

Geotechnical Measurements and Explorations

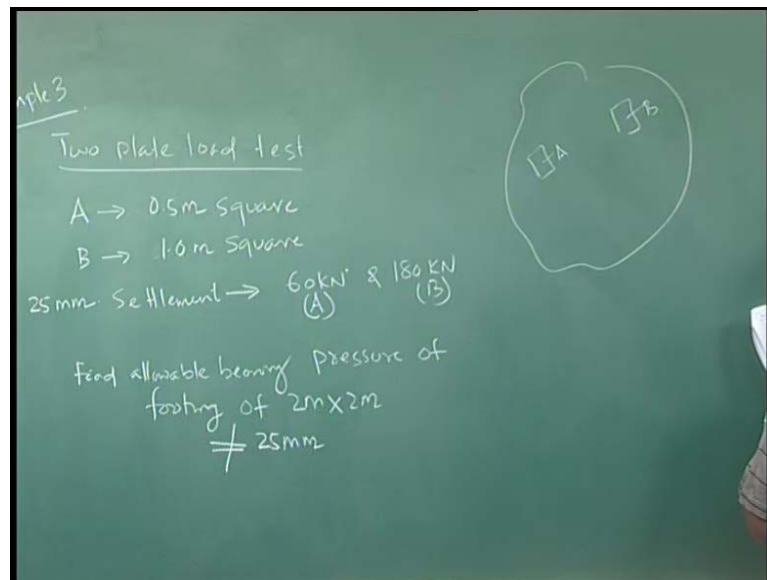
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Lecture No. # 16

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Example three: Two plate load test conducted on a site; one plate say A - plate A, a size 0.5 meter square, and plate B, a size 1.0 meter square. So, for a settlement of 25 mm, the load found to be 60 kilonewton for plate A, and 180 kilonewton for plate B. Determine allowable bearing pressure or maybe you can say find allowable bearing pressure of footing of 2 meter by 2 meter with a settlement not exceeding 25 mm. This is the third example, question is in a site, **in a site**, there are two plate load test has been conducted; one is A, other is your B. So the size of plate of A is 0.5 meter square size and B is 1.0 meter square size, and it is expected 25 mm of settlement, happened a no failure load occur with respect to 25 mm of settlement, plate A load failure load is 60 kilonewton, plate B is 180 kilonewton. Find allowable bearing pressure of footing, square footing of size 2 meter by 2 meter.

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The chalkboard contains the following handwritten content:

- General Formula:**

$$q_{ult} = m \times \sigma$$

$$x = \frac{P - P_{area}}{A_{area}}$$
- For footing:**

$$x = \frac{4 \times 2}{2^2} = 2 \text{ m}^{-1}$$

$$q = m \times \sigma = 100 \text{ kN/m}^2$$

$$Q_s = q_u \times A_{area} = 100 \times 4 = 400 \text{ kN}$$
- Plate Load Test A:**

