

Geotechnical Measurements and Explorations.

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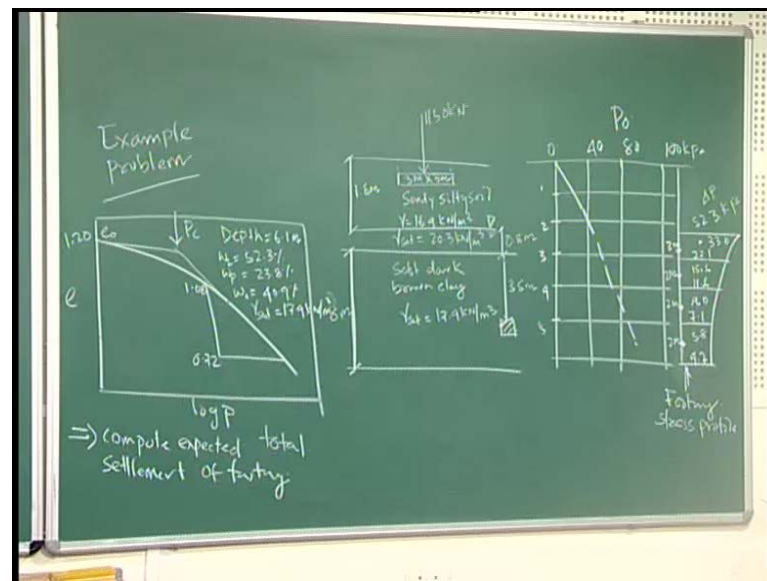
Department of Civil Engineering

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Module No. # 01

Lecture No. # 04

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Today we will solve one example problem on consolidation. So, sandy silty this is your 1.8 meter, then this is water table is there, 0.8 meter γ_{sat} is equal to 20.3 kilo Newton per meter cube. This is eight meter soft dark brown clay γ_{sat} is equal to 17.9 kilo Newton per meter cube and at a depth 3.5 meter the soil has been collected. Now, this is a typical result of 1 2 3 4 5 this is 40 80 100 k p a. Now, it will start at zero then this is 33.0 15.6 9.0 and then this is 5.8 footing stress profile. Another part is given, this is your consolidation problem. Consolidation means set consolidation diagram is it is given. Now, e_0 is equal to 1.20. Now, this is your p c e versus $\log p$, this is a 1.08, this is your 0.72.

Now, the question is compute expected total settlement of footing. Now, this is a case where this footing is of size three meter by three meter. So, acted upon this footing is 11.50 kilo Newton and this soil consist of first 1.8 meter is sandy silty soil and water table is located below the ground surface at 1.8 meter, this is your water table and

gamma saturated is given. Then below 1.8 plus .8 below this **this** entire depth is your eight meter. Total is your eight meter. This depth is your eight meter and it is a soft dark and brown clay and soil sample has taken at three meter from here. 3.5 meter from here so gamma saturated is equal to 17.9 kilo Newton per meter cube.

This has been soil sample has been taken here and this is given expected means earth pressure means **sorry** it is not earth pressure, this is p zero. Variation of p zero calculated or may it is given. Then footing stress profile this is your footing stress profile. Below the footing it is called delta p and this is your k p a. Every two, two meter interval it is given so at the surface it is 52.3, at below two meter it is 22.1, then below two meter it is 11.6, then two meter it is 7.1, then two meter it is 4.7, then the middle at the middle this stress has been also been given. At middle it is 33.0, here at middle at 15.6, here at middle is 9.0, here at middle is 5.8. Also, the consolidation graph is given. With this consolidation graph e zero got it 1.20 p c also got it and for depth 6.1. How come depth has come 6.1? 3.5 plus .8 4.3 plus 1.8. So, it will be 6.1. From here to here is 6.1. Liquid limit is 52.3, a plasticity index is 23.8 and natural moisture content is 40.9 percent and gamma **gamma** saturated is 17.9 kilo Newton per kilometer cube. So the question has been asked compute the expected settlement of footing because of your soft brown clay.

