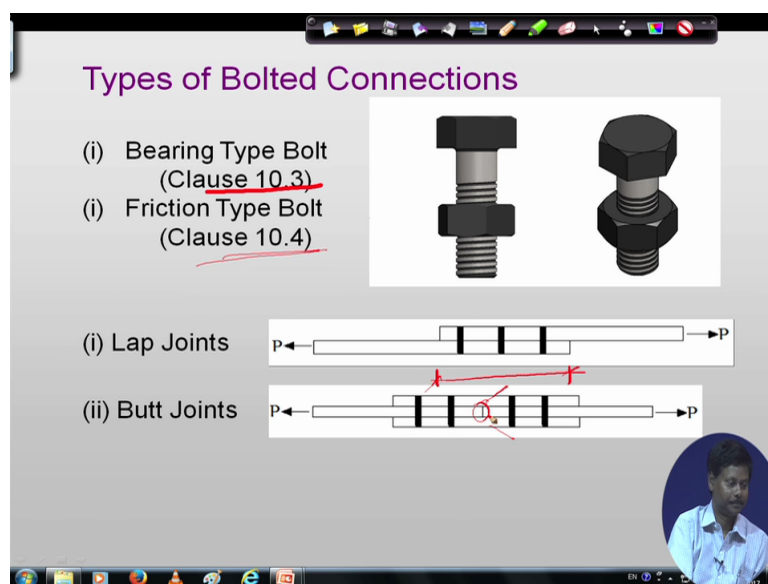


Course on Design of Steel Structures
Professor Damodar Maity
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Lecture 06
Module 2
Design of Ordinary Black Bolts

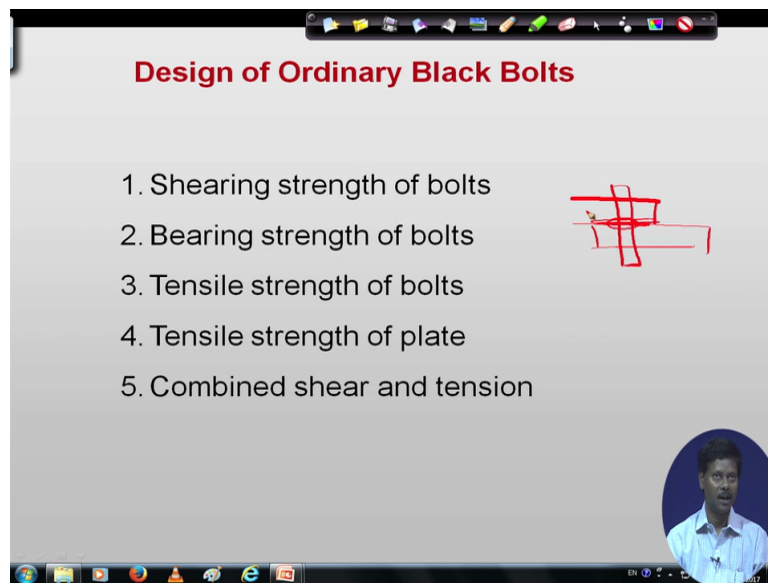
Hello today I am going to discuss the design procedure of bolts, bolt may be two types which are commonly used one is ordinary black bolt and another is high strength friction built bolt. Now today at first I will discuss about the design procedure of ordinary black bolt.

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Now as we know that one is bearing type of bolt which is given in clause 10.3 of IS: 800-2007 and another design procedure is friction type bolt which is given in clause 10.4. So while we will see the design procedure I would suggest the participants to follow the codal provisions also to open their code and to open this clause 10.3 while designing the ordinary black bolt and when we are going to design we will see the two type of joints will come across openly that is one is lap joint where the two plates are overlapped together at a certain length which is called lap joint and another is butt joint where two plates are in same plane are joint with some cover may be single cover or may be double cover which is called butt joint. So these two types of joints will be covered in todays lecture.

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Design of Ordinary Black Bolts

1. Shearing strength of bolts
2. Bearing strength of bolts
3. Tensile strength of bolts
4. Tensile strength of plate
5. Combined shear and tension

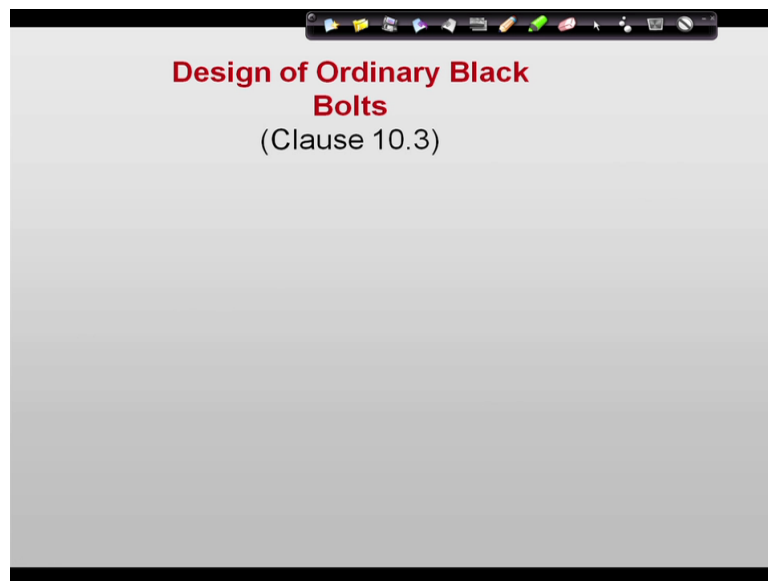
Now for designing of ordinary black bolt we will see that what are the failure criteria and from that failure criteria (what are the shearings) what are the strength like it may fail due to shear. We know the black bolt when we are going to design suppose the plates are here and another plates are connected here and we are designing a bolt then there is a chance of failure in this shear plane.

