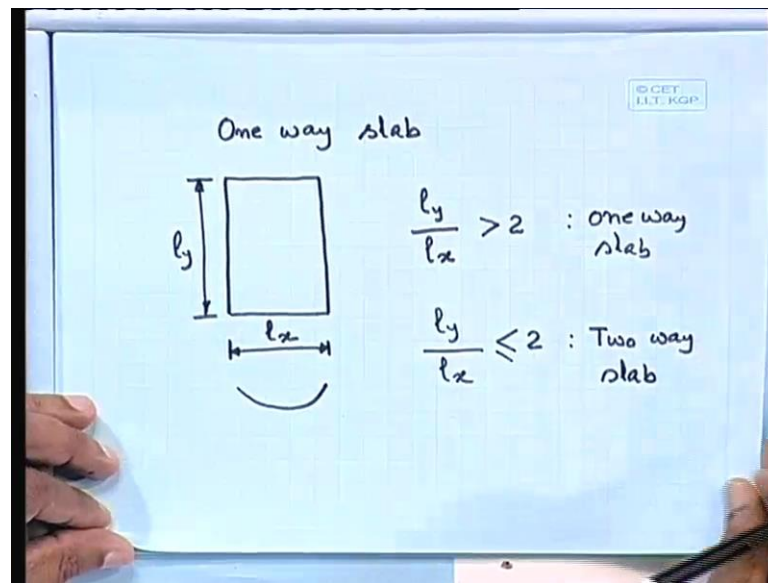


Design of Reinforced Concrete Structures
Prof. Nirjhar Dhang
Department of Civil Engineering
Indian Institute of Technology, Kharagpur

Lecture - 16
Design of Slabs Part III

We shall continue, with the same design of slabs and this is our part 3. So, lecture 16 design of slabs part 3 that we shall continue

(Refer Slide Time: 01:33)



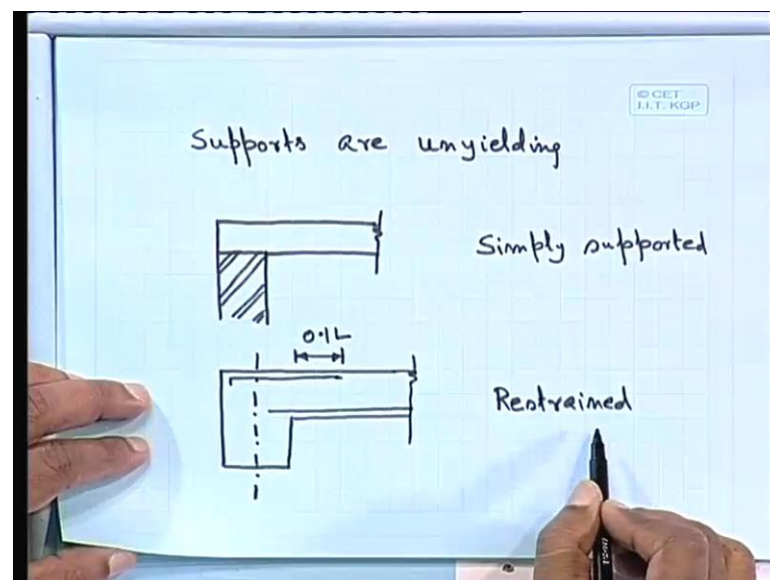
So far, we have done that 1 way slab it means; we have 2 parts spanning in the shorter direction. Let us say l_x and the other 1 longer direction. Let us say that is l_y . So, depending on the case l_y by l_x , if it is greater than 2 then; we shall find out as if that bending will be more in this direction; that we shall get and that is called 1 way slab. So, this is; that means, this is 1 way slab. So, the more load will be taken, in this direction shorter direction and we can design only the bending, we can design only in taking the shorter direction that we can take and we have done in the last 2 classes.

Today, we shall do the other 1 that is 2 way slab where l_y by l_x l_y by l_x less than equal to 2 and that is called 2 way 2 way slabs. So, in our case that l_y by l_x less than equal to 2 that is 2 ways slab; that means, not fully taken by l_x the shorter direction the other direction also taking certain portion of the load and that, we shall design how to design that 1 that we shall find out today. There are different supports we call it the supports are unyielding; that means, there is no displacement or deflection of the supports. It may be

we can define it say this is your as, if it supported over the masonry wall and this 1 we can take it as is simply supported case simply supported.

It can have certain kind of like this, there is a end beam say there is end beam and the reinforcement at the top. We shall get and then at the bottom also we have to provide the reinforcement generally, it happens the reinforcement this length that is $0.1L$ and this is restrained. So, 1 case is simply supported but, we are taking all of them that is supports are unlinked that there will be no deflection of the support; example you can say as, if it is not spring mounted the supports are not spring mounted.

