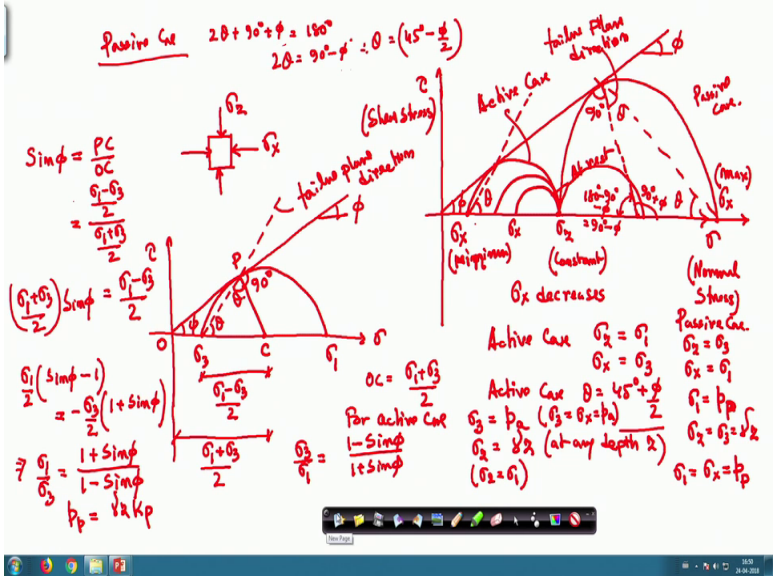


Foundation Engineering
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Lecture – 42
Earth Pressure – II

In the last class, I have discussed about the earth pressure theories and I was discussing about the active earth pressure. And, I have also discussed that what is the failure plane a direction and it is making an angle 45 degree plus phi by 2 with the horizontal. So, if I continue the same thing that as I have discussed that, if this is sigma plane sigma axis and this is the tau axis. Sigma is the normal stress, this is normal stress and this is shear stress and this is the Mohr coulomb failure envelop.

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And we are discussing these things for cohesion less soil. So, that is why Mohr coulomb failure envelope will pass through the origin and it is making an angle phi with the horizontal. So, phi is the friction angle. So, as I am discussed as I have discussed that for a at rest condition if this is the Mohr circle; that means, this is sigma z and this is sigma x ok. And that is I have discuss that if we are taking an element which is making this a sigma z is the vertical normal stress and this is sigma x which is the lateral stress.

Now, as I mention this is at rest condition. Now, as wall moves away from the backfill then the stresses in the horizontal direction decreases, but this sigma z value will remain

constant. So, σ_z is constant and σ_x decreases ok. So, the σ_z a σ_x decreases and σ_z constant; so, σ_x decreases means that this σ_x value will shift towards the origin ok. So that means, next Mohr circle will be this one and this will be the Mohr circle and where this is the σ_x minimum ok. This is the minimum σ_z x, we cannot get less than that because this Mohr circle as already touch this Mohr coulomb failure envelope. So that means, as I have already mentioned that any Mohr circle will not cross this line. So, this is the lowest σ_x value.

And so, I have mentioned that for a active condition for a active case that your σ_z is equal to σ_1 and σ_x equal to σ_3 . So, if you represent them in terms of effective stress then it will be bar or dash ok. So, σ_a z is σ_1 because, in this plane there is no shear stress. So, the all the stresses are principal stresses and this is as I mention the principal stresses are σ_1 and σ_3 . And so, I can write that this is the final Mohr circle where this is σ_1 , this is σ_3 ok. And this is I am talking about this is the active case ok. And, here as I have already mentioned that if I draw join these failure this point where the Mohr circle touches the Mohr coulomb envelope or failure envelope and if I join this σ_x and this line. So, this will make an angle θ and I have drawn for a active case this θ value is $45^\circ + \frac{\phi}{2}$.

So, I have already proved that that this is $45^\circ + \frac{\phi}{2}$. So, this is the same line failure envelop and I am taking the, this last Mohr circle so, this is the σ_3 . Now, we have to prove what is the value of or we have to determine what is the value of active earth pressure. So, this is the center C and this is the origin O and center C if I join this C point with this point where this a line touches this Mohr circle or Mohr circle touches this line. So, actually on this point P this line or failure envelop is a tangent acting along this point P on this circle.