So in this plane the shear failure may come into picture for which we have to find out what is the strength of the shear of the bolt then we will go for bearing strength of bolt means bolt may fail due to bearing also as I have discussed earlier so what will be the strength calculation for bearing failure that we will discuss.

Next is the failure due to tension in the bolt bolt may occur under tension so that also has to be calculated. Next we will go for tensile strength of plate, means plates are made hole for inserting the bolt. So the net strength of the plate will be reduced because of the presence of hole, so that also we will try to find out what is the net strength of the plate and another aspect is that combined shear and tension means sometimes bolt exerts under combined shear and tension.

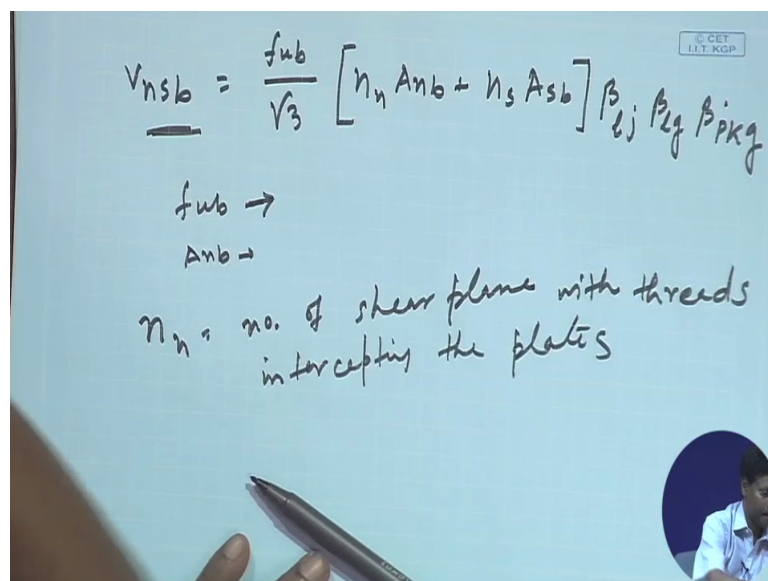
So that also we will check and out of this 5 we will see which one is the least strength and least strength will be the bolt strength, so we will try to find out the bolt strength for different failure criteria one is shearing, then bearing and then tensile strength and then tensile strength of plate and combined shear and tension.

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Next let us come to design of ordinary black bolt.

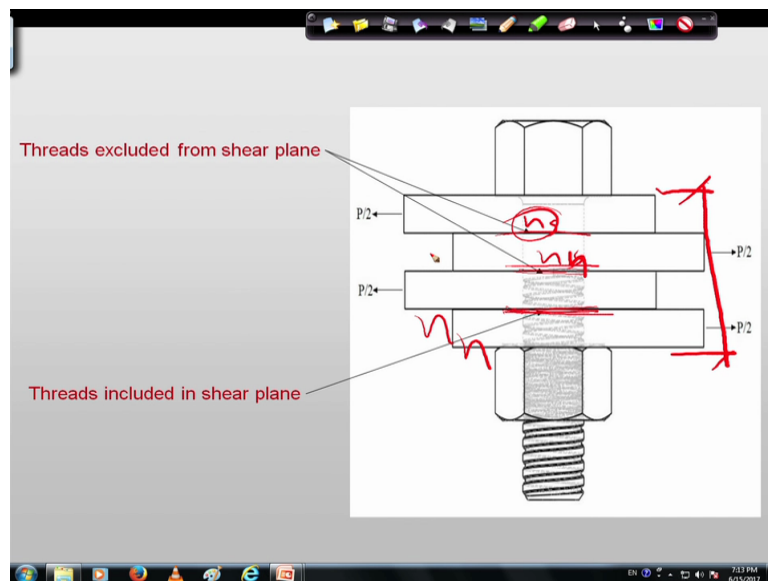
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Now in clause 10.3 you will see that bolt in shear V_{nsb} are given in the code as f_{ub} by root 3 into n_n into A_{nb} plus n_s into A_{sb} this is nominal capacity of bolt in shear into some reduction factor which is called β_{lj} then β_{lg} and β_{PKg} . So this V_{nsb} is nothing but the nominal capacity of bolts in shear and f_{ub} is the ultimate tensile strength of bolt.

So for different type of bolts the ultimate tensile strength of bolt will be different that we can find out from the code and this A_{nb} and A_{sb} I am coming, A_{nb} is the net tensile area of bolt to be consider at the root of the thread.