(Refer Slide Time: 3:35)



So, simply supported then we can take say strength the other 1 we can take. So, this is your continuous. So, continuous support it may be beam or it may be wall also. So, these are the 3 different supports that we take it for the design of slabs and in all the cases. We have to take that l_y by l_x the longer duration the span in the longer duration divided by the span, in the shorter direction should be less than equal to 2 that is our case. In that case only we can design it as I say 2 way slabs, we can design that 1 as a 2 way slab only 1 we shall get l_y by l_x less than equal to 2.

There are few more cases we can take that 1 generally it happens; the things that what I mean to say this is your 1 corner initially, it was like this. This is 1 corner and the due to say simply supported case it will happen say if you put a plate like this due to bending what will happen, you will find out that corners that will go up. So, if I have to keep it in

proper position. So, you have to provide reinforcement of the top as well as the bottom that also you have to provide the reinforcement.

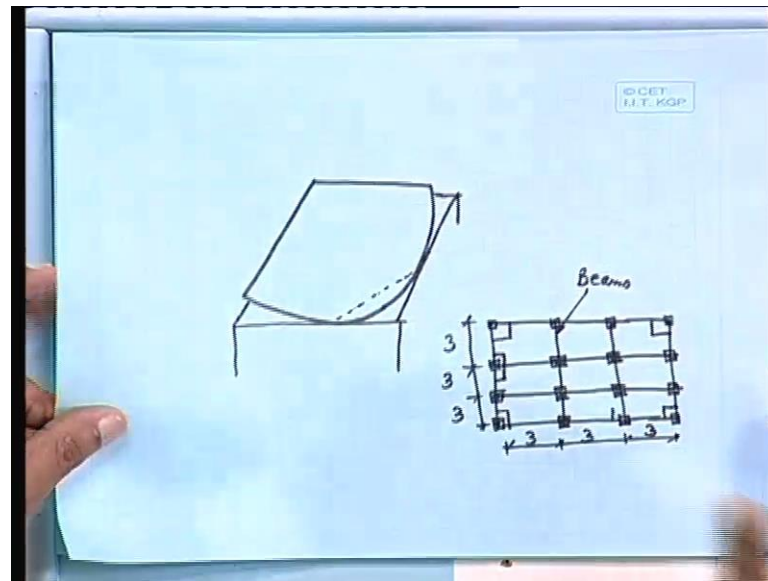
So, it is to so, here just what I am trying to show that if it have say corners are there that is 1 is possible case that corners are restrained or corners are not restrained. That if I have to restrain your corner; that means, you have to provide reinforcement in each corner but, that is true only where we have to provide the reinforcement, we have to provide the reinforcement only, in the corner that discontinuous the corner which is the support which is continuous.

There you need not specify you have to specify the that reinforcement only that y of the corner that you say isolated corner 1 force on this way or the other way, but, not like this. It may happen say something like this that; this is your panel. Let us say and we are providing say different beams. So, this is I am talking say plan these on I am talking plan and these are all beams to be more specific I can say there are so, many columns maybe, we can specify the column at each beam column junction here or it can be alternate also depending on the design.

So, we can say these are all each panel and i am talking this 1 plan. So, what we can do we have to design each of them depending on the l_y by l_x ratio. Let us say in this side all of them having certain portion. Let us say we can specify say 3 meter each something like that we can specify. This side also we can specify say 3 or 3 0.5 something like that let us say: not each of them we have to find out depending on the l_y by l_x we have to design it and design it as 2 way slab in this case, what we have to do.

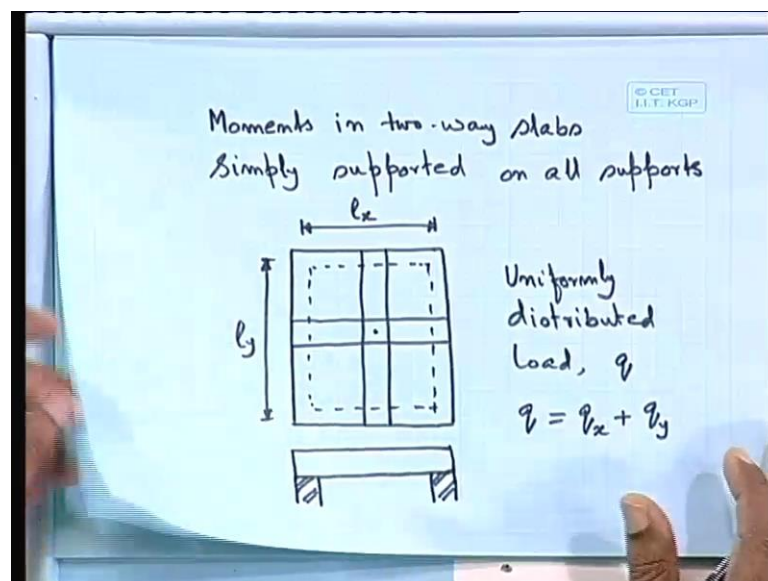
This is your 1 corner where it may happen that corner will go off so; that means, you have to provide the reinforcement of a certain distance you have to provide the reinforcement here sorry here like this. But, need not provide reinforcement here we need not provide reinforcement here, but at the same time we have to provide reinforcement here, because this 1. There is a discontinuity this side there is no edge. So, we have to provide the reinforcement only where we do not have any continuity. So, there you have to provide the corner reinforcement. That we shall that also we have to design and we have to provide reinforcement. That we shall that also we have to design and we have to provide reinforcement.

