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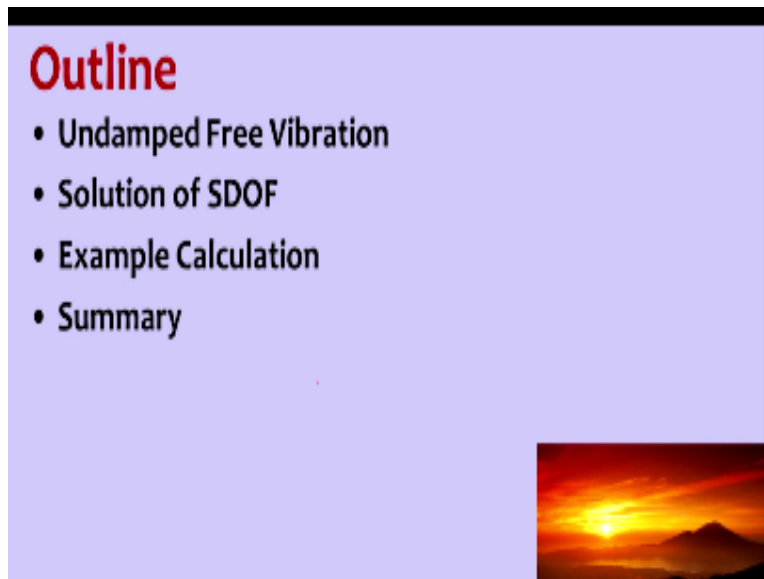
**Structural Dynamics
Week 1: Module 06**

Solution of Undamped Free vibration

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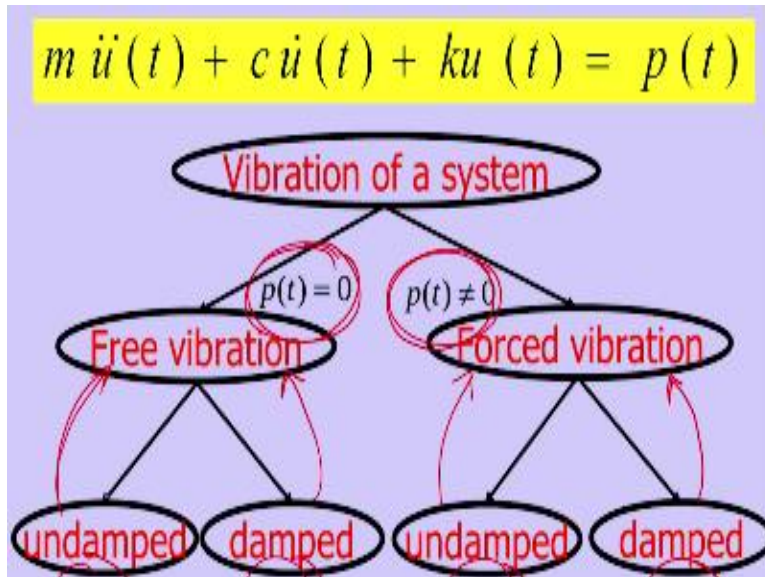
Now in this module we will discuss solution of undamped free vibration so this undamped free vibration means.

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Where force is 0 so there is no externally applied force and then when it will call as undamped so that means damping is 0 so undamped free vibrations. So the outline of this is discussion out the module is undamped free vibration and then how to find the solution of the undamped free vibration and then one example problem will solved in this module.