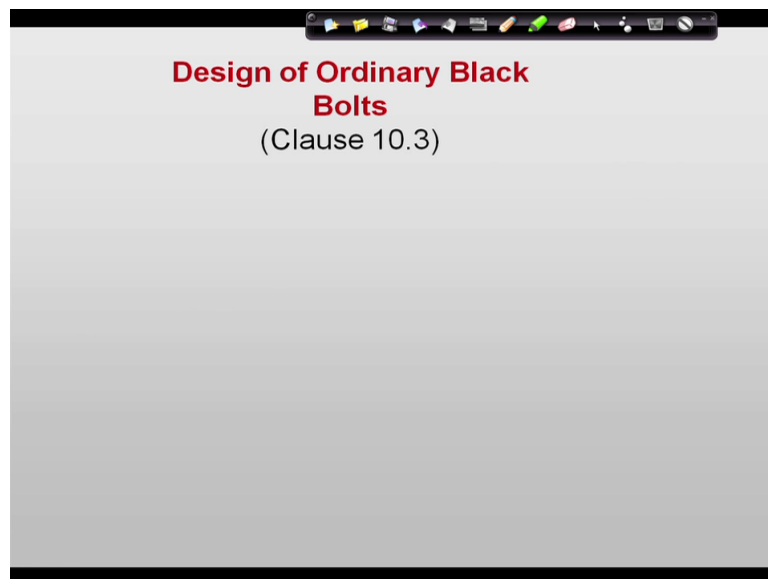
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That means if we go next page you will see here the entire if we see that entire bolt grip is this much sorry this is the entire bolt grip. Now say for example in this case the thread included in shear plane is this one this shear plane is included by that threads. Now the threads excluded in shear plane is this one or this one. So this is n_n and n_b , so n_n and n_b we can define as n_n as number of shear planes without thread intercepting the plate.

So n_n will be number of shear plane with threads intercepting the plate. That means this will be the n_n intercepting the plate this is n_n and n_s number of means here in in this case n_s is sorry this is n_s is n_n is one and if I consider this is also intercepting in thread then this is another one n_n and this is n_s , right. So similarly n_s is equal to number of shear plane with threads without threads intercepting the plates. I will start once again the last page from last page sorry I will start from this so now ok.

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So now I am going to discuss about the design of ordinary black bolts, this is given in clause 10.3 of IS: 800 now as I told that design of ordinary black bolts has to be designed under shear tension and bearing and tension of the plate. So we will go through one by one and we will see what are the codal provisions made and accordingly we will try to find out the design strength of the bolt under shear, under bearing and under tension. Now this is available in clause 10.3 of IS: 800-2007.

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shear

$$V_{nsb} = \frac{f_{ub}}{\sqrt{3}} [n_n A_{nb} + n_s A_{sb}] \beta_{lj} \beta_{tg} \beta_{rxg}$$

↳

$f_{ub} \rightarrow$

$n_n =$ no. of shear planes with threads intercepting the plane

$n_s =$ " " without " " "

$A_{ns} \rightarrow \frac{\pi}{4} \times d^2$

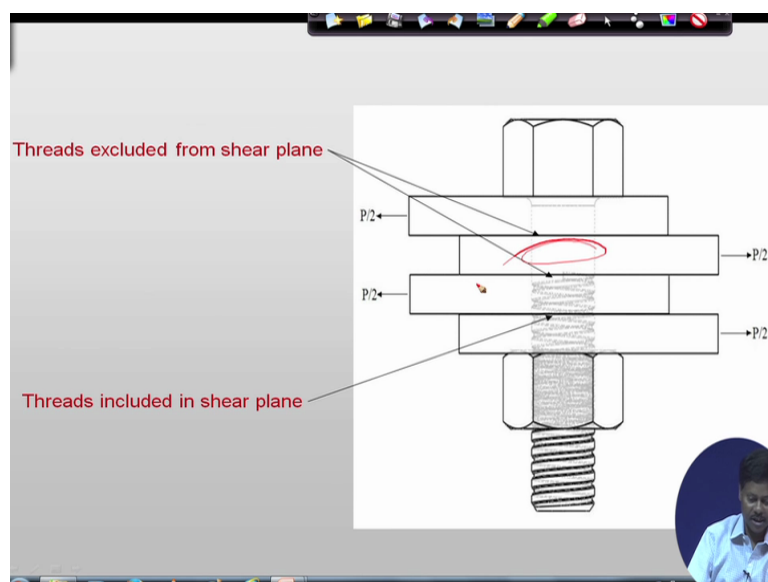
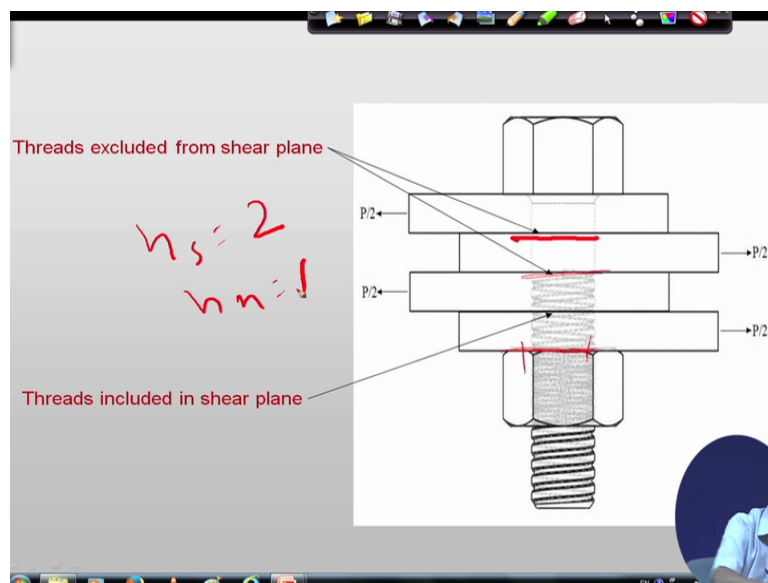
$A_{nb} \rightarrow 0.78 \times \frac{\pi}{4} \times d^2$