(Refer Slide Time: 06:53)



So, now let us come the few cases another 1; I can tell you that it happens because, due to dislift it may happen; there is a crack here why you do have to provide just for you for the reason that generally, if it lifts there is a possibility of crack here. So, to work on that care we provide the reinforcement in these corners. But whereas, in the continuous 1 since it is by all 4 sides. So, no need of provide that corner reinforcement yes; it will lift because of that at the bottom also that tension compression that will happen and that it will you will get that crack.

(Refer Slide Time: 11:59)



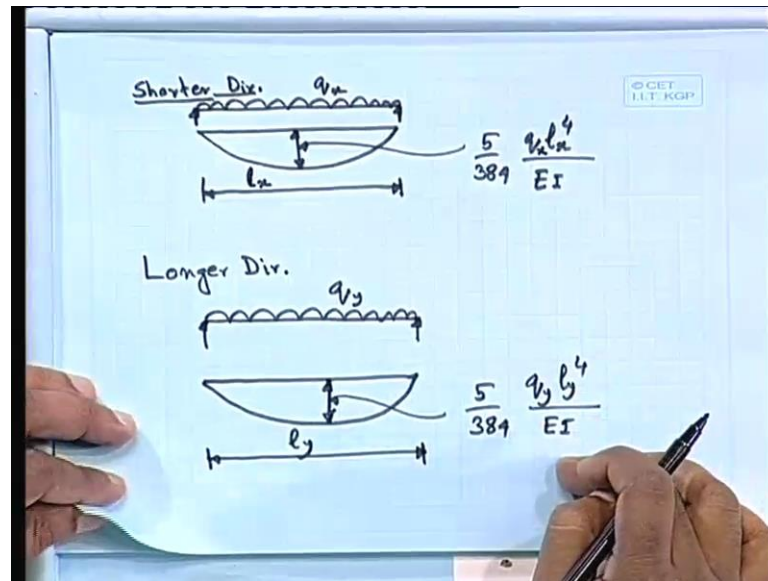
Now, let us come to the 1 case that moments in 2 way slabs special case say simply supported on all supports please note the supports are on yielding that supports, will not be deflected this is a 1 case I can say as if we are having the slab which is supported over brick wall in all 4 sides. So, brick wall in all 4 sides and we have to design that we have to find out that moment distribution; that means, whatever the load we are getting here may we can say uniformly distributed load.

Let us say: that is q we can say let us take centre to centre distance or the effective span whatever we call it this is l_x and the other side we are having l_y . So, we can get that l_x and l_y and then the total uniformly distributed load over this q ; that means, you say 4 kilo Newton per square meter and including you say also that 1 we can take. So, when you having this 1 we can say we have to find out that how much is q_x ; that means, q_x and how much you will be you say q_y that q_x ; that means, if I take a stripe say this is your 1 stripe may be 1 meter width the other 1 let us take around the y .

So, this 1 also will be bend the other 1 also will bend. Now, what we can find out that we can find out at this point the vertical deflection. So, since there is no stripes if I take this stripe that; as if 1 plate the other side also 1 another plate. So, the deflection here we shall get it whatever deflection, we shall get it the deflection will be the same taking this 1 and taking other another one. So, this is our consideration and from that point of view we can find out the distribution of q_x and q_y .

We can take q_x and q_y on the basis of that, we have taking that q due to q_x what is the vertical deflection here and due to q_y say what will be the deflection and that; 1 should be equal, because there is no difference say otherwise both of them deform the same time and there is no disconnection. So, that is why you can take that 1 displacement same the vertical displacement same that is our assumption.

(Refer Slide Time: 15:58)



So, we can write down. So, if this is your l_x . So, as if we are having 1 beam or whatever we consider a plate q_x and this is the span that 1 we are getting say your l_x . So, you can write down the displacement $\frac{5}{384} q l_x$ to the power 4 by EI this is your that vertical displacement. Similarly, this is shorter deduction. The other 1 longer deduction in the similar fashion you can write down. So, this side you have the UDL q_y and we have the vertical deflection and this is your l_y .

So, these deflection will be equal to $\frac{5}{384}$; sorry here, I have followed to write down this is $q_x l_x$ to the power 4 by EI and this 1 will be $q_y l_y$ to the power 4 by EI and this 2 should be equal. So, $\frac{5}{384} q_x l_x$ to the power 4 by EI and five by 384 $q_y l_y$ to the power 4 by EI that also should be equal since, there is no disconnection on the slab. So, you can write down $\frac{5}{384} q_x l_x$ to the power 4 equal to $\frac{5}{384} q_y l_y$ to the power 4 or $q_x l_x$ to the power 4 equal to $q_y l_y$ to the power 4.

