance minus this 1 and which is therefore, is given by $\Delta u \Delta y \Delta y$. So therefore, if we substitute all this what we find now what we get now, is this that limit Δx tending to 0 Δy tending to 0 this is $\Delta v \Delta x \Delta x$ divided by this distance A dash B2 is nothing but Δx plus $\Delta u \Delta x \Delta x$ plus this D2 D dash is nothing but $\Delta u \Delta y \Delta y$ divided by Δy plus $\Delta v \Delta y$ plus Δy .

So, this distance you can also consider to be this distance as original length plus rate of change of v with respect to y multiplied by Δy . Similarly, this distance to be equal to original length plus rate of change of u with respect to x , so that gives us the length of the sides a dash B2 and A dash A2 A dash D2.

So now, if you simplify all this it gives us now $\Delta v \Delta x$ by 1 plus $\Delta u \Delta x$ and this is $\Delta u \Delta y$ 1 plus $\Delta v \Delta y$. In fact, the strains are very small compare to unity therefore, we can neglect these quantities. And therefore, we can write this thing for small deformation this is nothing but $\Delta v \Delta x$ plus $\Delta u \Delta y$. So, this quantities can be neglected and we get the strains finally, it will be given by γ_{xy} is equal to $\Delta u \Delta y$ plus $\Delta v \Delta x$. So, it is the sum of the cross derivatives that gives you the shear strain at the point A that will have been considered. Now, the this shear strain is known as Engineering Shear Strain.

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The image shows handwritten mathematical derivations on a piece of paper. The equations are as follows:

$$\epsilon_{xy} = \frac{1}{2} \gamma_{xy} = \frac{1}{2} \left(\frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right)$$

$$\gamma_1 = \gamma_2 = \frac{1}{2} \gamma_{xy}$$

$$\gamma_1 = \epsilon_{xy} \quad \gamma_2 = \epsilon_{yx}$$

$$\epsilon_{xy} = \epsilon_{yx}$$

$$\begin{bmatrix} \epsilon_{xx} & \epsilon_{xy} \\ \epsilon_{yx} & \epsilon_{yy} \end{bmatrix} \quad \begin{bmatrix} \epsilon_{11} & \epsilon_{12} \\ \epsilon_{21} & \epsilon_{22} \end{bmatrix}$$

↳ 2-D STRAIN TENSOR

$\epsilon_{ij}, i,j=1,2$

We define the tensorial shear strain like this, that epsilon xy is equal to half of engineering shear strain and therefore, this is equal to half delta u delta y plus delta v delta x. So, this is tensorial shear strain I want you to note 1 point carefully that when we are trying to consider the element, which we have started with you see here the this side rotates by gamma 1 and this side is rotating in the opposite direction anti clockwise direction.

So, this is rotate this is rotating in the clockwise direction this is rotate in the anti-clockwise direction and this gamma 1 and this is gamma 2 we generally take it to be gamma 1 equal to gamma 2 and that is equal to half of gamma xy. And this gamma 1 is the rotation of the x direction in the x axis in the y direction. So this gamma 1 is nothing but rotation of x axis in the y direction.

Similarly, this gamma 2 can be considered to be rotation of the y axis in the x direction and therefore, we represent them by symbol gamma 1 is nothing but epsilon xy rotation of x axis in the y direction and gamma 2 to be rotation of y axis in the x direction. So, these are the tensorial shear strain and in fact since gamma 1 equal to gamma 2 we have shear strains epsilon xy is equal to epsilon yx.

Finally therefore, got a point we have distance like epsilon x epsilon xy and epsilon yx epsilon y. So, these are the strain associated with the x directions; these are the strains associated with the y direction this is known as 2-D Strain Tensor. And in the initial

notation we can write this thing as ϵ_{11} ϵ_{12} ϵ_{21} ϵ_{22} . So, in short we indicate this thing as, ϵ_{ij} with the implicit assumption that i takes up value 1 to 2 and j takes up value 1 to 2. So therefore, this is the 2 dimensional strain tensor ϵ_{ij} .