So clause 10.3 if you open you will see a formula is given for bolt in shear for shear the formula is given as V_{nsb} is equal to f_{ub} by root 3 into $n_n A_{nb}$ plus $n_s A_{sb}$ into β_{lj} these

are some reduction factor I will come into this reduction factor how to calculate and then beta P beta PKg.

So this V_{nsb} is basically nominal shear capacity of bolt and f_{ub} is the ultimate tensile strength of bolt and this ultimate tensile strength depends on the material property of the bolt. So what type of material property we are going to use depending on that we can find out the value of f_{ub} . Then n_s is the number of shear plane number of shear planes with threads intercepting the plane what is that intercepting the plane.

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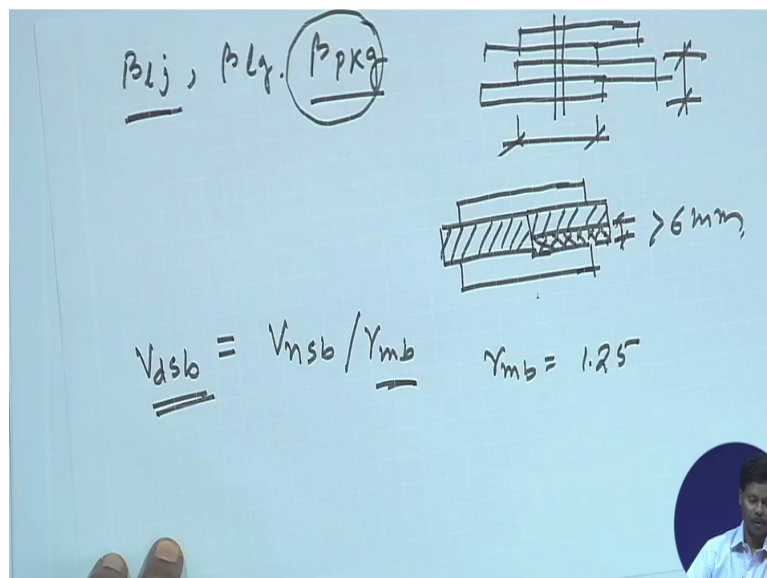
This is we can find out from this figure this is the figure where you see we have one type of plane that is thread excluded from shear plane that means thread is not there in this two and

another is thread included in shear plane that means this thread threaded portion is included in shear plane.

So n_n will be when threads intercepting the plane and similarly n_s will be number of shear planes without threads intercepting the plane without threads intercepting. So in this case n_s will be 2 and n_n will be in this case 1 and accordingly we have to find out the A_{nb} and A_{ns} that how do we find out that is A_{ns} is the cross sectional area of the plane shank cross sectional area of the plane shank that means this portion cross sectional area of the plane shank this portion and the threaded portion the cross sectional area we can consider as A_{nb} , so this is how we can calculate.

Now this threaded portion when we will calculate what will be the cross sectional area, now we know the cross sectional area of the shank portion will be $\pi/4 \times d^2$, where d is the nominal diameter of the bolt but when we are going to calculate the net area of the threaded portion we will reduce to to a certain extent which is suggested by your code as 0.78 times the cross sectional area of the shank area, that means this will be reduced to 0.78 times of $\pi/4 \times d^2$. So A_{nb} the net area of the threaded portion we will consider in this way.

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That one is β_{lj} , then β_{lg} and β_{PKg} . Now β_{lj} is the reduction factor which allows the overloading of n bolts that occur in long connection. If a connection is quite long if a connection is quite long then there will be a factor which we have to multiply with the capacity whatever is coming this factor is called β_{lj} which is a reduction factor.

Similarly β_{lg} is the reduction factor due to large grip length, if the grip length is large that means plate thickness if it is high or several plates are given then grip length increases, say suppose I am increasing the number of plates and joining with a bolt.

So if grip length is more then we have to multiply a factor which is called reduction factor for large grip lengths and also β_{PKg} β_{PKg} is the reduction factor for packing plates suppose we have a (gusset at) sorry butt joint we have two plates of different thickness now we have to join with certain number of bolts we have to make to join the two plates this is one plate and this is another plate.