So, 2 way we can write down here that q_y equal to q_y equal to l_x by l_y to the power 4 q_x and the other 1 q_x equal to l_y by l_x to the power 4 q_y . This is only writing and we have 1 more equation that is q equal to q_x plus q_y . So, we can write down here that is q this should be equal q equal to q_x plus q_y .

(Refer Slide Time: 19:57)

$$\begin{aligned}
 q_x \left(1 + \frac{l_x^4}{l_y^4} \right) &= q \\
 q_x &= q \left(\frac{l_y^4}{l_x^4 + l_y^4} \right) \\
 \therefore q_x &= q \left[\frac{(l_y/l_x)^4}{1 + (l_y/l_x)^4} \right] \\
 M_x &= \frac{q_x l_x^2}{8} = q \left[\frac{(l_y/l_x)^4}{1 + (l_y/l_x)^4} \right] \cdot \frac{1}{8} l_x^2 \\
 M_x &= \beta_x q l_x^2
 \end{aligned}$$

So, we write down here q_x plus l_x by l_y to the power 4 q_x equal to q or q_x 1 plus l_x to the power 4 by l_y to the power 4 equal to q therefore, q_x equal to q times l_y to the power 4 by l_x to the power 4 plus l_y to the power 4 equals q l_y by l_x to the power 4 divided by 1 plus l_y by l_x to the power 4.

So, this is your that equation q_x . So, you can write down this 1 depending on the l_y by l_x we can find out what will be the contribution of q_x here. Similarly, we can write down the other portion also. Similarly, we can write down the other portion also that what is the how much will be that q_y . So, we can write down and we can actually, we can make it. Let us come previous 1 that moment m_x equal to q_x 1 square x by 8 equals q times we can write down the whole thing.

So, here we can write down q times l_y by l_x to the power 4 by 1 plus l_y by l_x to the power 4 into 1 by 8 1 square x equal to we can write down as β_x times q 1 square x β_x means; taking care of whole thing including 1 by 8. So, that we can take it and that is β_x and q 1 square x . So, m_x will be equal to β_x and q 1 square x . So, m_x will be equal to β_x q 1 square x , please note we have written in this fashion. The other 1 you have to find out the moment. We are taking care moment bending moment due to this q which can get 1 coefficient called x . So, that 1 will be equal to m_x equal to β_x q 1 square x .

(Refer Slide Time: 22:45)

$$\begin{aligned}
 q &= q_x + q_y \\
 \left(\frac{l_y}{l_x}\right)^4 q_y + q_y &= q \\
 \text{or } q_y \left(\frac{l_y^4}{l_x^4} + 1 \right) &= q \\
 \text{or } q_y \left(\frac{l_y^4 + l_x^4}{l_x^4} \right) &= q \\
 \text{or } q_y &= \left(\frac{l_x^4}{l_x^4 + l_y^4} \right) q
 \end{aligned}$$

The same fashion we can write down here again; q equal to q_x plus q_y or we can write down here that l_y by l_x^4 q_y plus q_y q_x equal to this much equal to q or we can write down as q_y equal to sorry, I think I can make it here like this l_y to the power 4 by l_x to the power 4 plus 1 equal to q or q_y l_y to the power 4 plus l_x to the power 4 by l_x to the power 4 equal to q or q_y equal to l_x to the power 4 by l_x to the power 4 plus l_y to the power 4 times q . So, this is your q that we can get it so; obviously, this 1 will be l_x to the power 4 by l_x to the power 4 plus l_y to the power 4 this much we can get it.

(Refer Slide Time: 24:17)

$$\begin{aligned}
 M_y &= \frac{q_y l_y^2}{8} \quad \text{for simply supported case in the longer direction} \\
 &= \frac{1}{8} \left(\frac{l_x^4}{l_x^4 + l_y^4} \right) q l_y^2 \\
 &= \frac{1}{8} \left[\frac{1}{1 + \left(\frac{l_y}{l_x}\right)^4} \right] q l_y^2 \\
 M_y &= \frac{1}{8} \left[\frac{1}{1 + \left(\frac{l_y}{l_x}\right)^4} \right] \frac{l_y^2}{l_x^2} \cdot q \cdot l_x^2
 \end{aligned}$$

So, what about your m_y then. So, we can write down here m_y will be equal to $q_y l_y^2$ by 8 that is due to simply supported case. For simply supported case, we are getting

this much the qy that is the contribution that is the 1 that; we shall get it in the qy side. So, that is for simply supported case in the longer direction. So, you can write down here 1 by 8 l_x to the power 4 by l_x to the power 4 plus l_y to the power 4 times q times l square y equals.