$$x_1 = \frac{P_1}{A_1} = \frac{4 \times 0.5}{0.5^2} = 8 \text{ m}^{-1}$$

$$q_1 = \frac{60}{0.5 \times 0.5} = 240 \text{ kN/m}^2$$

$$240 = 8m + \sigma$$

$$m = 40 \text{ kN/m}, \sigma = 20 \text{ kN/m}^2$$
- Plate Load Test B:**

$$x_2 = 4 \text{ m}^{-1}$$

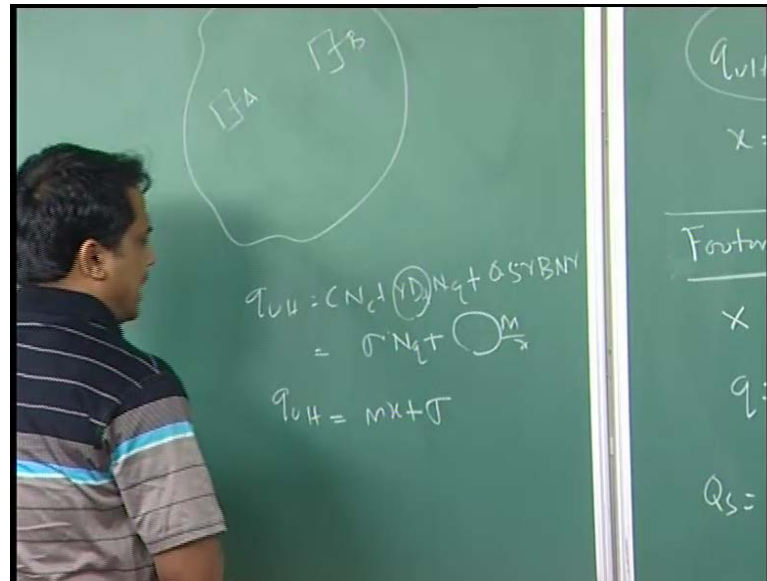
$$q_2 = \frac{180}{1 \times 1} = 180 \text{ kN/m}^2$$

$$180 = 4m + \sigma$$

Now so q ultimate, if I write it, $m \times \sigma$; so x is equal to P by A ; so, for first plate load test say A, and second plate load test say B. So, X_1 is equal to P_1 by A_1 , so P is perimeter, A is equal to area ratio or area. So in that case, 4 into 0.5 by 0.5 whole square x_1 is equal to 8 meter in inverse. Similarly, X_2 for plate 2, it will be 4 meter inverse; q_1 is equal to how much load it is given? Failure load is given 60 kilonewton; with this 60 by area, 0.5 into 0.5 , which is equal to 240 kilonewton per meter square. Similarly, q_2 for plate 2, which is equal to 180 by 1 by 1 , because the size is 1 meter square, so this will be 180 kilonewton per meter square. Now, if we look at here for first case, it will be for second case, it will be 180 , q ultimate is equal to $4 \text{ m} + \sigma$; case one it is $240 = 8 \text{ m} + \sigma$. So from this, m is equal to 40 kilonewton per meter and σ is equal to 20 kilonewton per meter square.

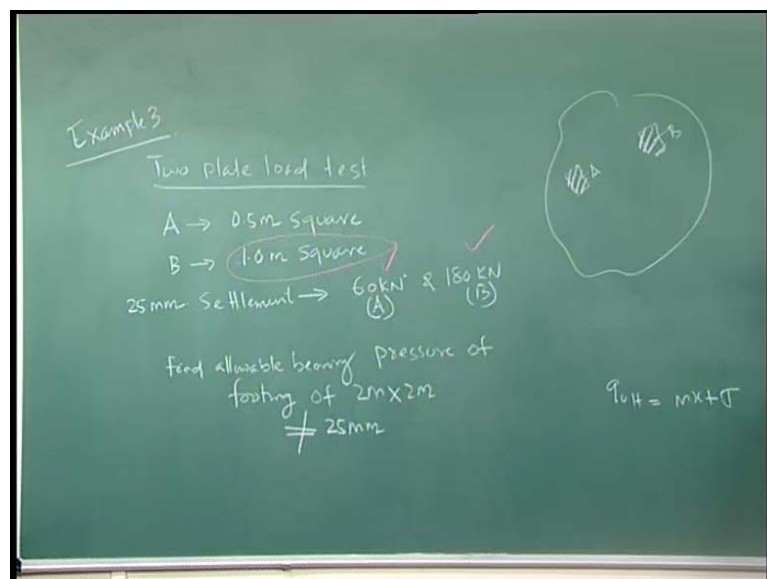
Now for real footing, these two parameters I got it from these two plate load test m and σ ; and from these for real prototype footing, x is equal to or you can write for real footing, x is equal to 4 into 2 **4 into 2**, this is a squares shape, so 4 into 2 perimeter divided by area, which is equal to 2 square, 2 meter inverse; and q is equal to $m \times \sigma$, where it will give 100 kilonewton per meters square, so Q_s is equal to q_u into area, which is equal to 100 into $4 = 400$ kilonewton. This is another way of getting bearing capacity of footing; in this case, about soil profile nothing has been given. So, this has been given by **(())**; so if there are two plates to load test at the side if you conduct, then you can easily get the **(())** parameter of m and σ .