So for this to reduce the gap we may have to provide packing plates, so this is called packing plates. So these packing plates when we are going to consider we are using a reduction factor for packing plates however if it is thicker than 6 mm if this packing plate is thicker than 6 mm then we have to multiply a reduction factor of β_{PKg} .

Now the design shear force which is called V_{dsb} , I am taking the parameter name as given in the code same parameter I am using. V_{dsb} is equal to V_{nsb} by γ_{mb} , this V_{dsb} is the design shear force and γ_{mb} is the partial safety factor, this γ_{mb} is given in IS code in table 5, in table 5 you will get a different type of partial factor for bolted connection, for welded connection and for different cases the value of partial safety factor of the bolt or rivet bolt or welded according to the material it has been given. Now this γ_{mb} we consider here as for bolt we use 1.25, right.

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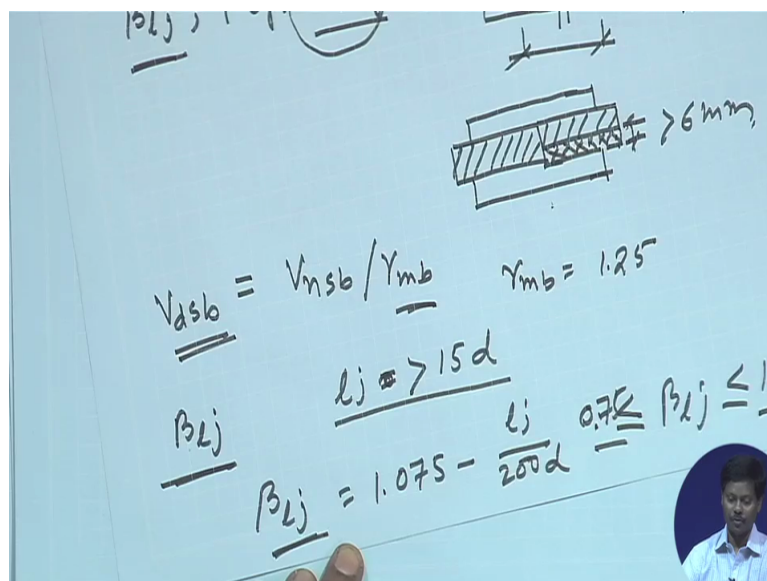


Diagram showing a bolted joint with a packing plate of thickness $t_p > 6 \text{ mm}$.

$$V_{dsb} = \frac{V_{nsb}}{\gamma_{mb}} \quad \gamma_{mb} = 1.25$$

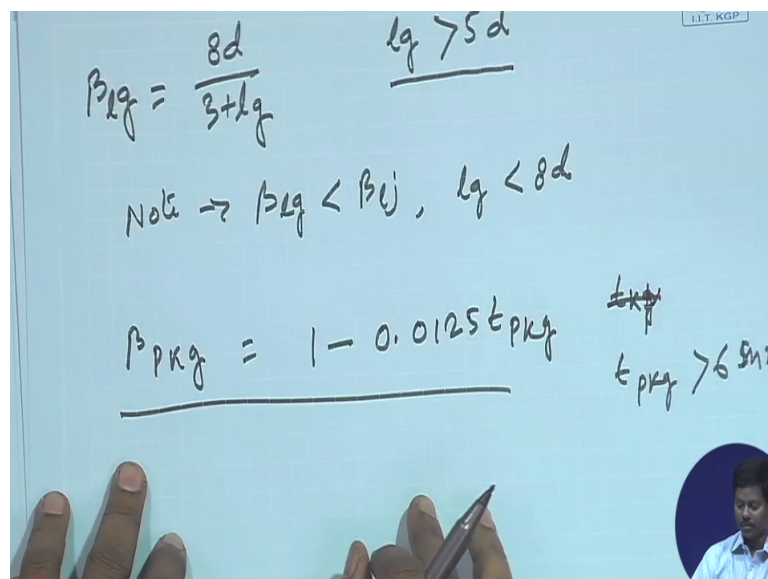
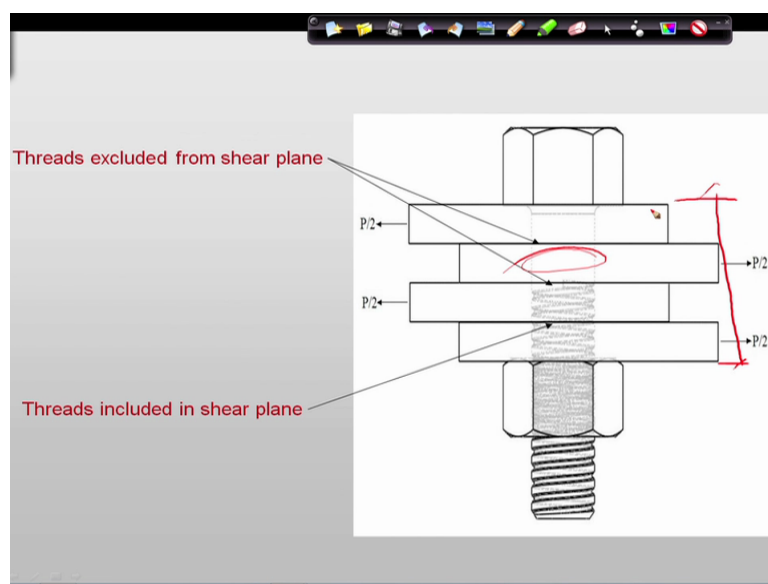
$$\beta_{Lj} = 1.075 - \frac{L_j}{200d} \quad \text{for } L_j > 15d$$

$$\beta_{Lj} \leq 1.0 \quad \text{for } L_j \leq 15d$$

Now let us come to the reduction factor how do we calculate β_{lj} this is reduction factor due to long joint. So long joint means what long joint means if this length of joint become more than 15 times of nominal diameter of the bolt if this length of joint become more than 15 times of nominal diameter of the bolt then we can calculate β_{lj} as this $1.075 - l_j$ by $200d$.