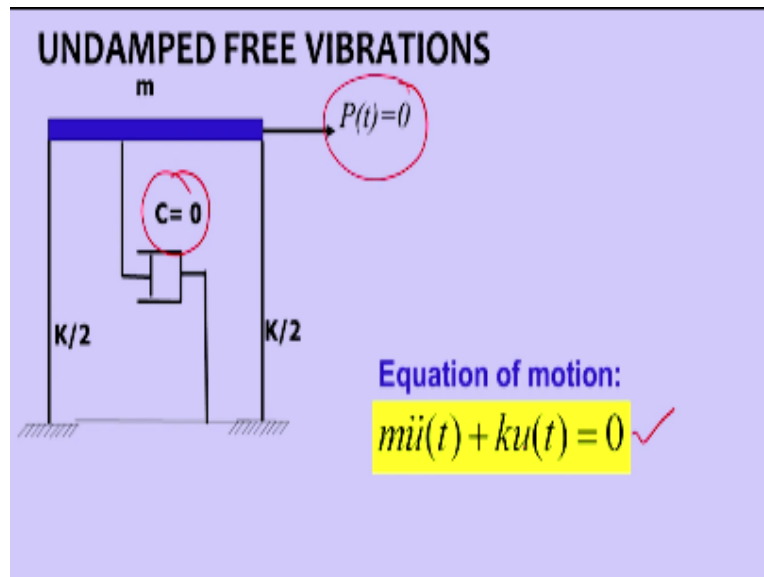
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So this is a general dynamic equilibrium equation or equation of motion we can call it as, so the equation of motion of a vibrating system so the free vibration and forced vibration free vibration means where force is equal to 0 and forced vibration means force is present it is not 0 and in that 4 cases can be formed one is undamped free vibration so undamped free vibration damped free vibration undamped forced vibration and damped forced vibration.

So undamped free vibration means damping is 0 force is 0 damped free vibration means damping is present but force is 0 undamped forced vibration means damping is 0 but force is present damped forced vibration means damping is present and also force is present so this four cases will discuss out of it we are discussing this first case that is undamped free vibration. So how does equation of motion look like?

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So it looks like this? $m\ddot{u} + ku = 0$ where damping term is removed from this one and force term is removed from this one as you can see from this picture so force is not there and damping is not there so when force is not there how the system is oscillating, so system will oscillate with initial disturbance so that initial disturbance can be in the form of initial displacement like this initial displacement oscillates or initial velocity which is applied in the form of impulse or velocity.

So this is initial velocity or both initial displacements along with initial velocity like that so if we supply initial disturbances or the initial displacement or the initial velocity of both so systems will set to oscillate. Then how do we define the solution of this equation of equilibrium that is $m\ddot{u} + ku = 0$ so for that we will discuss here.

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$$m\ddot{U} + kU = 0$$

Let, $U = e^{st}$ is a solution

$$\dot{U} = se^{st}$$
$$\ddot{U} = s^2 e^{st}$$
$$ms^2 e^{st} + ke^{st} = 0$$
$$(ms^2 + k)e^{st} = 0$$

$e^{st} \neq 0$ as it is solution

Hence, $(ms^2 + k) = 0$

trivial sol

So how to form a solution so this is $m\ddot{u} + ku = 0$ so this is the basic equation of motion so we want solution we are interested in the solution which is u so u we assuming it has $u = e^{st}$ is a possible solution, so this we are assuming that e^{st} is possible solution, so this is the usual solution taken for simple harmonic motion problems. So if $u = e^{st}$ is the solution then its derivative that is \dot{u} that is velocity is so if you differentiate this one we get $s \times e^{st}$ and then if you double differentiate the same then we get $s^2 e^{st}$.

So we substitute u and u double dot in the original equation so if we substitute that as you can see here so u double dot as $s^2 e^{st}$ we are substituted and then u as e^{st} we are substituted so that is $m u$ double dot + $ku = 0$ so in this we can take e^{st} is common and $m s^2 + k \times e^{st} = 0$, so from this equation we can clearly see that for this equation to become 0 either first term should be 0 or second term should be 0 mathematically speaking.

So if second term is 0 so that means what second term is nothing but the assumed solution so we cannot make this second term 0 so if second term is 0 that will become a trivial solution, so for non trivial solution this e^{st} which is a assumed displacement response it cannot be 0 so that means what if e^{st} cannot be 0 so then that means bracket term should be equal to 0 that is $ms^2 + k = 0$ so

this bracket term should be equal to 0. So if this bracket term is 0 then what is going to happen let us see this next slide yeah.