So, this angle is 90° and similarly you can also if we join these two points or this point so, this is the θ and this angle is also θ . So, this θ value is $45^\circ + \frac{\phi}{2}$, that I have already proved and now σ and τ . Now, if I take this triangle so, before you try this is the triangle so, if this is the Mohr circle. So, you know the radius we can determine by $\frac{\sigma_1 - \sigma_3}{2}$. This is the radius of the circle and the center of the circle C from the origin is $\frac{\sigma_1 + \sigma_3}{2}$.

So, I can write that OC is $\frac{\sigma_1 + \sigma_3}{2}$ and the radius r of the circle is $\frac{\sigma_1 - \sigma_3}{2}$. So, if we have a Mohr circle with σ_1 and the σ_3 are the two principal stresses. Now, from this circle we can so, this is the ϕ value we can write that $\sin \phi$ is equal to so, we can write that $\sin \phi$ value which is equal to PC divided by OC ok. So, this is the PC and this is the OC because, this total angle is 90 degree, this is the perpendicular line on this failure envelop. So, we can write that $\sin \phi$ is PC by OC.

And as I mentioned this is the failure plane and the direction ok. So, in the active case the failure plane will make an angle $45^\circ + \frac{\phi}{2}$ with the horizontal that I have discussed. So, this is $\sin \phi$ is PC by OC and PC is the, this PC is the radius of this circle. So, PC is the radius of the circle means this is $\frac{\sigma_1 - \sigma_3}{2}$ and OC is the distance of this center from the origin O and OC is $\frac{\sigma_1 + \sigma_3}{2}$ ok. So, I can write that $\frac{\sigma_1 + \sigma_3}{2} \sin \phi$ is equal to $\frac{\sigma_1 - \sigma_3}{2}$. And, if I take all the σ_1 this side so, $\frac{\sigma_1}{2}$ will be $\sin \phi$ minus 1 equal to if I take a minus $\frac{\sigma_3}{2}$ divided by 2. So, this will be $1 + \sin \phi$ ok.

So, I am taking I am just separating $\frac{\sigma_1 + \sigma_3}{2}$ and $\frac{\sigma_3}{2}$ from this equation. So, I can write $\frac{\sigma_1}{\sigma_3}$ will be equal to because, here it is minus. So, I can write $1 + \sin \phi$ divided by $1 - \sin \phi$ because this is minus so, I am taking minus also this side $1 - \sin \phi$. So, the $\frac{\sigma_1}{\sigma_3}$ will be $\frac{1 + \sin \phi}{1 - \sin \phi}$. So, as it is mentioned that your for active case your σ_x is equal to σ_3 . So, I can write σ_3 is equal to the active earth pressure because, that σ_x is acting in the lateral direction which is the lateral a earth pressure. So, I can write that σ_3 here is equal to p_a ; because, the σ_x is acting in the lateral direction and for the active case σ_x is equal to σ_3 . So, σ_3 is equal to p_a and σ_z will be γz at any depth at any depth z. So, finally I can write that $\frac{\sigma_3}{\sigma_1}$ is equal to $\frac{1 - \sin \phi}{1 + \sin \phi}$ ok.

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$$\frac{\sigma_3}{\sigma_1} = \frac{1 - \sin \phi}{1 + \sin \phi}$$

$$\frac{p_a}{\gamma z} = \frac{1 - \sin \phi}{1 + \sin \phi}$$

thus, $p_a = \frac{1 - \sin \phi}{1 + \sin \phi} \gamma z$
 $= K_a \gamma z$

where K_a is the coefficient of active earth pressure

$$K_a = \frac{1 - \sin \phi}{1 + \sin \phi}$$

$K_p = \text{Coefficient of passive earth pressure}$

$$= \frac{1 + \sin \phi}{1 - \sin \phi}$$

$p_a = K_a \gamma z$
 (Active pressure)
 $p_o = \gamma z K_o$

$$K_o = \frac{\mu}{1 - \mu}$$

$$K_o = 1 - \sin \phi$$

And, again further I can write that sigma 3 divided by sigma 1 is equal to 1 minus sin phi divided by 1 plus sin phi and sigma 3 is equal to p a and sigma 1 is gamma z. So, this will be 1 minus sin phi 1 plus sin phi thus, p a is equal to 1 minus sin phi 1 plus sin phi into gamma z ok. And, this we can write this is equal to K a gamma z where, K a is the coefficient of active earth pressure and K a is equal to 1 minus sin phi divided by 1 plus sin phi.