So from this formula we can find out the reduction factor due to long joint β_{lj} and this β_{lj} should not be less than 0.75 and should not be more than 1.0, so β_{lj} will vary from 0.75 to 1.0 this is how we can calculate the value of β_{lj} .

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Next we will calculate the value of β_{lg} which is reduction factor due to large grip length, as I told large grip length means when this length is becoming high and when this l_g large

grip length is more than $5d$ d is the nominal diameter of the bolt. So when l_g become more than $5d$ then I can calculate the β_{l_g} value as $8d$ by 3 plus l_g , right and note that that β_{l_g} should be less than β_{l_j} and l_g should be less than $8d$ then we can use this formula.

Another reduction factor is for packing plates that is β_{Pkg} this is calculated from this formula that is $1 - 0.125t_{Pkg}$. Now if packing plates is more (t_{pk}) I am sorry t_{Pkg} if this is more than 6 mm then we can use a reduction factor of β_{Pkg} as this, if it is less than 6 mm we do not have to multiply the reduction factor this is how we can calculate.

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(2) Bearing

$$V_{npb} = 2.5 K_b d \cdot t \cdot f_u$$

$$K_b = \text{smaller of } \left[\frac{e}{3d_0}, \frac{p}{3d_0} - 0.25, \frac{f_{ub}}{f_u}, 1 \right]$$

$$\phi V_{dpb} = \frac{V_{npb}}{\gamma_{mb}} \rightarrow 1.25$$

We have seen first is that bolt in shear now for bearing as I told that bolt may fail due to shearing effect and due to bearing effect and bolt may fail due to tension also the joint may fail due to tension of the plate tension failure of the plate that has also have to be consider.

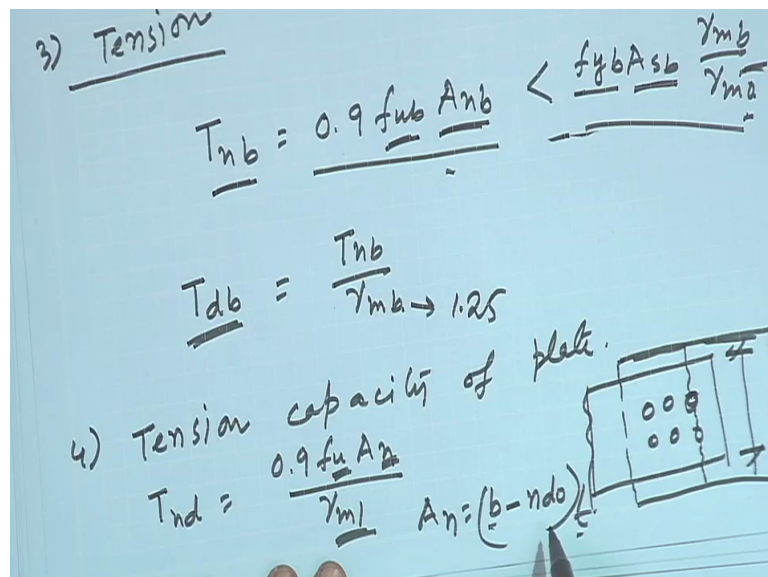
So shearing strength how to calculate we have seen now we will find out the bearing strength that is calculated from this formula that is V_{npb} is equal to $2.5 k_b d$ into t into f_u , here this V_{npb} is the nominal bearing strength of bolt nominal bearing strength of bolt and f_u is the ultimate tensile stress of plate not bolt remember, earlier we have calculated the ultimate tensile stress ultimate stress of bolt in earlier formula but here it is ultimate tensile strength of plate because it is bearing on plate.

And d is the nominal diameter of bolt d is the nominal diameter of bolt and t is the summation of thickness of connected plates that means the total thickness of the connected plate will be the thickness t . Now another factor is a constant which is K_b and this K_b can be calculated form smaller value of this few parameters e by $3d_0$, p by $3d_0$ minus 0.25 , f_{ub} by f_u

and 1 whichever is less, so smaller of this few what is d_0 , d_0 is the hole diameter that means nominal diameter plus clearance hole diameter d_0 .

Sometimes we represent as d_h in some books you will find d_h and sometimes it is d_0 and e is the edge distance and p is the pitch distance and f_{ub} and f_u we know so smaller of all these quantity will be the value of K_b . So K_b we can find out now the design shear force V_{dpb} you can calculate from the nominal shear force V_{npb} by γ_{mb} , γ_{mb} is the partial safety factor of (bear) bolt and this value is 1.25 which we can find out from table 5 of IS: 800-2007. So we can find out the value of γ_{mb} from table 5 and we can find out the design shear force sorry design bearing force.