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3-D

$$\begin{bmatrix} \epsilon_{xx} & \epsilon_{xy} & \epsilon_{xz} \\ \epsilon_{yx} & \epsilon_{yy} & \epsilon_{yz} \\ \epsilon_{zx} & \epsilon_{zy} & \epsilon_{zz} \end{bmatrix} \rightarrow \epsilon_{ij} \quad \begin{matrix} i=1,2,3 \\ j=1,2,3 \end{matrix}$$

$$\epsilon_{xy} = \epsilon_{yx} \quad \epsilon_{xz} = \epsilon_{zx} \quad \epsilon_{yz} = \epsilon_{zy}$$

SYMMETRIC TENSOR

$$\begin{bmatrix} \epsilon_{xx} & \gamma_{xy} & \gamma_{xz} \\ \gamma_{yx} & \epsilon_{yy} & \gamma_{yz} \\ \gamma_{zx} & \gamma_{zy} & \epsilon_{zz} \end{bmatrix} \rightarrow \text{ENGL. STRAIN TENSOR}$$

$\sigma_{ij} \quad \epsilon_{ij}$

We can extend this consideration to 3 dimensions and in 3 dimensions we will have the strains like ϵ_{xx} x axis rotating in the y direction, ϵ_{xz} x axis rotating in the z direction similarly, will have ϵ_{yy} y axis rotating in the x direction direct strain and then we have ϵ_{yz} y axis rotating in the z direction.

Similarly, ϵ_{zx} ϵ_{zy} and ϵ_{zz} , so these are the 9 components of strain tensor in 3 dimensions and in initial notation. We can again write, and then you can also write in sort ϵ_{ij} with the implicit assumption that i takes up value 1 to 3 j takes up value 1 to 3. So therefore, it is a 3 dimensional stress tensor and you also note, that we have the condition that ϵ_{xy} is equal to ϵ_{yx} similarly, ϵ_{xz} is equal to ϵ_{zx} and then ϵ_{yz} is equal to ϵ_{zy} .

So therefore, these shear strains components are equal and therefore, we have this equal to this; this component is equal to this component and again this component is equal to this component. So therefore, the strain tensor is also a symmetric tensor. Now, we have got engineering strain tensor which is like this in the case of 3 dimension it will be

epsilon x and it will be gamma xy gamma xz and this is gamma yx epsilon y gamma yz gamma zx gamma zy epsilon z.

So, this is engineering strain tensor. Herein the difference is that, the shear strains are double of the tensor shear strain. And this is also symmetric; the tensorial definition is very important because, both stress and strain we have symbol for stress is sigma ij, symbol for strain is epsilon ij. Now, this is going to be helpful in engineering to this tensor definition of shear strain will be helpful. Because, whatever relations we write for sigma ij can be directly adopted for epsilon ij, so this tensor definition is that way very useful.

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The slide titled "SIGN CONVENTION" shows the following content:

$$\begin{bmatrix} \epsilon_x & \epsilon_{xy} \\ \epsilon_{yx} & \epsilon_y \end{bmatrix} = \begin{bmatrix} 10 \times 10^{-6} & -10 \times 10^{-6} \\ -10 \times 10^{-6} & 5 \times 10^{-6} \end{bmatrix} \text{ mm/mm}$$

Below the matrix, the components are listed:

- $\epsilon_x = 10 \mu\epsilon$
- $\epsilon_y = 5 \mu\epsilon$
- $\epsilon_{xy} = -10 \mu\epsilon$
- $\epsilon_{yx} = +10 \mu\epsilon$

Two diagrams illustrate the sign convention for shear strain. The first diagram shows a square element with a dashed line representing the original shape and a solid line representing the deformed shape, with a negative angle indicating a decrease in the angle. The second diagram shows a similar square element with a positive angle indicating an increase in the angle.

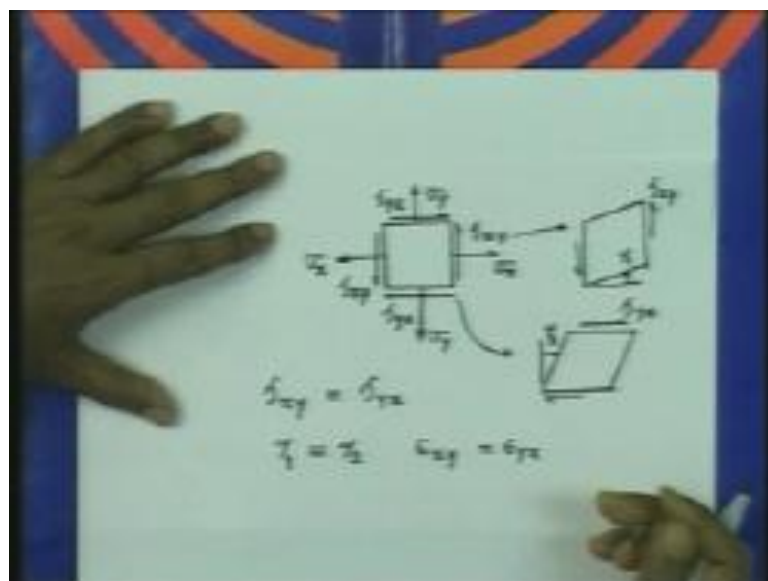
Now, convention for positive strain and negative strain I would like to give sign convention. If we have let us say, that the strains that this strain at a point is given by value like 10 10 to the power minus meter per meter and this is let us say 10 10 to the power minus 6 and this is 10 10 to the power minus 6 and this will 5 into ten to the power minus 6 and these are strains which the unit like meter per meter.