So, finally I can write that p a is the active pressure which is equal to K a into gamma into z, gamma is the unit weight of the soil and z is the depth at any depth from the ground surface within the soil ok. So, this way we can calculate the active earth pressure or active pressure which is K a into gamma z. And, as per this Rankine's theory this K a is 1 minus sin phi divided by 1 plus sin phi ok. Now, if I go back to the previous slide so, this is our active case.

Now, in the passive case, what will happen? The wall will move towards the backfill. Now, if wall moves towards the backfill then the lateral stress on the soil element will increase ok. So, if its moves away from the backfill, then there will be an extension in the element and the stresses lateral stresses in the element that will decrease. But, if wall moves towards the backfill then there will be a compression in the element so; that means, the lateral stress in the element will increase.

But, your sigma z value will remain same again, here also whether it is active a condition or passive condition your sigma z value will remains same. So, this is the sigma z value, it is it is remain same for the active case now, passive case also it will remains same. So, now, at suppose this is our, at rest Mohr circle. So, when it is moves away from the backfill then a sigma z remains same. So, this Mohr circle will shift towards the origin because your sigma x value decreases.

Now, if it moves towards the backfill now, sigma x value will increase. So, what will happen from the, at rest condition this sigma x value will start moving away from the origin. So, there will be it will it will move this side and there will be point where this sigma x and sigma a z will remain say I will be same. Then it will moves in this direction and there will be one particular Mohr circle for which you will get the maximum sigma x value, this sigma x value this will be the maximum ok. So, this is also sigma x value maximum as a, if you look at this active case where sigma x value is minimum and here it is maximum.

So, in case of passive it is maximum this is an active case, this is the passive case ok. So, remember that because here also these Mohr circle touches the failure envelop or this line. So, it we will we will not get sigma x value more than this, but this is the maximum sigma x. And, again if I join these two lines these two points and if this is also theta so, again this is the active passive case. So, in the passive case this theta value will be 45 degree minus phi by 2, you can prove that all this one also. Because, this is if this is the center if you draw a line if you draw a line so, this is the 90 degree this is the 90 degree. So, this angle is how much? This is the 90 degree, this angle is this is 90 degree, this is phi.

So, this angle is 180 degree minus 90 degree minus phi. So, this angle is 90 minus phi ok. So, this angle will be 90 plus phi fine because, this is 90 minus phi so, this will be 90 plus phi. So, now if I consider this triangle because this is also theta, if this is theta this will be theta because this is the radius of the circle. So, this is a one radius and this is the radius so, these two are same. So, now we can write for the passive case or for the passive case your $2\theta + 90^\circ + \frac{\phi}{2}$ is equal to 180 degree ok. So, your theta value is $90^\circ - \frac{\phi}{2}$ a 90, 2θ is equal to $90^\circ - \phi$ sorry and this will also be phi. So, this is $2\theta + 90^\circ + \frac{\phi}{2}$ that will be equal to 180. So, 2θ will be $90^\circ - \phi$ so, theta will be $45^\circ - \frac{\phi}{2}$.

So, if you look at this thing that for the active case this failure plane is making an angle with horizontal is $45^\circ + \frac{\phi}{2}$, but for the passive case it is $45^\circ - \frac{\phi}{2}$. So, this is also failure plane direction ok. So that means, here remember that for the active case failure plane is making an angle $45^\circ + \frac{\phi}{2}$ with horizontal and for the passive case it is making an angle $45^\circ - \frac{\phi}{2}$ with the horizontal. So, now as I mentioned that for the active case your σ_z equal to σ_1 and σ_x is equal to σ_3 . But, for the passive case for the passive case it is a opposite ok. For the passive case your σ_z is equal to σ_3 and σ_x is equal to σ_1 ok.