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$s = \sqrt{\frac{-k}{m}}$
 $s_{1,2} = \pm i \sqrt{\frac{-k}{m}}$
 $\omega_n = \sqrt{\frac{k}{m}}$
 $s_{1,2} = \pm i \omega_n$

$m s^2 + k = 0$
 $s^2 = \frac{-k}{m}$
 $s = \sqrt{\frac{-k}{m}} = \pm i \sqrt{\frac{k}{m}}$
 $s_1 = +$
 $s_2 = -$ natural frequency of the systems

So if the bracket term is 0 so which is $m s^2 + k = 0$ so it means $s^2 = -k/m$ then s will become $\sqrt{-k/m}$ so this $\sqrt{-k/m}$ if we take out this $-\sqrt{-1} \sqrt{k/m}$ to separately with the symbol plus or minus because it is in the inside the \sqrt it is coming out so then we can have 2 answers top this one s_1 and s_2 . So that s_1 is with $+$ symbol and s_2 is with $-$ symbol and then this $\sqrt{k/m}$ can be represented by as ω_n which is call natural frequency of the system.

So then that gives as 2 \sqrt to this equation that is $s_1 = + i \omega_n$ and $s_2 = - i \omega_n$ so this 2 values are there s_1 and s_2 , so now upon substitute in this s_1 and s_2 in the back in the equations.

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$$U(t) = A_1 e^{+i\omega_n t} + A_2 e^{-i\omega_n t} \quad \leftarrow \text{LDE}$$

Also, $\cos x = \frac{e^{ix} + e^{-ix}}{2}$ and $\sin x = \frac{e^{ix} - e^{-ix}}{2}$

$$U(t) = C_1 \cos \omega_n t + C_2 \sin \omega_n t \quad \checkmark \text{ @ } t=0, U(0)=0$$

Initial Condition, at $t=0$

$U = U(0)$ and $\dot{U} = \dot{U}(0)$

$C_1 = U(0)$

$\dot{U}(t) = -\omega_n C_1 \times 0 + \omega_n C_2 \times 1$

$C_2 = \frac{\dot{U}(0)}{\omega_n}$

$$U(t) = U(0) \cos \omega_n t + \frac{\dot{U}(0)}{\omega_n} \sin \omega_n t$$

Handwritten notes on the right side of the slide:
 $U(0) = 0$
 $U(0) = C_1 \times \cos(\omega_n \times 0) + C_2 \times \sin(\omega_n \times 0)$
 $U(t) = C_1 \times \cos \omega_n t + C_2 \times \sin \omega_n t$
 $U(0) = -C_1 \times \omega_n \times \sin(\omega_n \times 0) + C_2 \times \omega_n \times \cos(\omega_n \times 0)$
 $U(0) = C_2 \times \omega_n \Rightarrow$

So we get this so $u(t) = A_1 e^{+i\omega_n t} + A_2 e^{-i\omega_n t}$, so this is the form of solution of linear differential equations with constant coefficients. Linear differential equations with constant coefficients are complicated, so we need a simplified solution. There we use trigonometric functions which can be represented in the form of exponential terms. $\cos x$ can be represented as $(e^{ix} + e^{-ix})/2$ and $\sin x$ can be represented as $(e^{ix} - e^{-ix})/2i$. So $\cos x$ and $\sin x$ along with these exponential terms can be rearranged, and this solution can be written in the form of $C_1 \cos \omega_n t + C_2 \sin \omega_n t$.

So in this equation, we have two constants C_1 and C_2 . So we need to get rid of these constants C_1 and C_2 for getting a complete solution. So C_1 and C_2 can be calculated or can be found out by using initial conditions. We have two initial conditions: at time $t = 0$, what is the status of displacement? At time $t = 0$, what is the status of velocity? So at time $t = 0$, what is displacement and at time $t = 0$, what is velocity? So this is something like this: so time $t = 0$ is the first initial condition and \dot{u} at $t = 0$ is the second initial condition.