We can write down here as 1 by 8 1 divided by 1 plus l_y by l_x to the power 4 times q l square y 1 by 8 1 by 1 plus l_y by l_x to the power 4 q l_y square just, I have divided by l_x to the power 4; which we can further write down as 1 by 8 what I shall do it here. Let us divided by l_x square and times l_x so, what shall I do it here 1 by 1 plus l_y by l_x to the power 4 times l_y square by l square x times q times l square x , I shall write down this 1 as because I know everything we always mention here that l_y by l_x since why mention it l_y by l_x .

(Refer Slide Time: 26:35)

The image shows handwritten mathematical derivations for the moments M_y and M_x on a grid background. The equations are as follows:

$$M_y = \frac{1}{8} \left[\frac{\left(\frac{l_y}{l_x}\right)^2}{1 + \left(\frac{l_y}{l_x}\right)^4} \right] \cdot q l_x^2$$

$$\boxed{M_y = \beta_y q l_x^2}$$

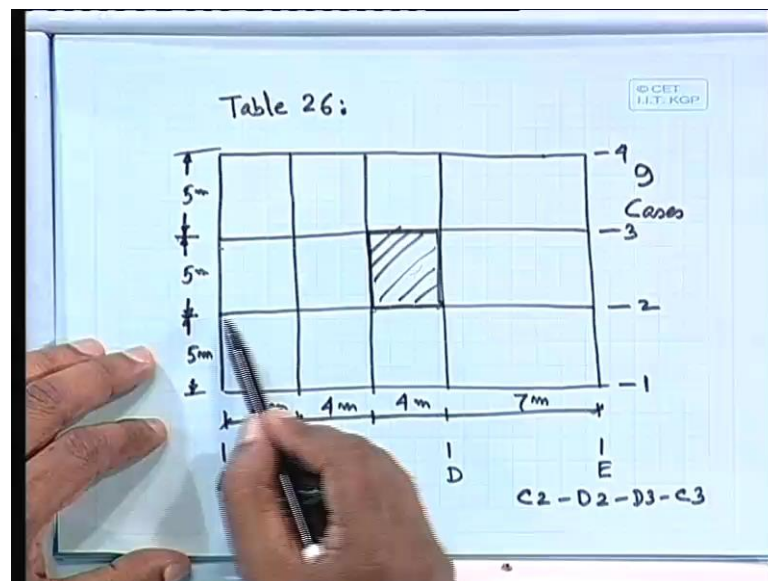
$$M_x = \frac{1}{8} \left[\frac{\left(\frac{l_y}{l_x}\right)^4}{1 + \left(\frac{l_y}{l_x}\right)^4} \right] q l_x^2$$

$$\boxed{M_x = \beta_x q l_x^2}$$

So, we shall write down that m_y that 1 will be equal to times q l square x . So, what we can do it here then; we can get it m_y equal to 1 by 8 l_y by l_x whole square divided by 1 plus l_y by l_x to the power 4 times q l square x . So, we can write down as m_y equal to $\beta_y q$ l square x . So, please note if I write down the same m_x then; it will be clear. So, m_x equal to we have written as 1 by 8 l_y by l_x to the power please note, but, has square 1 plus l_y by l_x to the power 4 times q l square x and which; we have written as $\beta_x q$ l square x .

So, you are getting 2 equations in no case we are interested to get that l_y . So, please note when you are doing your computation for moment, because you are interested to find out the moment only m_x and m_y for simply supported case. So, what we shall do it here we have to get β_x and β_y we shall get β_x and β_y and times $q l^2$ square x ; when you are calculating m_y and similarly, when you are calculating m_x that time also it is $q l^2$ square x please note that it is not that l_y square and l_x square generally you that do that mistake.

(Refer Slide Time: 28:38)



But, please note that in both the cases; we are taking the $q l^2$ square x and β_x and β_y that they are coefficients and these coefficients also you need not calculate again; what you have to do they are those coefficients will get it in table 27 is 4 5 6 2000. So, you can get that 1 table just to mention this table means; bending movement, coefficients for slabs spanning in 2 directions at right angles that these are not 29.29 right angles and simply supported on 4 sides.