Because now, in the active case you see that your σ_z is greater than σ_x , but for the passive case your σ_x is greater than σ_z ok. You can see that for the passive active case σ_x is a less σ_z is greater than σ_x , for the active case and for the passive case σ_x is greater than σ_z . So, for the passive case σ_z is equal to σ_3 and σ_x equal to σ_1 , σ_1 and we can say that that for the passive case your σ_1 is equal to p passive ok.

And, again σ_z is equal to σ_3 which is γz ok. So, here I have seen that σ_3 is equal to σ_x equal to p_a , similarly here it is σ_1 is equal to σ_x is equal to p ok. So, remember that for active case σ_3 is p_a lateral earth pressure and in that case your σ_z is equal to σ_1 and that is σ_z is equal to σ_1 which is γz equal to σ_1 which is γz . But, for the passive case your σ_1 is equal to p and σ_z is equal to γz which is σ_3 because, here your active case σ_z is greater than σ_x , but for the passive case σ_x is greater than σ_z . So, that is why σ_z is here σ_3 for the passive case and σ_x is σ_1 . So, now we have this expression σ_3 or σ_1 divided by σ_3 is $1 + \sin \phi$ $1 - \sin \phi$. So, passive case I can write now its σ_1 is equal to p .

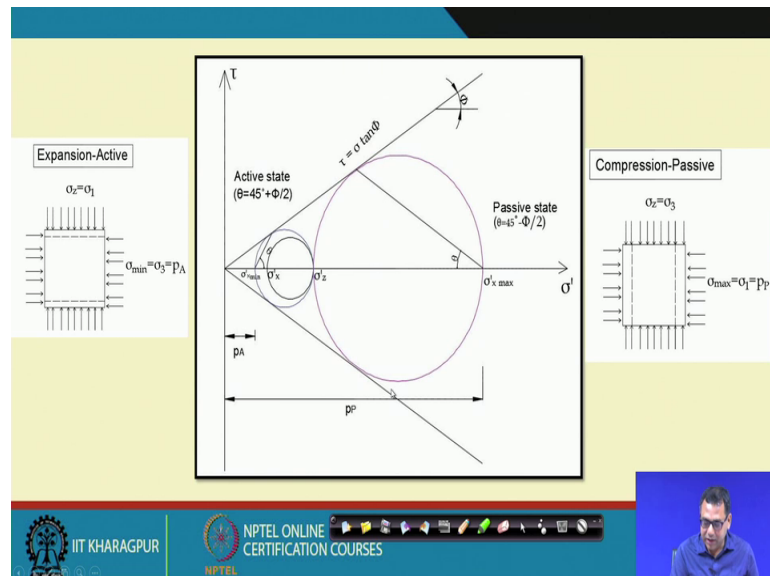
So, I can write p is equal to σ_3 is nothing, but γz γz into K_p ok. So, what is K_p ? Your K_p is the coefficient of passive earth pressure or passive pressure and this K_p is $1 + \sin \phi$ divided by $1 - \sin \phi$, you can see that this is $1 + \sin \phi$ $1 - \sin \phi$. So, this is the K_a expression and this is the K_p expression. So, you can write from this correlation that K_p is 1 by K_a or K_a is 1 by K_p ok. So, but these are the two coefficient as per the Rankine's earth pressure theories and I

have already mentioned that K_0 you can write by $\mu_1 - \mu$ or K_0 you can write $1 - \sin \phi$ by Jockey's expression.

So, these are the three expressions for three different conditions of earth pressure. For the at rest K_0 is $1 - \sin \phi$, for active $1 - \sin \phi$ divided by $1 + \sin \phi$. And, for the passive coefficient of earth pressure $1 + \sin \phi$ divided by $1 - \sin \phi$ ok. So, these are the as for the Rankine's earth pressure theory and before I go to the next part, just remember that in case of a active your σ_z is equal to σ_1 and σ_x is equal to σ_3 . But, in case of passive σ_z is equal to σ_3 and σ_x equal to σ_1 ok.