So if you substitute these initial conditions in the equation, we will get C_1 and C_2 . It's something like if you substitute a u_0 so let initial condition that u displacement at time $t = 0$ is u_0 , so if you

substitute this so what is going to happen is time $t = 0$ $\cos 0$ and $\sin 0$ so $\sin 0 \cos 0$ is 1. So if $\sin 0$ let me write here $u(0) = C_1 \times \cos \omega n \times 0 + C_2 \sin \omega n \times 0$, so this gives as $\sin 0$ is 0 so this entire term goes to 0 and $\cos 0$ is 1 so that gives as $C_1 = u_0$ so this c_1 as constant is evaluated.

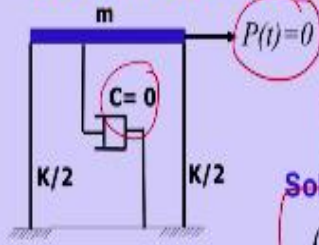
Now we need c_2 so C_2 is having a term as we like in second initial condition is having $u \dot{}$ but we have solution u so we need to do the differentiation of this solution so $u \dot{}$ of $t =$ single term will remain like that then differentiation of \cos will be $-\omega n \sin \omega n t$ + differentiation of the second term that is c_2 will remain like that $\omega n \cos \omega n t$ now we need to apply the second initial condition in this one.

So the second initial condition is $u \dot{}$ so $u \dot{}$ = 0 if here using then c like ω or $C_1 \times \omega$ and with $-$ symbol and then $\sin \omega n 0 + C_2 \omega n \cos \omega n \times 0$ so again here you can see $\sin 0$ is 0 it is entire term goes to 0 $\cos 0$ is 1 so that means what $u \dot{}$ = $C_2 \omega n$ this implies $C_2 = u \dot{}/ \omega n$ so we got c_1 constant and we got C_2 constant and C_1 constant so if you substitute this c_1 and C_2 constants in the original equation that is a solution.

So we get this so from this you can see this so the solution of undamped free vibration so in this u_0 is the initial displacement and $u \dot{}$ is a initial velocity so from this equation we can clearly see that if we supply initial displacement or initial velocity or both then only undamped free vibration then only there will be oscillation in the system. Otherwise if initial displacement is 0 or an initial velocity is 0 system cannot oscillate system will be in the static position only.

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UNDAMPED FREE VIBRATIONS



$P(t)=0$

$C=0$

$k/2$ $k/2$

m

Equation of motion:

$$m\ddot{u}(t) + ku(t) = 0$$
$$\ddot{u}(t) + \omega_n^2 u(t) = 0$$

$\omega_n = \sqrt{\frac{k}{m}}$

Solution:

$$u(t) = u(0)\cos\omega_n t + \frac{\dot{u}(0)}{\omega_n}\sin\omega_n t$$

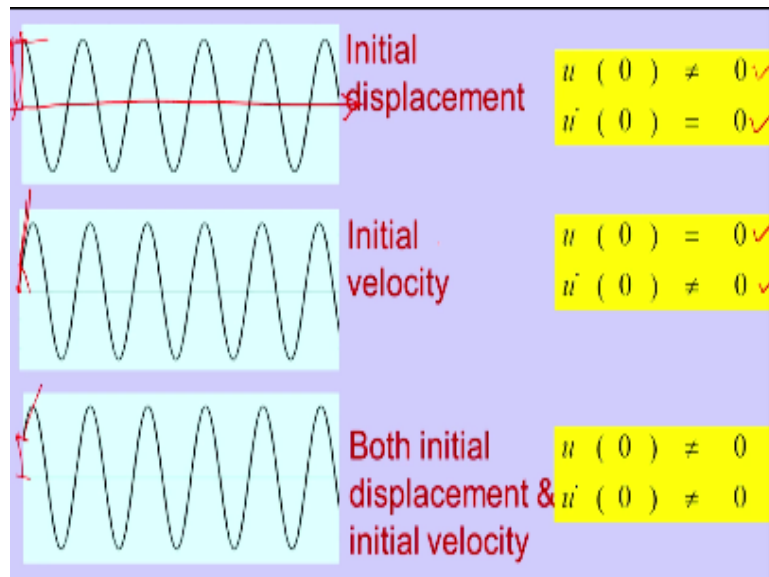
$u(0)$: Initial displacement

$\dot{u}(0)$: Initial velocity

$\omega_n = \sqrt{\frac{k}{m}}$ Natural frequency

So now this slide shows the overall summary of undamped free vibration so this is system where damping is 0 and force is 0 then equation of equilibrium is $m\ddot{u} + k_u = 0$ if we divide this equation by m so that will give us $\ddot{u} + k/m = 0$ we have already discussed it that is equal to frequency $\omega_n = \sqrt{k/m}$ so if k/m is nothing but ω_n^2 so this is the equation of the motion the solution of equation of motion is this. Solution of the equation of the motion is $u(0)\cos\omega_n t + \frac{\dot{u}(0)}{\omega_n}\sin\omega_n t$ so in this $u(0)$ is initial displacement $\dot{u}(0)$ is initial velocity and natural frequency which is $\omega_n = \sqrt{k/m}$.

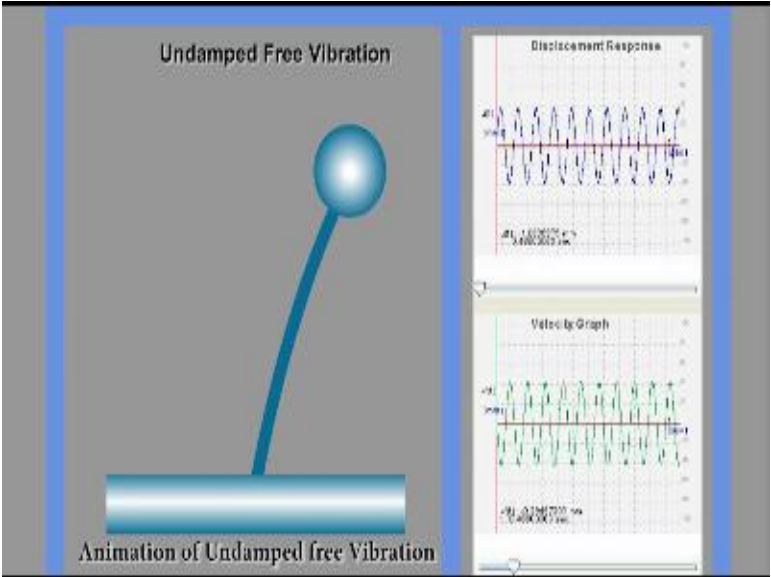
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Yeah again initial displacement yeah you can see here a time $t = 0$ this is the time scale a time $t = 0$ initial displacement is having some value it is non 0 value, okay some value is there but initial velocity is that is a tangent is horizontal to x axis that means initial velocity 0 initial displacement is non 0 in the second case initial velocity is present initial displacement is 0 and initial velocity is non 0 and here in third case initial displacement is there also initial velocity is so this 3 cases are there.

Initial displacement initial velocity initial displacement along with initial velocity now with initial velocity.