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$$\gamma_{sat} = \frac{113 + 276}{141.13} (9507)$$

$$= 17.9 \text{ kN/m}^3$$

Effective overburden pressure P_o'
 $y = m$

Point	$y = m$	P_o'
1	$y = 2.6$	$(18)16.9 + 0.8(20.3 - 16.9) = 38.8 \text{ kPa}$
2	3.6	$38.8 + 1(17.9 - 9.6) = 46.9$
3	5.6	$46.9 + 2(8.1) = 63.1$
4	7.6	$63.1 + 2(8.1) = 79.3$
5	9.6	$79.3 + 2(8.1) = 95.5$
Bottom	10.6	$95.5 + 1(8.1) = 103.6$

$C_c = 0.009(W_L - 10)$
 $= 0.38$
 e_0 (from graph) = 1.13
 $e_0 = W_n G_s$
 $G_s = \frac{1.13}{0.409} = 2.76$

$V_p = 1.13$
 $V_s = 1.0$
 $V_v = 2.1$

Now C_c from co-relation, $C_c = 0.009$ into W_L minus ten so which is equal to 0.38, e_0 from graph, 1.13. So, e_0 is equal to $W_n G_s$. So, G_s is equal to 1.13 by 0.409

which is equal to 2.76. Then taking a block diagram, this is a block diagram. v_v is equal to 1.13, v_s is equal to 1.0, w_s is equal to 2.76 and one plus 1.13 w_f is equal to 2.76 plus 1.13. Taking block diagram where v_s is equal to assumption is volume of solid is equal to 1.0. So, from there you can calculate gamma saturated is equal to 1.13 plus 2.76 divided by one plus 1.13 into 9.807 which is equal to 17.9 kilo Newton per meter cube.

Now what is this 1.13? 1.13 is your e_0 . $e_0 g$ plus e_0 gamma saturated g plus $e_0 g$ is equal to 2.76 by one plus e_0 into gamma w . This is your gamma saturated. Now once you get gamma saturated you can find it out what is your effective stress over burden pressure, effective over burden pressure p_0' . **effective over burden pressure p_0'** .

So now, this point and the y is equal to meter p_0 . So, this will be 1 2 3 4 5 and bottom so y is equal to 2.6 meter. We will start with this p_0' effective stress 1.8. This will be 2.6 y is equal 2.6. This layer has been divided **divided** into five parts, you see from here we will start it now. At y is equal to 2.6 p_0' is equal to 1.8 into 16.9 plus 0.8 into 20.3 minus 9.8 which is equal to 38.8 k p a. So, this is giving 1.8 into 16.9 then this water table is at below 1.8. So, it is coming 0.8 **0.8** meter into this is your gamma saturated. So here it will be gamma submerged because water table is lying. Gamma submerged is equal to gamma saturated minus gamma water, gamma w . So 20.3 minus 9.8 this will give 38.8.

Now second point is your 3.6 so it will be 38.8 plus one into 17.9 minus 9.8 which is equal to 46.9. Now, every one meter, one meter interval we are finding now here from here to ground surface to here is 2.6 meter then you go upto 1 meter. So, here it will be here it will be 3.6. This is your 1 meter 3.6. With this 3.6 we got it 38.8 one meter. This is your height meter, one meter into 17.9 minus 9.8. If you look at here, here it is gamma saturated is given. 17.9 then minus 9.8. It is your 46.9. Then third part is your 5.6 so it will be 46.9 plus two into 8.1 which is equal to 63.1, then fourth is your 7.6. So, it will be 63.1. Similarly you can calculate two into 8.1 which is equal to 79.3. These are all k p a's. So, fifth layer is 9.6 which is equal to 79.3 plus two into 8.1 which is equal to 95.5 and bottom is equal to 10.6. So, this will be 95.5 plus one into 8.1 which is equal to 103.6. So, these are the values effective over burden pressure p_0' , we got it point wise. It is given one two three four five bottom.