So, you can see from this Mohr circles these two Mohr circles ok. So, this is very important and in case of active a failure plane is making an angle $45^\circ + \phi/2$ in the horizontal. And, for the passive failure plane is making an angle $45^\circ - \phi/2$ with the horizontal ok. So, these are the all expressions for active and passive condition. So, again the passive is γz into K_p and active is also γz into K_a and at rest p_0 is equal to γz into K_0 . So, all the cases it is γz multiply with the coefficient of active or passive or at rest earth pressure ok.

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So, next part that I will discuss so; this is the figure that I have already discussed. So, in case of active it is expansion, in case of passive it is the compression. So, here σ_x value increases, here σ_x value decreases and this is the two a active and

passive cases. And, this active cases theta value is 45 plus phi by 2, for the passive case this theta value 45 degree minus phi by 2. So, these things I have already discussed.

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Active earth pressure

- If the wall moves away from the backfill, the soil element expands and the horizontal pressure decreases to a minimum value so that a state of plastic equilibrium is developed.

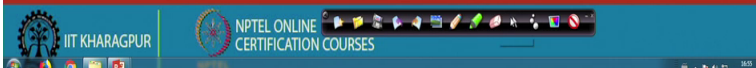
$$\sin \phi = \frac{\sigma_1 - \sigma_3}{\sigma_1 + \sigma_3}$$

$$\Rightarrow \frac{\sigma_1}{\sigma_3} = \frac{1 + \sin \phi}{1 - \sin \phi}$$

Here, $\sigma_1 = \sigma_2 =$ weight of soil at depth z , that is $\sigma_1 = \gamma z$.
 The minimum value of σ_3 is defined as the active earth pressure p_A ; that is $\sigma_3 = (\sigma_3)_{\min} = p_A$

$$p_A = K_A \gamma z$$

where K_A is the coefficient of active earth pressure $= \frac{1 - \sin \phi}{1 + \sin \phi} = \tan^2 \left(45^\circ - \frac{\phi}{2} \right)$



So, and this is the expression I have already discussed, this is your p_A is K into γz , K is equal to $1 - \sin \phi$ divided by $1 + \sin \phi$ or you can write in this form $\tan^2 45$ degree minus ϕ by 2. You can write with this form also $\tan^2 45$, which is the same thing in different form $\tan^2 45$ degree minus ϕ by 2 ok.

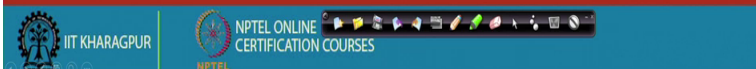
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Passive earth pressure

- If the wall moves towards from the backfill, there will be a uniform compression in horizontal direction. There will be an increase in the horizontal pressure while σ_2 remains constant.

Passive earth pressure can be estimated as $p_p = K_p \gamma z$

Where K_p is the coefficient of active earth pressure $= \frac{1 + \sin \phi}{1 - \sin \phi} = \tan^2 \left(45 + \frac{\phi}{2} \right)$



And, for the passive K_p into γz and K_p is $1 + \sin \phi$ divided by $1 - \sin \phi$ or $\tan^2 45^\circ + \phi/2$ ok.

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Active pressure on retaining walls: Cohesionless Backfill

a) Dry backfill with no surcharge

Total Active thrust $P_A = \frac{1}{2} K_A \gamma H^2$ $z = H$

$P_A = \frac{1}{2} \times H \times K_A \gamma H$
 $= \frac{1}{2} K_A \gamma H^2$
 $P_P = \frac{1}{2} K_P \gamma H^2$
 $P_0 = \frac{1}{2} K_0 \gamma H^2$

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Then the next one that and then this how we will get this is the pressure. How we will get the force? The same thing that it is making an triangle so, area of this triangle. So that means, the half height is H is the height of the wall is H and into this is K_A into γ into H , here z is equal to H ok. So, this is the P_A is equal to so, I can write half $K_A \gamma H^2$ and it will act with an height of $H/3$ from the base of the wall ok.