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3) Tension

$$T_{nb} = 0.9 f_{ub} A_{nb} < \frac{f_{yb} A_{sb} \gamma_{mb}}{\gamma_{ma}}$$

$$T_{db} = \frac{T_{nb}}{\gamma_{mb} \rightarrow 1.25}$$

4) Tension capacity of plate.

$$T_{nd} = \frac{0.9 f_u A_n}{\gamma_{m1}} \quad A_n = (b - n d_0) t$$

The diagram shows a rectangular plate with four bolts arranged in a 2x2 grid. The plate has a width b and thickness t . The distance between the center of the first and last bolt is p . The distance from the edge to the center of the first bolt is e . The hole diameter is d_0 .

Next will come bolt in tension, so for tension we can calculate the nominal capacity of bolt in tension as T_{nb} is equal to $0.9 f_{ub}$ into A_{nb} and it has to be less than $f_{yb} A_{sb}$ into γ_{mb} by γ_{m0} , right. So T_{nb} we can calculate from this formula also we will calculate this and we have to check that T_{nb} is becoming less than this otherwise you will calculate this value.

So as we told f_{ub} is the ultimate tensile stress of bolt f_{ub} and A_{nb} is the shank area sorry A_{nb} is the net area at the thread and A_{sb} is the cross sectional area at the shank and γ_{m0} is 1.1 which again we can find out from table 5 of IS: 800 and then γ_{mb} also we can find out from similar table from same table table 5 of IS: 800 which is 1.25 and this is how we can find out the value of T_{nb} and F_{yb} is the yield stress of the bolt yield stress of the bolt material we have to find out which is called f_{yb} .

Then we can find out T_{db} the design tensile force that will be T_{nb} by γ_{mb} . So and we know γ_{mb} is basically 1.25 and we can find out the design tensile force as T_{nb} by γ_{mb} . So what we have seen that the strength of bolt due to shearing, due to bearing and due to tension we have calculated.

Now another aspect is that bolt may fail means the joint may fail due to tensile failure of the plate. So if plate fails then the joint is going to fail. So the tension capacity of the plate also has to be calculated while calculating the bolt strength of the joint. So we will calculate now tension capacity of plate.

So in this case we can find out the tension capacity of plate T_{nd} as $0.9 f_u A_n$ by γ_{m1} and we know f_u is the ultimate tensile stress of plate and γ_{m1} is the partial safety factor which is 1.25 and A_n is the net effective area of plate. Suppose a plate is joint like this, two plates are joint like this, right. Now we have bolt here so one scope is that it may fail due to tensile force exerted on the plate and so when we are going to calculate the tensile strength of plate we will calculate the net area of the plate net effective area.

So what will be the effective area A_n , A_n will be b if this is b width of the plate then b minus n into d_0 into t , because b if we have to calculate the tensile strength of the solid plate then simply we can find out b into t , but because of the presence of hole the net area is going to be reduced, when net area is going to be reduced, so the tensile capacity of the plate also is going to be reduced because in tension the bolt area has to be reduced means hole area hole area has to be deducted and hole area is the d_0 , that means bolt nominal bolt diameter plus clearance of the clearance on the hole.

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5) Combined Shear & Tension

$$\left(\frac{V}{V_{sd}} \right)^2 + \left(\frac{T_e}{T_{nd}} \right)^2 \leq 1.0$$

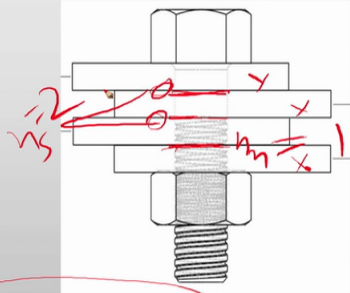
$V < V_{sd}$
 $T_e < T_{nd}$

Now another failure may come which is 5th one that is bolt with combined shear and tension, sometimes bolt are exerted to combined shear and tension. So when we are calculating individually the shear strength and tension strength of the bolt we have to also calculate that if both the shear and tension acts then what will be the combined strength of the bolt and that is found from this formula from this interaction formula that it has to fulfill this criteria and it has to be less than 1.0.

So here V is the applied shear force, V_{sd} is the design shear capacity, T_e is the externally applied tension T_{nd} is the design tension capacity. So not only V should be less than V_{sd} it has to maintain T_e should be less than T_{nd} but also it has to follow this means this check has to be conducted and the summation of these two will be less than 1.

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Example: Calculate the shear strength of 16 mm diameter bolt of grade 4.6. The bolt is under triple shear as shown in the figure below.