So if you look at here; this has been divided because this settlement has to be calculated in this clay layer. So, this clay layer has been divided into one meter one meter one meter sub layers. So, for each one meter the p_0 has been calculated. p_0 is effective overburden pressure or p_0' we write it effective once it is effective it should be p_0' and in this case also it will be p_0' . So now, the first calculation I have shown in detail. Now, this second third fourth fifth as per this calculation has been made. Below water table it will be $\gamma_{submerged}$, above water table it will be γ_{bulk} . Now once you get it your p_0 effective overburden pressure versus depth you can plot it there and the plotting has been made there effective overburden pressure versus your depth. This plot already I have shown. This plot has come from these calculations **from this calculations**.

Now the second stage will be once you get it e_0 , first step is find it out e_0 c_c then g_s then $\gamma_{saturated}$. Here $\gamma_{saturated}$ is not given at this point $\gamma_{saturated}$ initially it is not given. So, once you get it from here $\gamma_{saturated}$ we got it from this. This is your e_0 this is your e_0 plus g_s by one plus e_0 into γ_w . You will get your $\gamma_{saturated}$ once you get the $\gamma_{saturated}$ the profile, all the profiles γ , $\gamma_{saturated}$ and bottom of this soil is your clay is your $\gamma_{saturated}$ it is given. All the profiles have been given so effective overburden pressure p_0' you can calculate from here.

So this will be like you see, this **this** is like, this is the only unknown, $\gamma_{saturated}$ here it is the only unknown. So, which is required for calculation of p_0 this $\gamma_{saturated}$ came from your e versus $\log p$. **from your** From here you can calculate you c_c e_0 , you can calculate from here c_c from your empirical co-relation, you can get it. Then once you get it then you can find it out e_0 is equal w_n into g_s natural moisture content is given from there. You find it out g_s , once you get g_s then you go for calculation of $\gamma_{saturated}$. Once you get $\gamma_{saturated}$ then, you calculate your effective overburden pressure. So this part is over, effective overburden pressure layer wise we have calculated.

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$C_c' = \frac{C_c}{1 + e_0} = 0.17$
 $\gamma_{sat} = \left(\frac{e_0 \gamma_w}{1 + e_0} \right)$
 $e_0 = W_n \gamma_w = 2.76$

Point	H	P ₀	ΔP	Σxi	Δhi
1	—	38.8	52.3	0.063	
2	2m	46.9	33.0	0.039	0.078
3	2m	63.1	15.6	0.016	0.032
4	2m	79.3	9.0	0.008	0.016
5	2m	95.5	5.8	0.004	0.008
Bottom	—	103.6	4.7	0.003	
					ΣΔhi = ΔH = 0.134 m

$\Sigma_1 = C_c' \log \frac{P_0 + \Delta P}{P_0}$
 $= 0.17 \times \log \frac{38.8 + 52.3}{38.8}$
 $= 0.063 \text{ m/m}$
 $\Sigma_2 = 0.17 \times \log \frac{46.9 + 33.0}{46.9}$
 $= 0.039$
 $\Delta H_i = H_i (\Sigma_i)$

Now, C_c' is equal to C_c by one plus e_0 . From there C_c' is equal to 0.17. Now, you will calculate obtain Δp increase in stress increase in stress is already given below the footing **below the footing**. Now, it has been calculated based on Boussinesq's theory. **it has been calculated based on Boussini's method based on Boussini's methods Boussini's method.**

Now you can calculate the settlement. Here this is in tabular form, this is your point and this is your H_i . So, this will be one two three four five then bottom, then H_i , this is blank, this is two meter, this is again two meter, this is two meter, this is two meter and this will be blank. So, this is your P_0 then Δp then Σx_i then ΔH_i . Now first layer P_0 is we have already calculated 38.8, second is your already we have shown it 46.9 third is 63.1, 79.3, 95.5 then 103.6, then Δp is equal to 52.3, 33.0, it will be 15.6 then 9.0 5.8 4.7. This increase in stress due to Boussinesq's equation, Boussinesq's method it has been calculated and shown here or particularly here two, two meter it has been shown here 52.3, then at the center it is 33, then at the center it is 15.6, then here at the center it is 9.0, then here at the center it is 5.8, then bottom it is 4.7.