So, there dimensions so meter per meter now what it means, that this strain is positive epsilon x is equal to 10 you sometime write it as micro strain. So this strain micro strain it is positive therefore, it is causing elongation, so also epsilon y is equal to 5 micro strain it is causing elongation in the y direction whereas, this strain is causing epsilon xy is equal to minus 10 micro strain it is causing a change in the angle.

So, this must be understood that if we have an element like this; this particular definition is such that this angle in the first quadrant is going to increase. So this, is really the angle has now increased therefore, this means that the angle is increasing that deformation is different. If it is positive let us say, ϵ_{xy} is equal to let us say 15 micro strain then the deformation would be like this the deformation will give rise to the angle of this sort.

In the first quadrant therefore, this is going to be the final. So, please note that this ϵ_{xy} when it is negative it is going to be increasing the angle over 90 degree and this is positive means it is going to indicate. So, these are the conventions which are followed for strains. You must be wondering why this ϵ_{xy} and ϵ_{yx} they are same there is some physical reason behind back.

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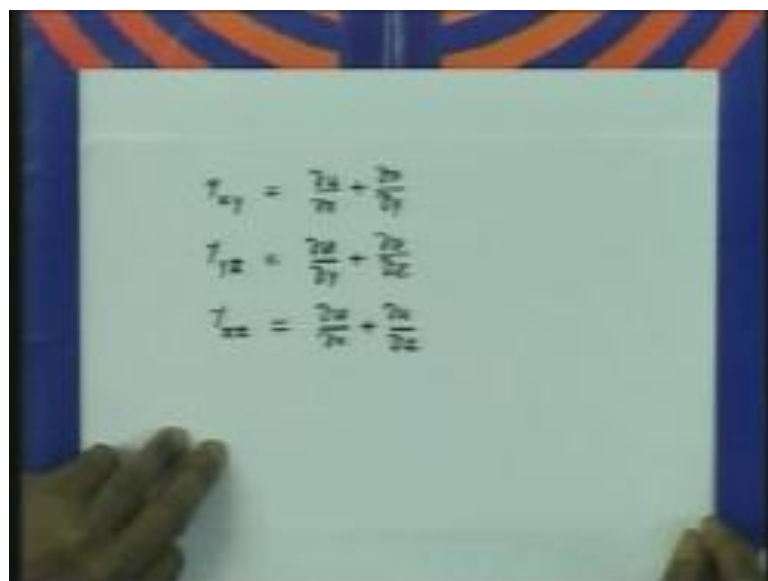
Look at the stresses, so if you consider the element to be subjected to stresses σ_x , σ_y and the shear stresses. So this is τ_{xy} , this is τ_{yx} , this is σ_x , this is τ_{xy} , τ_{yx} and then of course, we have also the σ_y stress here. This τ_{xy} , this shear is trying to rotate the element like this, so it this shear causes the rotation of the element like this. So, in fact that is responsible for this rotation what we indicated as γ_1 and these 2 components τ_{yx} they are responsible for the rotation of the element like this. So, in fact these components are responsible for rotating the elements like this. So, it is responsible for the rotation of the y axis in the x direction.

So therefore, you see that this rotation is nothing but $\gamma/2$ so these are the stresses τ_{xy} and these are the stresses τ_{yx} the element is square in shape. And therefore, see the rotation due to these stresses will have some magnitude it will depend on the moment that is produced by them. Similarly, the rotation $\gamma/2$ is going to be caused by these stresses and moment produced by them, therefore it will depend on the magnitude.

Since the dimension is the same which is square therefore, these 2 moments are going to depend on the magnitude of these 2 stresses and you can see that stress is a symmetric tensor that there by indicating the τ_{xy} is equal to τ_{yx} . Therefore, the moment produced by these 2 pairs is same as the moment produced by this 2 plus. And hence there is nothing to see that these 2 rotations are going to be different; they must be the same.

And hence we have since these stress tensor is symmetric we are going to see that, this γ_{xy} is equal to γ_{yx} ; rotation of the x axis in the y direction is equal to rotation of y axis in the x direction. So, that is why the 2 angles or shear strains are considered to be the same. So, we have now been able to quantify the stresses and strains at a point.