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Because they say if you write it, $q_{ultimate}$ is equal to ultimate bearing capacity $C N_c + \gamma D_f N_q + 0.5 \gamma B N_{\gamma}$, if we look at here, it will be if I take it $\sigma' N_q$ plus m into x , x is perimeter by area, so here this term will come m by x , N_q and N_{γ} are constant; so that is why they have proposed, which is equal to in terms of $m x$ plus σ' .

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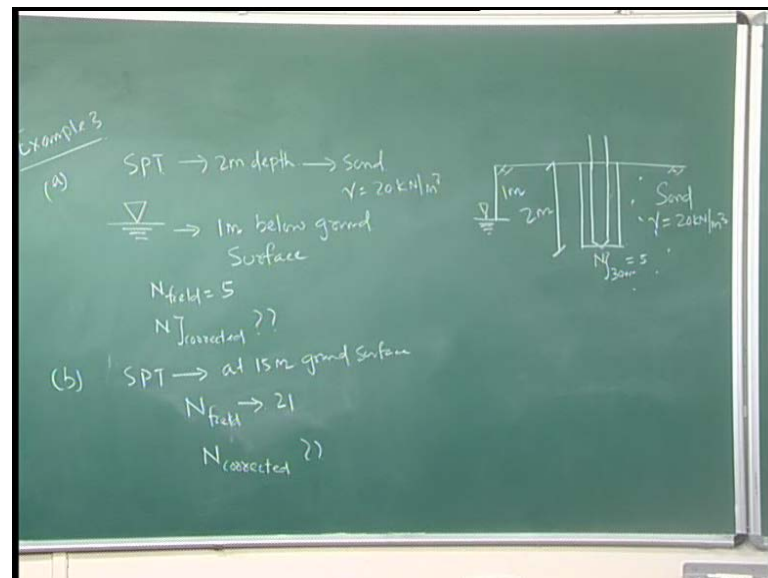


So that means there are two parameters; one is m and other is σ' ; for two parameters, two unknowns that means two equations are required. So to get that, at least you have to

do testing at least two plate load test required. So here in this example, two plate load test was conducted were conducted; for plate A, the size is 0.5 meter square and plate B, 1.5 meter square, and up to 25 mm settlement the failure load observed 60 kilonewton and 180 kilonewton. With this, you find it out what is the value of x ; x is nothing but perimeter to area ratio. For plate A, x you find, for plate B x you find. Similarly, ultimate load or q you can find it out failure load divided by area; so this is your failure load for plate A, failure load for plate B, so q_1 you will get it, q_2 you will get it; with these parameters you can find it out what is the value of m and σ .

As these two plate load test has been conducted in the same side, so for real footing, same formula can be applicable; and this **this** same formula has been applied. And for real footing, you find it out x and as well as means, q you can get it, because you have m value you know, and σ value you know; this is your m value and σ value, you get it from the plate load test. From there you can find it out q ; once you get q , this is your ultimate load you can find it out safe or how much is your total load coming, total load carrying capacity of this footing you can find it out.