Solution:
$$V_{dsb} = \frac{f_{ub}}{\sqrt{3}\gamma_{mb}} (n_n A_{nb} + n_s A_{sb}) \beta_{lj} \beta_{lg} \beta_{pkg}$$

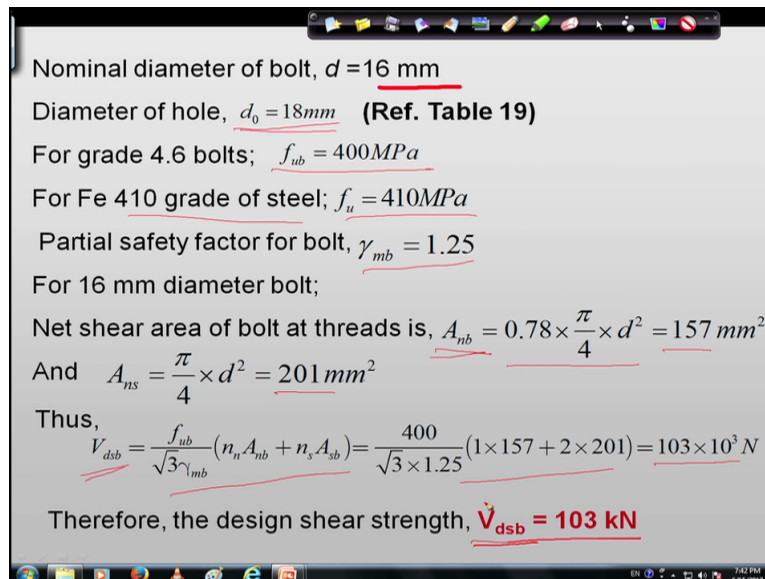
n_n = no. of shear planes with threads intercepting the plane = 1
 n_s = no. of shear planes without threads intercepting the plane = 2

Now whatever we have discussed we will go through one example and we will try to understand that how to calculate the bolt strength. Here first we are giving an example of calculation of the shear strength of the bolt, say for example this is a connection where three plates one, two and three plates are joint by a bolt. So the problem is that calculate the shear strength of 16 mm diameter bolt of grade 4.6 and the bolt is under triple shear as shown in the figure below. So we can see here that shear plane is this is one, this is two and this is three.

So when calculating the shear strength we know the formula is this that V_{dsb} is equal to f_{ub} by root 3 gamma mb into $(n_n A_{nb} + n_s A_{sb})$ into $\beta_{lj} \beta_{lg} \beta_{pkg}$. Now suppose we have the joint of short joint means joint is not long and grip length is not larger and also packing plate is not given. So we can omit this three terms this reduction factors terms we can omit.

Another thing now we have to calculate then the value of V_{dsb} on the basis of this formula, right. Now what is n_n as we know n_n is the number of shear planes with threads intercepting the plane, so here n_n is basically 1, number of shear plane with threads, this is only 1 n_n , this is n_n is equal to 1 and number of shear planes without threads intercepting the plane we have 2, this is 1 another is this this 2 we have n_s , right. So while calculating the design strength of the bolt we will use n_n as 1, n_s as 2.

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Nominal diameter of bolt, $d = 16 \text{ mm}$
Diameter of hole, $d_0 = 18 \text{ mm}$ (Ref. Table 19)
For grade 4.6 bolts; $f_{ub} = 400 \text{ MPa}$
For Fe 410 grade of steel; $f_u = 410 \text{ MPa}$
Partial safety factor for bolt, $\gamma_{mb} = 1.25$
For 16 mm diameter bolt;
Net shear area of bolt at threads is, $A_{nb} = 0.78 \times \frac{\pi}{4} \times d^2 = 157 \text{ mm}^2$
And $A_{ns} = \frac{\pi}{4} \times d^2 = 201 \text{ mm}^2$
Thus,
$$V_{dsb} = \frac{f_{ub}}{\sqrt{3} \gamma_{mb}} (n_n A_{nb} + n_s A_{sb}) = \frac{400}{\sqrt{3} \times 1.25} (1 \times 157 + 2 \times 201) = 103 \times 10^3 \text{ N}$$

Therefore, the design shear strength, $V_{dsb} = 103 \text{ kN}$

So if we see now that nominal diameter of bolt was given as 16 mm, so hole diameter will be d_0 will be 16 plus 2, because as per table 19 the clearance of the bolt for 16 mm diameter will be 2 mm. So the whole diameter will become 18 mm and for 4.6 grade bolt we know f_{ub} the ultimate stress of bolt will be 400 MPa and for Fe 410 grade of steel f_u we know f_u is the ultimate tensile strength of the plate that is 410 MPa and partial safety factor from table 5 we can find out as γ_{mb} as 1.25.

So now we can find out the value of A_{nb} A_{nb} will become 0.78 into π by 4 into the square which is 157 and A_{ns} will become π by 4 into d square which is coming 201. So thus V_{dsb} the design strength of the bolt due to shear we can calculate from this formula and if we put this value we can find out the value as 103 into 10 to the power 3 Newton or we can say the design shear strength of the bolt will become this much V_{dsb} will be 103 kilonewton.

So this is a small example we have shown where only shear strength has been calculated and shear strength has been calculated due to multiple shear that means we have tried to understand here that what will be the value of n_n , how we will calculate the value of n_n and n_s and what will be the A_{nb} and A_{ns} and accordingly what will be the V_{dsb} the design shear strength due to shear in the bolt.

So due to multiple shear in this case the triple shear the example has been worked out and has been shown I hope you have understood this example, thank you very much.