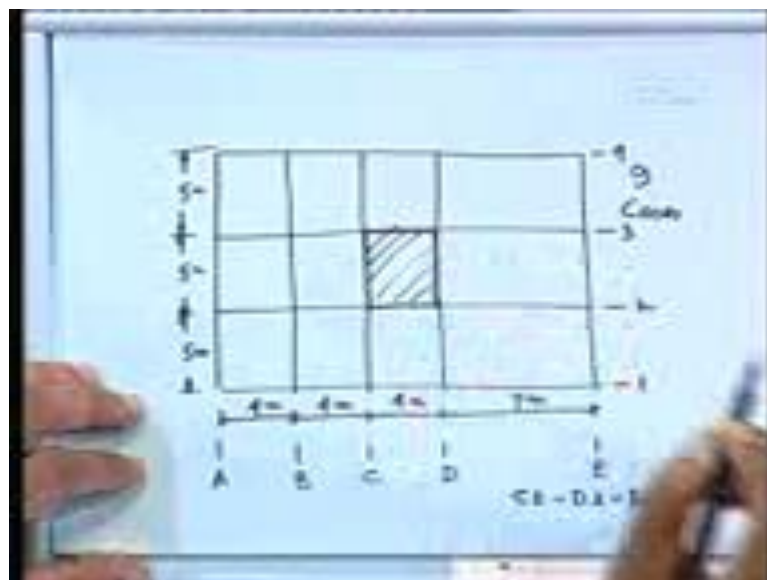
So, this is your table while we shall get for different value of l_y by l_x just for your difference for different value of l_y by l_x . So, this is 1.0 just I shall give you 1.1 1.2 then 1.3. Let us take and 1.4 since, I cannot accommodate in this paper and instead of telling β_x and β_y they have written as α_x and α_y the same thing only, if that in the table they have retained that α_x α_y here we have written as β_x and β_y

the same thing please note $\alpha \times \alpha$ here $\beta \times \beta$ that are the same thing in this case.

We are getting those values 0.62; obviously, it should be same, because when you are getting $l_y \times l_x$ same. So, you should have equal say shear so, then 0.074. and you see this is decreasing then; 0.084 0.05 0.09 3.055 then; 0.099 and 0.051 it goes like that it goes in the for simply supported case. We go little more and it goes that l_y by l_x our quote says you can go upto 3 l_y by l_x you can go up to 3 and that 1 is available in this particular table, but, where as for other cases we shall go up to 2.

So, what are those other cases I shall explain now. So, this is your case but, generally you need not memorize all those things, but. So, because whenever you design; obviously, we consult these is 456. So, those values you will get or it will be supplied, but for Reference that you should note that, while that is available, in table 27 is 456 2000. So, next we can come I think before going for any example I think that I should say the other cases what are the other cases 1 is simply supported case and what about the other cases. The other cases we can say I have already told.

(Refer Slide Time: 32:15)



Let us say this is the plan of the building and it can happen; these are all beams I can just simply keep it like this also we can have certain. Let us give certain dimension maybe I can write down. This 1 is a 4 meter this 1 is a 7 meter this 1; I can write down say 5 meter and thus yes this is also has a 5 meter this is 5 meter our quote says that we can

have 9 different cases according to the panel what are those panels. There should be 9 different cases 9 cases depending on the edge boundary.

If you take this panel because each and individual; that means, these are separated by beams or in other way these slab is supported over by beam and when it is supported then each and individual 1; that means, these are all panel we call it panel different and that you have to design separately, if I take this case it means; that left side of this panel you have left side of this panel you have another slab. This side also you have another slab this side also you have another slab this side also that means, all sides are continuous to be more specific.

Let us give certain say your panel we generally, specify like this a b c d e depending on the column position or some other break or some other bend we generally, give this type of thing and this side we shall write down say 1 2 3 and 4. So, now what we can say; that means, this panel what is this panel this pane is c c2 d2 d3 c3 this panel I am talking. So, this panel you can refer it as because you are starting here so, c 2 d 2 d 3 and c3.

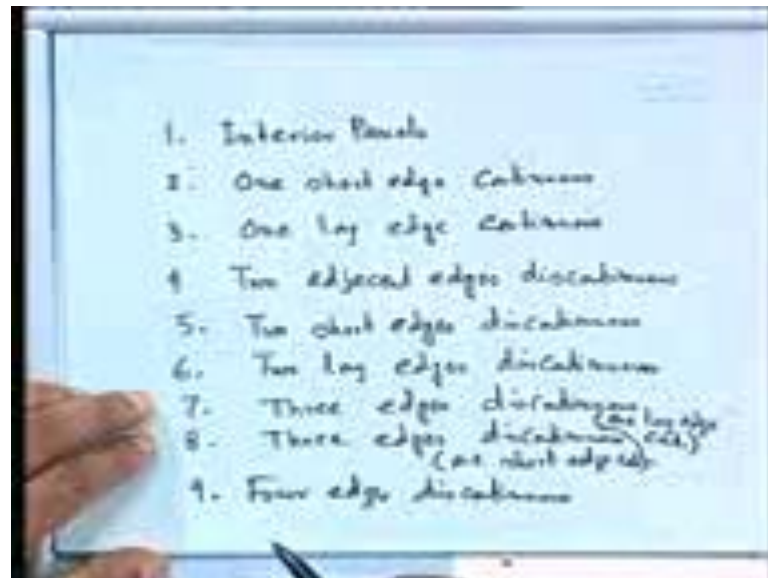
So, whenever you will design that in your design calculation that is it means; that panel c2 d2 d3 c3 that you are doing and for that what you have to do in our simply supported case only we have 2 values those are span that means; that is a span movement we have to calculate span movement or any have. So far whatever, you have done and that is why you are having 2 values here 1 this side 1 and the other side 1 that α_x α_y you are getting here that is 1 along this another 1 this and that those are span movements. But whereas, in this case depending on the continuity we can have 4 different values 1 value will be in the span.