Now Σx_i , you can calculate this is your strain C_c' $\log P_0 + \Delta p$ by P_0 so which is equal to 0.17 into $\log 38.8 + 52.3$ divided by 38.8 which is equal to 0.063. Now Σx_i is equal to also 0.17 into $\log 46.9 + 33.0$ divided by 46.9 which is equal to 0.039. The typical calculation is given for Σx_1 Σx_2 Σx_3 Σx_4 Σx_5 Σx_6

six. If you look at here, this train one means at .1, first layers train at .1 so it is c_c prime $\log p_0 + \Delta p$ by p_0 . So this will be 0.17 into $\log 38.8$ and the plus 52.3 divided by 38.3. This will be 0.063 **zero six three**.

Similarly for x_2 , for second point 0.17 $\log$ if you look at 46.9 plus 33.0, so it will be 46.9 increase in stress is your 33.0 divided by 46.9. So, this will be p_0' , $p_0' + \Delta p$ divided by p_0' . So p_0' is **p_0'** forty-six p_0' is your in this case second case is 46.9. That means increase in stress. First case it is 38.8, so it is 38.8, so increase in stress by means of Boussinesq's; it is 52.3. It will be 52.3. So now this values I have written 0.063, this is a typical calculation. So, 0.039, then 0.016, 0.008, then 0.004, then 0.003. Now, increase in means particularly change in height **change in height** will come from your strain. So, this will be this minus this. So, it will be coming 0.078, then this is coming 0.032 0.016 0.008 **sorry** this Δh is coming **Δh is coming** height h_1 into x_2 or x_1 . Δh_1 is equal to height h_1 into x_1 .

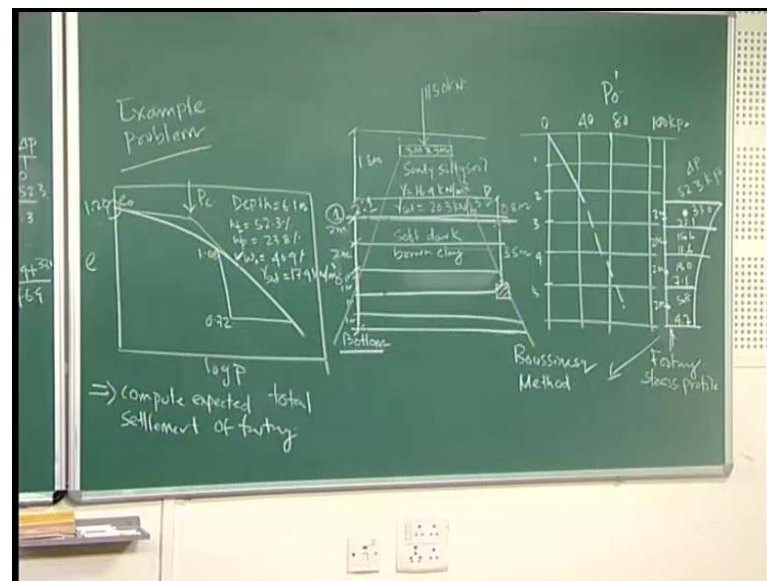
So change in height Δh is equal to what is your height? H_1 is equal to that is your two meter into x_1 how much is your x_1 ? 0.63 So, from there it is coming 0.078 0.032 0.016 then 0.008. Now, with this Δh is equal to Δh which is equal to 0.134 meter. This is nothing but, Δh change in height is nothing but, a change in this is nothing but, your total settlement. Total settlement, expected settlement is equal to 0.13 meter below the footing. So this problem is a kind of solved one. Example problem, how the consolidation **how the consolidation** is going to be used for particularly settlement calculations. If you look at this consolidation particularly used for settlement calculations initially this e versus change in height versus $\log p$ is given from this because this soil sample is taken at a distance 3.5 meter below particularly this layer of soil. Once soil has been taken and consolidation **consolidation** test has been done this graph obtained. With this graph once again you can get it your e_0 value. You can get it e_0 as **as** well as also you can find it out what is your also c_c . c_c you can get it either you get it c_c from here or you can get c_c from empirical co-relation.