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Ex:1. System Parameters (m) and k ✓

Find the mass and stiffness of the given system

Thickness of slab: 120mm

Column size: 230 x 300mm

Grade of concrete: M20; Height: 3m

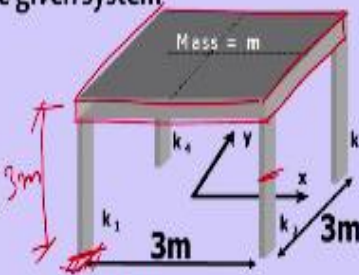
Note: All the columns are fixed at bottom and slab is rigid

Mass of slab:

Weight of slab = $3 \times 3 \times 0.12 \times 25 = 27\text{kN}$ ✓ $m = \frac{W}{g}$

Mass of Slab = $27000/9.81 = 2750\text{ kg}$

Note: Columns are assumed as massless



Now let us try to find out the system parameters so which is very much essential in linear single degree of freedom system for finding the solution so the 2 important system parameters in undamped free vibration or mass and stiffness that is m and k so mass and stiffness so if we look at this structure as slab supported on four columns so thickness of the slab is given is 120mm thick and column size is 230/ 300, so 230 on one side 300 on the other side 230 / 300 mm column size and the grade of concrete used as a m20 grade concrete material grade.

And height given as 3meters so a note is given all the columns are fixed at vast fixed as there here fixed it at vast slab is reject in its plane so there are more internal deformation in the slab internal deformation are not consider in the slab then how do we calculate the system parameters, so first of all we need to calculate mass of the slab for calculating mass of the slab we calculate the weight of the slab first so weight of the slab is we can get is through volume multiplied by weight density. So volume is $3\text{m} \times 3\text{m} \times 0.12\text{m}$ / this 120mm thick.

So this is the slab volume, volume of the slab concrete multiply by weight density so since this is reinforce cement concrete slab so 25 kN/m^3 is a weight density so that gives us 27kN as a weight of the slab. So mass of the slab is equal to weight/gravity so gravity value we are taking it as

9.81m/sec² and the 27kN that is multiplied by 1000 we get 27000N/9.81 so we get 2750kg is the mass of the system, so this is the first parameter.

Now we need to find out second parameter stiffness one note that columns are assumed as mass lasses, so usually columns contribute to the slab mass but in this case we are ignoring the mass of the columns, columns are contributing only for stiffness.

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Stiffness of columns:

Modulus of Elasticity = $5000\sqrt{f_{ck}} = 5000\sqrt{20} = 22360 \text{ N/mm}^2$ ✓

Along x direction:

Moment of inertia of column section: $300 \times 230^3 / 12 = 3.04 \times 10^8 \text{ mm}^4$

Lateral stiffness: $4x \frac{12EI}{l^3} = 4x \frac{12 \times 22360 \times 3.04 \times 10^8}{3000^3} = 12.08 \times 10^3 \text{ kN/m}$

Along y direction:

Moment of inertia of column section: $230 \times 300^3 / 12 = 5.18 \times 10^8 \text{ mm}^4$

Lateral stiffness: $4x \frac{12EI}{l^3} = 4x \frac{12 \times 22360 \times 5.18 \times 10^8}{3000^3} = 20.59 \times 10^3 \text{ kN/m}$

So how to calculate the stiffness of the column, so first of all we find out the modulus of elasticity of the material, column material that are concrete material since concrete material is given as m 20 grade concrete so according to IS456 2000 we calculate modulus of elasticity has 5000 under $\sqrt{f_{ck}}$ so which is 22360 N/mm² that is modulus of elasticity and then moment of inertia of column section i is $300 \times 230^3 / 12$ this is along one direction and lateral stiffness.

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Ex:1. System Parameters (m) and k ✓

Find the mass and stiffness of the given system

Thickness of slab: 120mm

Column size: 230 x 300mm

Grade of concrete: M20; Height: 3m

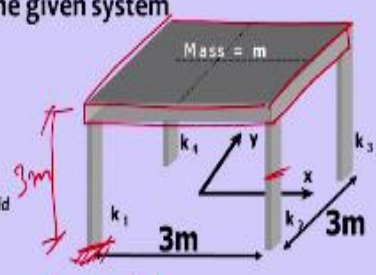
Note: All the columns are fixed at bottom and slab is rigid

Mass of slab:

Weight of slab = $3 \times 3 \times 0.12 \times 25 = 27 \text{ kN}$ ✓ $m = \frac{W}{g}$

Mass of Slab = $27000/9.81 = 2750 \text{ kg}$

Note: Columns are assumed as massless



So one column, so if we take stiffness of this column along this direction if it is moving like this in this direction so one unit, so this we have already discussed in detail so lateral stiffness of this can be quantified like $K_1 = 12EI/h^3$ in this one E is a modulus of elasticity or the concrete material I is the moment of inertia along the direction of motion and h is the height of the column, h is the height of the column.