So, shorter direction in the span as well as in the support longer duration in the span as well as in the support; that means, we can have 4 different values and those 4 values that those 4 values we have to take and then on the basis of that we can calculate that your bending movement. So, bending movement at span at support, in the shorter direction bending movement at span and support in the longer direction and we have to check our calculation we have to check out design on the basis of that means, the movement direction be different values of moment.

So, we shall take the maximum movement out of 4 then we shall get the depth and then you provide the reinforcement for other values. So, this is your general procedure to

provide the reinforcement. So, what are the different cases then; those things we shall get it for different cases that is in table 26; the last 1 that you have told table 27 this is table 26. So, that 1 it says that bending movement coefficients for rectangular panels supported on 4 sides with provision for torsion at corners. So, that is the table it says and here we have first case that is your interior panels.

(Refer Slide Time: 37:50)



So, I can write down this 1 as case 1 that is your interior panels. So far interior panels we are having depending on the l_y by l_x we can find out those different values of α_x and α_y in support as well as in span number 2: we have 1 short edge continuous, number 3: long edge continuous, number 4: 2 adjacent edges discontinuous, number 5: 2 short edges discontinuous, number 6: 2 long edges discontinuous, number 7: 3 edges, number 8: also let us write down I shall tell you what is the difference 3 edges discontinuous; what are the difference number 7: 3 edges discontinuous, number 4: 8 also 3 edges.

First 1 the seventh 1 long edge continuous whereas, in this case 1 short edge continuous that is possible and number 9: 4 edges discontinuous. So, these are the 9 different cases possible. So, now, let us come that the figure which we have drawn in the last 1 in the interior panel. So; obviously, this is your interior panels whether we can have 1 short edge continuous here 1 short edge continuous so; that means, all other here so; that

means, here in this case we can have this panel this is your shorter edge. So, that is continuous.

Similarly, we can have 1 long edge continuous 1 long edge continuous it means that this is your longer edge and this shorter edge. So, this edge is continuous so; that means, this is your another panel this is your another case now, 2 adjacent edges discontinuous. So, adjacent edges discontinuous it means; this corner 2 short edges discontinuous that case you will not get it here 2 short edges discontinued; that means, that I can draw that figure other way; that means, it can happen something like this.

So, if this is your that panel. So, your case 2 short edges discontinuous it means; as if that we are having this type of thing this edge other way this edge discontinuous. So, longer edge that is continuous. So, depending on the situation you can get similarly, you can get 2 long edges discontinuous like that 3 edges discontinuous that is also possible only it is 1 side only continuous and 3 edges discontinuous in the shorter side as well as longer side that 2 different cases and 4 edges discontinuous that is also possible not the simply supported 1. We want to say not the simply supported 1.

So, these values that 1 can find out from the period of elasticity that you would say plate bending from there also 1 can find out those values that also 1 can do it and what; we do generally do not go to that case. We simply take that table I have told already that table 26 where all the cases are given table 26 is 456 2000. So, you can get all the values from there depending on the l_y by l_x just for 1 sample, I think I should tell for your sample how it comes in the similar fashion. For interior panels itself, I can tell for others you can find out.

(Refer Slide Time: 43:32)

Interior Panels for shorter span				for longer span
Span	1.1	1.2	1.3	
Negative moment at continuous edge	0.037	0.043	0.047	0.032
Positive moment at midspan	0.028	0.032	0.036	0.024

Table 26
IS 456:2000

$$M_x = \alpha_x w l_x^2$$

$$M_y = \alpha_y w l_y^2$$

Span
Support

Span
Support

So, it says like this negative movement at continuous edge and positive movement at mid span. So, we can get say let us write down here say 1 it starts from 1.0, but, latter me write down here say 1.1 by 1.1 1.2. Let us write down and we are getting here 0.037 this 1 0.028 0.043 0.0321 more; I can write down 1.3; for that 0.047 0.036 but, this is for shorter span. These values for shorter span but, for longer span we shall get for all by by 1x that we shall take the same value 0.032 0.024; that means, we have to for shorter span only we shall get the variation depending on by 1x but, for longer span we shall get the same value.

So, that is 0.032 0.024 for interior panel we shall take it. So, what we shall calculate we shall calculate by 1x; we shall calculate and then we shall find out it in which interval it is coming whether, it is coming say it may happen it is 1.15 it may happen 1.25. So, that way what we shall do that we shall find out at what interval it is coming which interval and then just simply you shall take linear interpolation and we shall get that value α_x and α_y .