So here c_c we got it from here. I have written earlier c_c by means of empirical co-relation. With this you can get it also empirical co-relation if it is not given 0.009 into w_L minus ten. If this is not given this e versus $\log p$ is not given then you can get it here if it is given do not use, you can get it from your slope this is called virgin curve from this

slope you can get it. What is your c_c ? Once you get c_c e_0 you get it then, c_c you got it. Once you got e_0 and c_c you can find it out your g_s . You see e_0 is equal to w_n into g_s specific g_s .

So this e_0 you got it from here, natural moisture content of this soil at this depth it is given w_n is given. So, from there you can find it out g_s . So, g_s is coming 2.76 **two point seven six**. Once you get the g_s from here, then your next step is initially the gamma is not given. Initially gamma saturated was not given so, with this value **with this value** e_0 g_s with this value you have to calculate your gamma saturated. So then, what is your gamma saturated? Your gamma saturated is equal to this is your e_0 plus g_s by one plus e_0 into gamma w . This is your gamma saturated you can calculate. Once you get your gamma saturated this comes from particularly e_0 g_s gamma saturated whatever the available data for this consolidation test from there and c_c you can find it out consolidation. Once you get it gamma saturated, then find it out second step is your find it out p_0' over burden pressure at this from here to here.

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So that means p_0' I have shown the calculations particularly here because this is your clay layer where this consolidation settlement is going to happen. So, this means this clay layer has been divided into equal number of layers of one, one meter **of one meter**. With this one meter at this, what is your p_0' ? If you look at the calculation p_0' has been done this is your particularly this, this is above this soil

has no water table effects. So, that is why 1.8 into gamma 16.9 and below this 1.8 there is your water table. So once water table is there, you have to use submerged unit weight. So that means your gamma saturated is given 20.3. So, submerged is equal to gamma saturated minus gamma w you calculate particularly this layer **this layer this layer** and you proceed.

So, this is your typical plot I have shown this plot variation of p_0 versus the depth how this effective over burden pressure is varying. Once you get effective over burden pressure next step for settlement calculation you have to find it out increase in stress Δp . So, Δp increase in stress Δp from Boussinesq's theory you can get it. Either Boussinesq's theory you can get it or by means of two is to one distribution or by means of **two is to one distribution two is to one distribution** you can get it.

So here, because this is a lower point load acted upon this footing; Boussinesq's theory has been used to find it out increase in stress because of footing in this clay layer. Because consolidation settlement is going to occur in this clay layer so increase in stress because of footing in this clay layer so this has been calculated so every two, two meter interval how much is your increase in stress? So, if this is the two meter interval so increase in stress here 52.3 that means increase in stress here is 52.3 then two meter interval 22.1. So, we are going to calculate the settlement at the center of the layer so that means this is your 33.1. So, here it is going to be 11.6 then 22.1 and at the center it is 15.6 then another two meter it is 7.1 and here 11.6. So, it will be 9.0. So, last layer is your 4.7 and 7.1 at the center it is 5.8.

Now, once you get the Δp increase in stress either from Boussinesq's method or by means of two is to one distribution then, write it column wise. So, every two, two meter interval we got our increase in stress. So, this is not one, one meter interval this is every two, two meter interval because stress increase also we got it every two, two meter interval. So you write it .1 .1 is at this point. This is your surface you are writing at the point, not surface. This clay layer starts from 0.8 to this is your **this is your** at here 0.8 here, it will be eight meter total clay layer is eight meter. So, this is your .1 and this is your bottom. Now, .1 height of the solid is 0 and p_0 has been written 38.8 and Δp is written 52.3 and from there this strain has been calculated though the strain formula is $c \cdot c' \log \frac{p_0 + \Delta p}{p_0}$.