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Stiffness of columns:

Modulus of Elasticity = $5000\sqrt{f_{ck}} = 5000\sqrt{20} = 22360 \text{ N/mm}^2$

Along x direction:

Moment of inertia of column section: $300 \times 230^3 / 12 = 3.04 \times 10^8 \text{ mm}^4$

Lateral stiffness: $4 \times \frac{12EI}{l^3} = 4 \times \frac{12 \times 22360 \times 3.04 \times 10^8}{3000^3} = 12.08 \times 10^3 \text{ kN/m}$

Along y direction:

Moment of inertia of column section: $230 \times 300^3 / 12 = 5.18 \times 10^8 \text{ mm}^4$

Lateral stiffness: $4 \times \frac{12EI}{l^3} = 4 \times \frac{12 \times 22360 \times 5.18 \times 10^8}{3000^3} = 20.59 \times 10^3 \text{ kN/m}$

So like that we are evaluating because four columns are present in x direction four columns are resisting the opposing the motion along x direction again four columns are opposing the motion along y direction. So what we do is when we are calculating along x direction so we will take moment of inertia accordingly in that direction. So if you look at this one four times multiplied by $12EI/l^3$ l is the length of the column or height of the column so this is stiffness of one column multiplied by four columns.

So E is modulus of elasticity that is 22360xI along one direction so 3.04×10^8 divided by height is 3m so 3000 mm^3 so we get $12.08 \times 10^3 \text{ kN/m}$ that is lateral stiffness along x direction and then moment of inertia of column section along y direction is obtained as $5.18 \times 10^8 \text{ mm}^4$ and accordingly if we calculate stiffness so that is 4 times $12EI/l^3$ we get 20.59×10^3 so in this we can observe one thing is here.

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Ex:1. System Parameters (m) and $k \checkmark$

Find the mass and stiffness of the given system

Thickness of slab: 120mm

Column size: 230 x 300mm

Grade of concrete: M20; Height: 3m

Note: All the columns are fixed at bottom and slab is rigid

Mass of slab:

Weight of slab = $3 \times 3 \times 0.12 \times 25 = 27\text{kN}$

Mass of Slab = $27000/9.81 = 2750\text{ kg}$

Note: Columns are assumed as massless

Diagram showing a 3x3m slab supported by four columns. The slab is labeled 'Mass = m'. The columns are labeled with stiffnesses k_1 , k_2 , and k_3 . The height of the columns is 3m. The slab dimensions are 3m by 3m. Handwritten notes include $m = \frac{W}{g}$ and $k_1 = \frac{12EI}{h^3}$.

It looks like you can see column size is 230/300 so columns are 300mm depth along y direction and size of column along x direction is 230 so that means what along x direction structure is little flexible and along y direction structure is little stiff this is the relative stiffness so along x direction it is flexible relatively along y direction it is stiffer relatively, so same thing we can see in the stiffness also.