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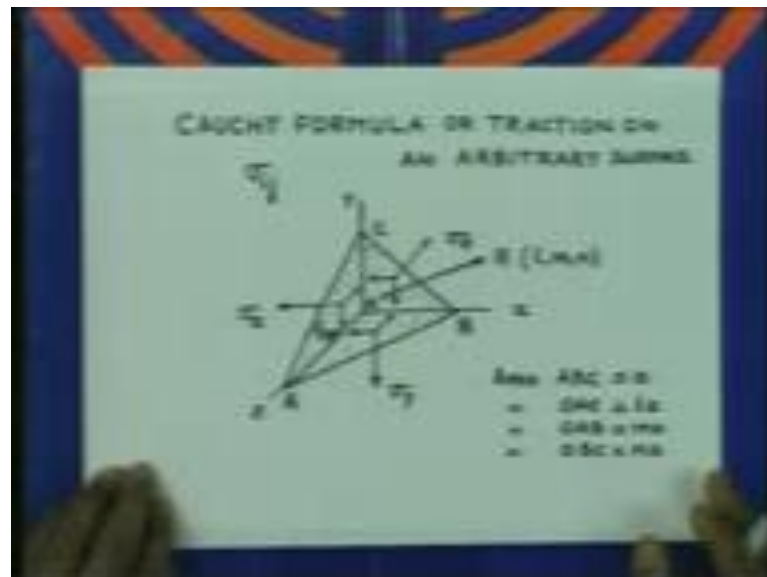
$$\begin{aligned}\tau_{xy} &= \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \\ \tau_{yz} &= \frac{\partial u}{\partial z} + \frac{\partial v}{\partial y} \\ \tau_{zx} &= \frac{\partial u}{\partial x} + \frac{\partial v}{\partial z}\end{aligned}$$

It is also important to note, that the shear strain expressions are that γ_{xy} is equal to $\delta u / \delta x$ plus $\delta v / \delta y$. So herein, these 2 directions and the definition of

shear strain involves the 2 displacement involved these 2 displacement have in that 2 Cartesian directions that we have here; by similar argument you can say that γ_{yz} shear strain is nothing but will have displacement v and w and there will be cross derivative.

Therefore, it is $\frac{\partial w}{\partial y} + \frac{\partial v}{\partial z}$ and γ_{xz} is equal to $\frac{\partial w}{\partial x} + \frac{\partial u}{\partial z}$, so these are the shear strain in 3 dimensions. Now, we would like to consider, if the stresses are given at a point by σ_{ij} we are interested in trying to see what is going to be the traction on any arbitrary plane.

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Now, this stresses you can show it to be like this let us consider the Cartesian direction as it is and then you try to consider an arbitrary surface. Let us, consider an arbitrary surface like this. So let us, say that this is my origin O this is let us, say that this plane is $A B C$. Now, the stresses at the point O is given by σ_{ij} so we can represent the stresses by showing the stress component on the phases OCA , OAB and OBC .

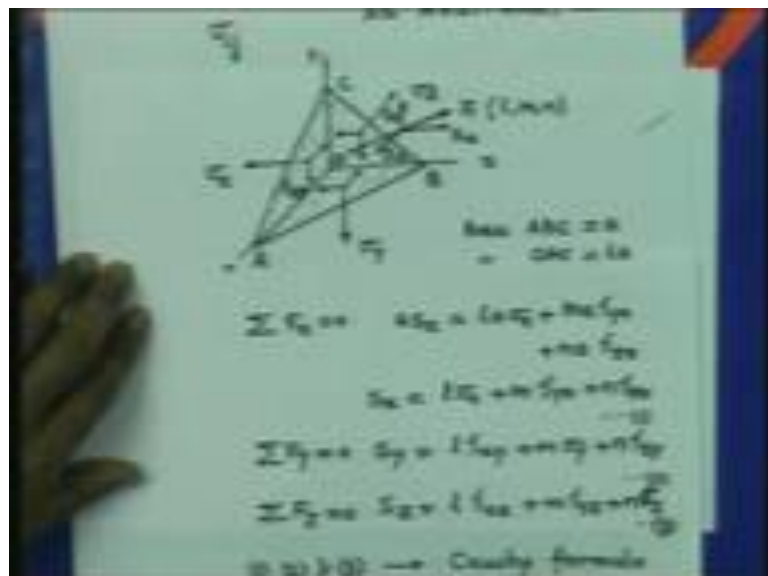
So these are going to be like this that will have the stress components here it is σ_x and then its negative phase so if you consider that this is your x direction this phase is negative similarly, let us axis xyz . So, this is σ_x and now the shear stress is going to be directed like this; this is τ_{xy} and the other component is going to be like this; this is τ_{xz} .