So, you shall get from here out of this 1 I shall get 1 α_y for the shorter span and α_x and α_y α_x that this is for α_x for span and support α_y for span. And support and then from there you shall get that moment; which will be equal to say M_x equal to $\alpha_x w l_x^2$ and please, note we shall write down in this fashion α is $w l^2$ and M_y equal $\alpha_y w l_y^2$. So, not the by so, we shall always write

down l_x and this is we shall get α_x for span and support similarly, α_y also for span and support.

So, we shall get 4 different values depending on the l_y by l_x and that we shall get it from table 26 please note, this is I have refer repeating number of times just to remind you that this is the table from where we shall get those values. These values I can calculate I can say when I am doing let us say: computed design the 1 alternative could be in initial stage 1 attended could be that I can simply put that table and then; we can find out at what interval that l_y l_x coming and then we can do the problem.

But, I shall show you that another example seeing, in the next class where you shall take it as say your concentrated load the 1 I have told it in the simply supported case similarly, for this 2 way slab also concentrated load; which is applicable for say wheel load and for bridges and they are we does require those calculation; that means, how to find out that you say α_x α_y all those things, but, they are what I have done I have directly calculated using the formula not the 1 that using table.

That is also possible 1 can take it as a table and showed it in the computed and then from there 1 can find out that 1 just 1 linear interpolation. The other alternative also since these values is calculated based on a some equation. So, 1 can directly 1 can go because, after all it will not take much time that the time it will take for your computation by calculation and the time it will take for direct comes as you for almost the same. So, I need no timing to find out no time it is computed. So, 1 can try that 1 also.

So, this is the thing. So, at least in this particular class and we see that the very beginners one. So, that is why you shall not go in detail of that how to calculate. We shall let us take it granted that these values are correct and we shall use these values from the table only that table 26 but, in higher class if you are interested if you would like to modify those because, 1 can argue that what is the value I can say 0.037.

Let us say this is 0.037. So, if I find out 0.037 what we can do. So, if I take that how much is the percentage w_l square by 8.

(Refer Slide Time: 49:30)



Let us take this way that 1 example before going to the 1 numerical example I can try this 1. We know few things and this 1 should be your basis 1 is that simply supported. So, your span movement and in our case we are taking. So, w_1 square by 8 because, we are very familiar with this 1 say possibly another 1, I can consider fixed end and here the support movement w_1 square by 12; that means, these are the values what you get it w_1 square by 8 in 1 end.

That is in the higher side or w_1 square by 12 that 1 you are getting that is other side of value even in we can get w_1 square by 24 also like that. Now, when you are talking say certain value say m_x equal to 0.037. Let us say w_1 square or l_x or l square. So, if I write down the value we can get these value as. So, you are getting that value w_1 square I want to say I, would like to like write down in this fashion because I am familiar I say which is coming 27 0.027.

Let us write down 20 w_1 square by 27 that means; you can find out that how much is the movement the w_1 square by 8 here and here you are getting w_1 square by 27. So, like that you can get it say I could find out few values; which is coming say your that 1 value I am getting say 0.107 0.1 like that. So, when you are getting 0.017 which is coming as 9.3; that means, w_1 square by 9.3; what I want to say the value whatever you are getting you can comparing your numerical value.

Since, you are similar with these say wl square by 8 or wl square by 12 or wl square by 24 or wl square by nine or the other 1 that we mention, in the table 12 and table 13 table 12 of is 456 or it is given that you say different that values in that fashion 1 by 8 1 by 12 1 by 10 like that. So, you can compare that in the plate problem how much actually you have that value that what is the value, because some have we are not familiar with this 1.037 or 0.024 like that what you are very familiar with this 1 by 8 1 by 12., because it will give you some idea and on the basis of that only, you can find out whether your values are correct or not.

The design is such the reinforced concrete design is such that when you are doing that problem you should not do it blindly that your values that when it is coming that; it will automatically it will stop you that whether, you are doing it right in the right direction or wrong direction and that feeling you should have say for example. We generally now a days at least we are able to do that whether, say just looking the particular as building whether, that column with it is not say whether, it is economic or that whether it is optimized or it is simply given like that those all those things you find out.

If you see that fortunately there are so, much construction going on in different even if you walk you will find out different construction. If you now, look that you said reinforcement at the bottom that show the beam top reinforcement of the beam or say slab or columns you will find out say. They are either giving say 60 millimeter dia bar 12 millimeter dia bar like that and from there also you can find out upper the construction, when you see the building this is the building that 2 building or 3 building for this they are provided these reinforcement.

It will help you to understand the problem and because, otherwise in the very beginning since. So, far I think that in the last test only you have done the first problem in reinforced concrete design on your own 1 which; I hope that you are not completed fully possibly few of you. So, any way whatever it is. So, if you do more than more problems than; only will be able to solve and then only, it will be able to solve and then only, you will have an aptitude in reinforced concrete design.

Let us stop it here this one. So, I shall we shall continue with, the next class, with you that example.

Thank you.