

Mechanics of Material
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Stresses and deflection in homogeneous beams loaded about one principal axis
Lecture – 54
Variation of axial stress

Now, σ_{xx} is given by $M z$ by $I z z$ minus into y minus y naught.

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Let us assume the cross section to be rectangular

$$\sigma_{xx} = \frac{-M_z (y - y_0)}{I_{zz}}$$

$$y_0 = 0 \quad I_{zz} = \frac{bh^3}{12}$$

$$\sigma_{xx} = \frac{-M_z (L - x^2)}{\frac{bh^3}{12}} y$$

$$\sigma_{xx}^{\max} = \frac{M_z^{\max}}{2} \left(\frac{h}{2} \right)$$

$$\sigma_{xx}^{\max} = \frac{M_z^{\max}}{12} \cdot \frac{6}{bh^2}$$

$$I_{zz} = \int_{-h/2}^{h/2} \int_{-b/2}^{b/2} y^2 dy dz$$

$$I_{zz} = \int_{-h/2}^{h/2} \frac{y^3}{3} \Big|_{-b/2}^{b/2} dz = \frac{b^3}{12} \int_{-h/2}^{h/2} dz = \frac{b^3 h}{12}$$

$$\sigma_{xx}^{\max} = \frac{3M_z}{4b} \left(\frac{h}{2} \right)^2 = \sigma_{yy} \left(\frac{3}{4} \right) \left(\frac{h}{2} \right)^2$$

$$M_z = \int_{-h/2}^{h/2} \sigma_{yy} dz = \sigma_{yy} b$$

$\frac{d}{h}$ for beams > 10
 $\sigma_{xx}^{\max} \sim 100$ times σ_{yy}

$M z$ we have found before, $I z z$ for a given cross section let us assume, the cross section to be rectangular with the following dimensions this is y , this is z , this is b , this is h . Now, y naught should be the center of the cross section which means this is the C G cross section, you know that for the rectangular section is C G is set of the width and of the height of the cross section.

Hence, this distance is going to be b by 2 and this distance is going to be h by 2 . Now, I am locating I am measuring y from the center of the cross section. So, y naught is 0 , from here y naught is 0 because the original cross section coincide with the center of the cross section.

Now, what is $I z z$; now $I z z$ for this cross section is integral y square dy minus h by 2 to h by 2 into dz minus b by 2 to b by 2 . So, in z I am integrating from this point to this

point, the width is the same. So, it does not matter how I integrate, $y^2 dy$ is from this end to this end I have to integrate, $y^2 dy$ I have to integrate from this end to this end. So, it is from $-h/2$ to $+h/2$.

The width is remaining constant, so it does not matter how I, the art of the integration is, what the art of the integration is. So, this integrates from $-b/2$ to $+b/2$ along the y direction. So, this will be $\int_{-b/2}^{b/2} y^2 dy$; $y^2 dy$ from $-h/2$ to $+h/2$ into dz which will be $h^3/12$ into $\int_{-b/2}^{b/2} dz$ which will be nothing but $h^3/12$.

So, I_{zz} is, from here you got I_{zz} to be $b h^3/12$. So, from the previous derivation you got M_z to be $W L x - x^2$. So, σ_x is now given by $W/2 L x - x^2$ divided by $b h^3/12$ into y .

Now, you find that across depth to the cross section the σ_x varies linearly. So, across the depth of the cross section, the variation of σ_x is linear; with what happens at the top, at the top y is positive bending moment is also positive. So, the stress has to be negative, so that is why it is in compression acting like this and the bottom surface y is negative; y is negative at the bottom; at this end y is negative and hence this stress has to be positive because the bending moment is positive, so it varies like this.

Now, this is just the σ_x variation with respect to y and this point is the C G of the cross section this is the C G of the cross section, but what are we interested in? We are interested in where the maximum σ_x occurs and what is the value of this maximum σ_x .

Maximum σ_x will occur, where the bending moment M_z is maximum divided by $b h^3/12$ and you know that since it is very linearly the extreme points is where the maximum occurs; from the σ_x is varying linearly extreme points is where the maximum stresses occur. So, it will be at $h/2$.

We, just now saw that $M_G \max$ is given by $W L^2/8$ and so this expression becomes into $6/b h^2$, $\sigma_x \max$. Now, let us do an analysis now; I will rewrite $\sigma_x \max$ as $W/4 b$ into L^2/h^2 . Now, the W is force per unit length, that is W by definition was $\int_{-b/2}^{b/2} y dy$ into dz from $-b/2$ to $+b/2$, that is $\int_{-b/2}^{b/2} y dy$ into b was W , so W/b is $\int_{-b/2}^{b/2} y dy$.

So, this happens to be σ_y into $\frac{3}{4}$ into l by h the whole square. Typically, for a beam l by h for beams would be greater than 10; for those beams only what are we are doing is valid. If the beam is too short or too deep what are we are doing is not valid.

So, then you find that σ_x max is roughly standard time σ_y . So, even though the applied stress is σ_y , the stress resultant the stress that is (Refer Time: 07:45) in the beam because of the application of σ_y stress is 100 times more than the applied stress, that is why this important to compute what the stresses. It is not equal; just enough to find what is the stress that is applied the (Refer Time: 08:03) stress can be much more than the applied stress; this is the thing we will see repeatedly in this course now, so this is σ_x .