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Stiffness of columns:

Modulus of Elasticity = $5000\sqrt{f_{ck}} = 5000\sqrt{20} = 22360 \text{ N/mm}^2$

Along x direction:

Moment of inertia of column section: $300 \times 230^3 / 12 = 3.04 \times 10^8 \text{ mm}^4$

Lateral stiffness: $4x \frac{12EI}{l^3} = 4x \frac{12 \times 22360 \times 3.04 \times 10^8}{3000^3} = 12.08 \times 10^3 \text{ kN/m}$

Along y direction:

Moment of inertia of column section: $230 \times 300^3 / 12 = 5.18 \times 10^8 \text{ mm}^4$

Lateral stiffness: $4x \frac{12EI}{l^3} = 4x \frac{12 \times 22360 \times 5.18 \times 10^8}{3000^3} = 20.59 \times 10^3 \text{ kN/m}$

Sp along x direction we are getting stiffness that is resistance to deformation as $12.08 \times 10^3 \text{ kN/m}$ and along y direction $20.59 \times 10^3 \text{ kN/m}$ along y direction so this is the stiffness calculation along x direction and y direction.

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Ex:1. System Parameters (m) and (k)

Find the mass and stiffness of the given system

Thickness of slab: 120mm

Column size: 230 x 300mm

Grade of concrete: M20; Height: 3m

Note: All the columns are fixed at bottom and slab is rigid

Mass of slab:

Weight of slab = $3 \times 3 \times 0.12 \times 25 = 27 \text{ kN}$

Mass of Slab = $27000 / 9.81 = 2750 \text{ kg}$

Note: Columns are assumed as massless

Handwritten notes and diagrams include:

- A diagram of a square slab with mass m supported by four columns of height 3m . The columns are fixed at the bottom. Stiffness values k_1, k_2, k_3, k_4 are indicated for the columns.
- A diagram showing a column of height h with a load W at the top, used to derive the stiffness formula $k_1 = \frac{12EI}{h^3}$.
- Handwritten calculations for mass: $m = \frac{W}{g}$ and $k_1 = \frac{12EI}{h^3}$.

So that completes the calculation of stiffness, along x direction along y direction so if structure is vibrating along x direction we can use mass as well as stiffness along x direction. If structure is oscillating along y direction then we can use mass and stiffness along y direction.

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Ex:2. Natural frequency

The roof of one storey building has mass 2000 kg . All the columns supporting the roof together offer a lateral stiffness of 30000 N/m . Find the natural period of the building.

Undamped system

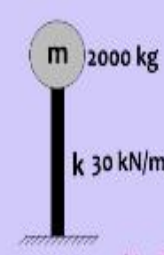
$T_n = \frac{2\pi}{\omega_n}$

$\omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{30000}{2000}} = 3.87\text{ rad/sec}$

$T_n = \frac{2\pi}{\omega_n} = 1.62\text{ sec}$

$\omega_n = 2\pi f$

$\omega_n = \frac{2\pi}{T}$ $f = \frac{1}{T}$ $m, k \rightarrow \omega_n, T_n$



In this example, we will learn how to evaluate natural frequency of the system where parameters of the system are given to us like question reads like this the roof of one storey building has mass 2000kg, mass is one parameter which is given already 2000kg. All the columns supporting the roof together offer a lateral stiffness of 30000 N/m so it is 30 N/m or 30000 N/m this is second parameter, so mass is given to us and stiffness is given to us.

So we need to find out natural period of the building, so natural period of the building and natural frequency of the building are related in this form $\omega_n = 2\pi f$ so this frequency is number of cycles per second, so in one second how many cycles are taking place that is represented by frequency f and natural period is time taken for 1, 2 and 4 or one full cycle, so they are related $f = 1/T$ or $T = 1/f$ so in this relationship so $\omega_n = 2\pi f$ we can also write this as $\omega_n = 2\pi/T$ or $T = 2\pi/\omega_n$ so T_n .

So we know how to calculate ω_n which is under $\sqrt{k/m}$ so k we know 30000 N/m and then m is 2000 kg so we get 3.87 radian per second. So this is circular frequency so this $3.87 \times 2\pi/3.87$ we get 1.62 seconds, so natural period of the system is 1.62 seconds so that is what is asked to calculate. So system parameters are mass and stiffness are essential for finding natural

parameters nor natural characteristics of the vibrating system that is natural frequency or natural period.

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