If it is passive then P_p will be half into $K_p \gamma H^2$ square, if it is P_0 at rest this will be half $K_0 \gamma H^2$ square. But, all the three cases this force will act from the a act at $H/3$ from the base of the wall where, H is the height of the wall ok. So, next part that here all the derivations, all the things I have derived without any surcharge ok, that mean I have consider only the backfill soil weight ok.

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b) Effect of uniform surcharge

- If a uniformly distributed load of intensity q /unit area is acting over the entire surface of backfill, then effective stress at any depth is increased by $K_a q$.

$P_a = H K_a q$

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Now, if we have a surcharge here so, this is the surcharge here is surcharge is applied and that surcharge is q per unit area or unit meter; so, because it is in the plane strain condition. So, in that case your distribution of the earth pressure will be $K_a q$ ok, K_a is the coefficient of active earth pressure; if it is passive then K_p into q ok. So that means, here K into q and the force will be definitely area of this rectangle. So, in that case your P_a active will be area is H into K_a into q ok. So, that is the force and again this will act because it is a rectangle. So, it will act at $H/2$ from the base or from the top so, this is this will act at $H/2$.

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c) Submerged backfill

Active earth pressure at base of wall
$$p_a = K_a \gamma H_1 + K_a \gamma' (H - H_1) + \gamma_w (H - H_1)$$

$p_a = K_a \gamma H_1 + (K_a \gamma' + \gamma_w) (H - H_1)$

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And so remember so, next one that is that if there is a water table is there ok, all these two cases I have discussed without considering the water table effect. Now, if there is water table, then what will be the changes ok? So, now for the first case suppose we have a water table at a height of H_1 from the ground level. So, up to H_1 this value is simple, if I am talking about K_A active case which is the submerge backfill that water table is at a height of H_1 from the from the top or ground level. So, this value is K_a small a or you can write capital A both are same; K_a into γ into H_1 that I have discussed and here this will be the γ_{dry} or γ_{bulk} and here it is γ_{sat} ok.

So, I can write this is $K \gamma_{dry}$ or γ_{bulk} and then this weight of the soil will act as a surcharge because, if I know now if I take two different layers say this is one layer, this is another layer. So, for the first suppose this is a one layer and this is the second layer. So, for the first layer this is the earth pressure. Now, on the second layer this first layers load will act an surcharge and that surcharge is γ_d into H_1 . And, if this surcharge is acting over here so, that will be K_a into that surcharge because, the soil is same so, that is why here also it is K_a , here also it is K_a .

If K_a changes later on I will discuss that then there will be a different story, but now both it is the same a soil. But, to assume it is two different layer; one is the above water table another is the below water table and the both the cases your K_a is same because, the ϕ value will remains same about water table and below water table. So, your K_a value will also remain same. So, I have consider this is K , now for the first layer it will be $K_a \gamma_d$ into H_1 the soil above the water table and that soil will act as a surcharge this soil below the a water table.

So, in that surcharge will be γ_d into H_1 into the K_a so, that is the case. So, it will act as surcharge rectangular, then it will additional part for the earth pressure below this a soil ok, for this soil below the water table. So, this one will be this value will be K_a into γ_{sat} minus γ_w because, this is below the water table into this height H minus H_1 because, this is H minus H_1 . So, now up to this the earth pressure due to the soil. Now, this third-one or the second triangle this is the earth pressure due to water ok. So, these earth pressure due to water will be w into H minus H_1 because, the water table height is also from the base of the wall is H minus H_1 .

So, now this is the total earth pressure diagram. This is above the water table where you have to consider the gamma bulk or gamma dry. Then this is the below water table where first one is the surcharge of the upper layer, second-one is the additional stress due to the a lower layer and third-one is the stress due to the water. So, the final stress you have to add this one plus this one and this one. This is the p_a acting at the base of the wall ok. So, this is the effect of water table.

Now, in the next class I will discuss if we have a inclined backfill. Now, the backfill in the soil we have assumed that it is perfectly horizontal, even the initial Rankine's theory also assumes that backfill is perfectly horizontal. But, later on this theory is been extended for inclined backfill also. So, that thing we will discuss in the next class.

Thank you.