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$C_c' = \frac{C_c}{H_{c0}} = 0.17$

$C_0 = W_n 95 \quad 2.76$

Point	H_z	P_0	ΔP	$\Sigma \epsilon_z$	ΔH_z
1	—	38.8	52.3	0.063	
2	2m	46.9	33.0	0.039	0.078
3	2m	63.1	15.6	0.016	0.032
4	2m	79.3	9.0	0.008	0.016
5	2m	95.5	5.8	0.004	0.008
Bottom	—	103.6	4.7	0.003	

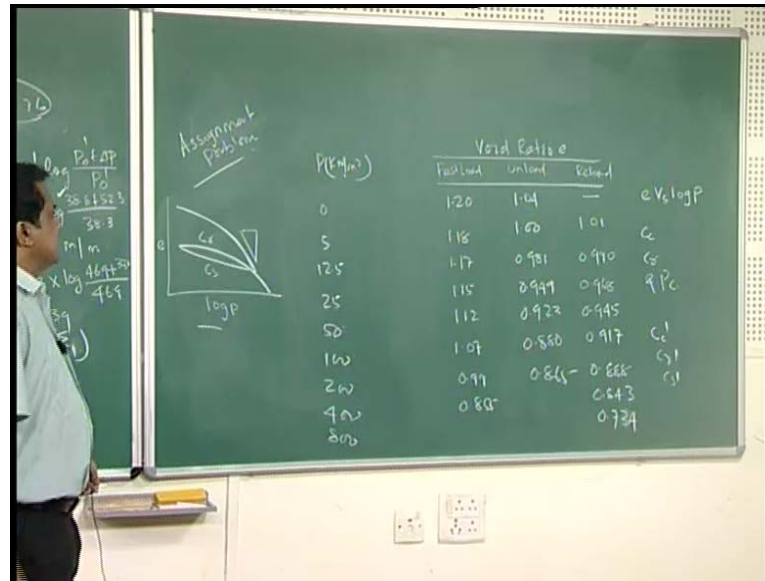
$\Sigma \Delta H_z = \Delta H = 0.134 \text{ m}$

$\Sigma \epsilon_1 = C_c' \log \frac{P_0 + \Delta P}{P_0}$
 $= 0.17 \times \log \frac{38.8 + 52.3}{38.8}$
 $= 0.063 \text{ m/m}$
 $\Sigma \epsilon_2 = 0.17 \times \log \frac{46.9 + 33.0}{46.9}$
 $= 0.039$
 $\Delta H_1 = H_1 (\Sigma \epsilon_1)$

So here you are getting 0.063. So calculated 0.063 next point is your .2 if I mark it here, this is my .1, this is my .2, this is .3. It will continue and this is your bottom. So, .2 particularly, .2 height is equal to two meter and p zero is equal to 46.9 whatever we calculated and increase in stress Δp if you look at here Δp it is your 33.0. Initially it is 52.3 at the surface. At the center it will be 30.0. Now, from there you calculate your ϵ_1 two. Then similarly, your .32 meter 63.1 15.6 all you calculate p zero and Δp you just calculate one by one. Then once you calculate p zero Δp then you calculate your ϵ_1 two, then find it out settlement. The settlement is nothing but, your h one into what is your strain. So, every cases h is equal to two two two two two meter. So, it is two into what is your change in strain, you can multiply you can calculate your strain, calculate your settlement then add it. So, total settlement consolidation settlement will be 0.134 meter.

So this is one **one** of this example, solved example problem. I just solve it so it may helpful particularly this assignments problems later on. One more problem I am giving this as an assignment may be student can practice. You can practice this one more problem as an assignment so, you try at your home. This is simple assignment problem. It is given p kilo Newton per meter square versus void ratio, different void ratio from the laboratory. This value has been given so first load unload and reload. So, plot first e versus $\log p$ diagram, e versus $\log p$. Once you get e versus $\log p$ from there you find it out what is your c_c . Then c_r , then you find it out p_c .

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So this kind of curve will come e versus $\log p$, this type of curve will come once you get it **once you get it** then find it out your c_c slope of this. Then find it out your c_s find it out your c_r , then once you get it, then find it out pre consolidation pressure from this curve you can find it out pre consolidation pressure and you do it in the graph paper and do it very neatly so that you can you will **you'll** get the appropriate results. Use your scale, proper scale it will be e versus $\log p$ diagram. So this is one assignment. So try and do it. Once you do it from there you can find it out what is your c_c prime **c_c prime**, c_r prime, c_s prime that you can do it. Suppose this is e versus $\log p$ is there from this diagram can you plot strain versus your $\log p$. That you can think strain versus $\log p$ if you can do that would be fine. **Thank you.**