Similarly, will have the components like this this is σ_y and the other components are shear stress components are like this; this side we are going to have the normal components like this σ_z and the shear stress components are going to be this and this. Now, we are interested in finding out the traction on this area ABC arising out of the stresses at the point O, so this is the area.

Let us, say area ABC is equal to a and let us also consider that the outer normal to the plane ABC is directed like this; this is the outer normal. This outer normal would have some direction cosine its inclination with the x y z axis will cosine of the angle will indicate by the direction cosines L , M and N these are the direction cosines. Then, in that case it is clear that area OAC is nothing but L into a area OAB is nothing but M into a similarly, area OBC is equal to N into a , so these are strength of the geometry.

Now, we are interested in considering the traction on this phase ABC. So, if we consider that the traction force per unit area on this plane ABC in the x direction it will have 3 components. Suppose, we will have 3 components 1 in the x direction will represent that by S_x similarly, the component in the y direction will represent by S_y and the component in the z direction this is S_z . So, this is S_z , this is S_y so these are 3 components.

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Now, if we try to consider the equilibrium in the x direction f_x equal to 0 the force in the x direction on this area is a into s_x acting in the positive x direction. And now, we are going to have forces in the negative x direction arising out of σ_x acting on this area

which is nothing but L into a into σ_x . Then, we will have forces coming from this stress which is nothing but τ_{zx} and also this is τ_{yx} .

So therefore, this force arising out of this component acting on an area is nothing but N into a multiplied by τ_{yx} plus this component which is acting on an area is equal to n into a it is τ_{zx} . So therefore, this is what is the equilibrium equation in the x direction and we can now write these simplified it is S_x is equal to N into σ_x plus N into τ_{yx} plus N into τ_{zx} . I can write these equilibrium equations in the y direction.

So, that will give me by similar consideration it is very simple you can find out the force in this y direction, it is force in this y direction is given by this traction. And it is balanced by the traction or forces coming up due to this stress, it is coming out due to the shear stress this 1 and the shear stress this 1 . And if you simplify it will look like this it is going to be equal to l into τ_{xy} plus m into σ_y plus n into τ_{zy} .

Similarly, if we consider the equilibrium in the z direction it will give you S_z is equal to l into τ_{xz} plus m into τ_{yz} plus n into σ_z . So, these are the 3 relations this 1, 2 and 3 these are known as Cauchy formula. So, to calculate the traction on any arbitrary plane if you know the direction cosines of the plane and the stresses at the point p then you can calculate the tractions by this formula.

(Refer Slide Time: 53:06)

Handwritten notes on a green background showing the derivation of Cauchy's formula for traction components. The text is as follows:

$$\sum F_x = 0 \quad \sigma_x = l^2 \sigma_x + m^2 \tau_{xy} + n^2 \tau_{xz}$$

$$\sum F_y = 0 \quad \sigma_y = l^2 \tau_{xy} + m^2 \sigma_y + n^2 \tau_{zy}$$

$$\sum F_z = 0 \quad \sigma_z = l^2 \tau_{xz} + m^2 \tau_{yz} + n^2 \sigma_z$$

(1.20) > (3) \rightarrow Cauchy formula

$$T_x = l_1 \sigma_x + l_2 \tau_{xy} + l_3 \tau_{xz}$$

$$T_y = l_1 \tau_{xy} + l_2 \sigma_y + l_3 \tau_{yz}$$

$$T_z = l_1 \tau_{xz} + l_2 \tau_{yz} + l_3 \sigma_z$$

$$T_j = l_i \sigma_{ij}$$

$$T_j = l_i \sigma_{ij} \quad i=1,2,3 \quad j=1,2,3$$

And in initial notations, if you make use of the initial notations we can write that s_1 is equal to $l_1 \sigma_{11} + l_2 \sigma_{21} + l_3 \sigma_{31}$ S_2 is equal to $l_1 \sigma_{12} + l_2 \sigma_{22} + l_3 \sigma_{32}$ and S_3 is equal to $l_1 \sigma_{13} + l_2 \sigma_{23} + l_3 \sigma_{33}$. In short, in tensor notation these 3 relations can be written like this is S_j equal to $l_j \sigma_{ji}$ wherein, S_j is equal to $l_i \sigma_{ij}$. So, this is an initial notation and with the implication that i takes up value 1, 2, 3, so also j takes up value 1, 2, 3 so we get the traction 3 traction.