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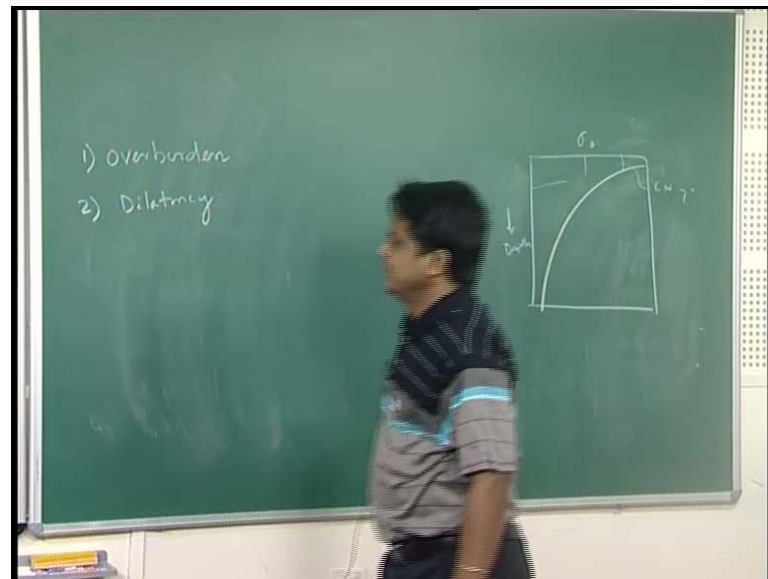


Now come to example 3: A SPT that means Standard Penetration Test, SPT conducted at a depth of 2 meter depth, soil is sand and gamma is equal to 20 kilonewton per meter cube. Then water table, water table for the site is located 1 meter below ground surface; the N value was observed to be $N_{\text{field}} = 5$. Then it has been asked, what is the N

corrected? (No audio from 11:42 to 11:52) Now, next part is this is part a; part b is at the same side, SPT conducted at 15 meter ground surface. N value, N field is observed to be 21; then asked what is the value of N corrected.

Now initially, what is the question? Question is this is ground surface, and standard penetration test has been carried out below 2 meter, this is 2 meter, so SPT test has been carried out below 2 meter; and this entire soil is sand deposit, gamma is equal to 20 kilonewton per meter cube. And there is a water table, we say that water table is there below 1 meter of ground surface; now n field n for 30 centimeter below, which is equal to 5. Now find the... Case 1, find the corrected value. Now, what are the two corrections, supposed to be or what are the corrections applied in standard penetration test.

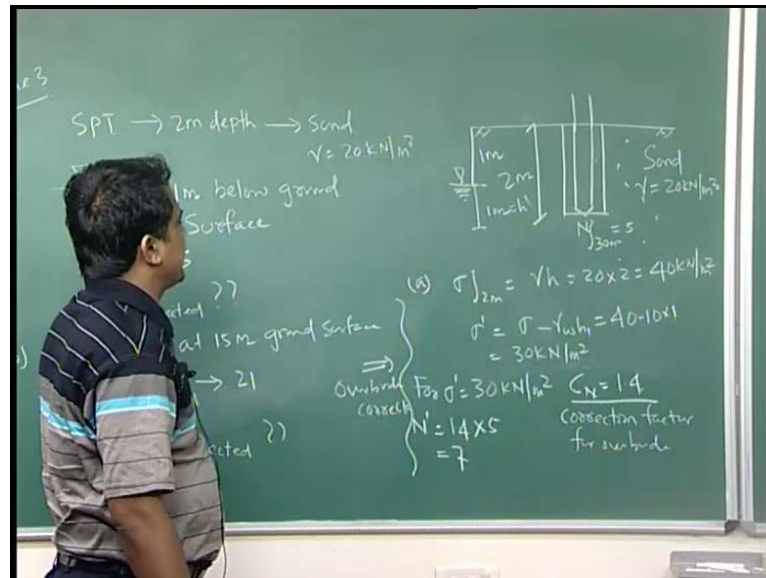
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There are two corrections; one is your overburden, second is your dilatancy; one is your overburden correction, second is your dilatancy. Now, come to part 1 or a, what is the overburden at 2 meter depth, sigma 2 meter is equal to gamma into h; gamma is equal to 20 into to h is equal to 2, which is equal to 40 kilonewton per meter square. Now sigma prime is equal to because water table is at 1 meter; so it will be, below 1 meter, it will be water table effect, so definitely we will go for gamma submerge unit weight. So it will be sigma minus gamma w h prime, so here h prime is your 1 meter, which is equal to 40 minus 10 into 1, 30 kilonewton per meter square. Now if we look at there, there is a

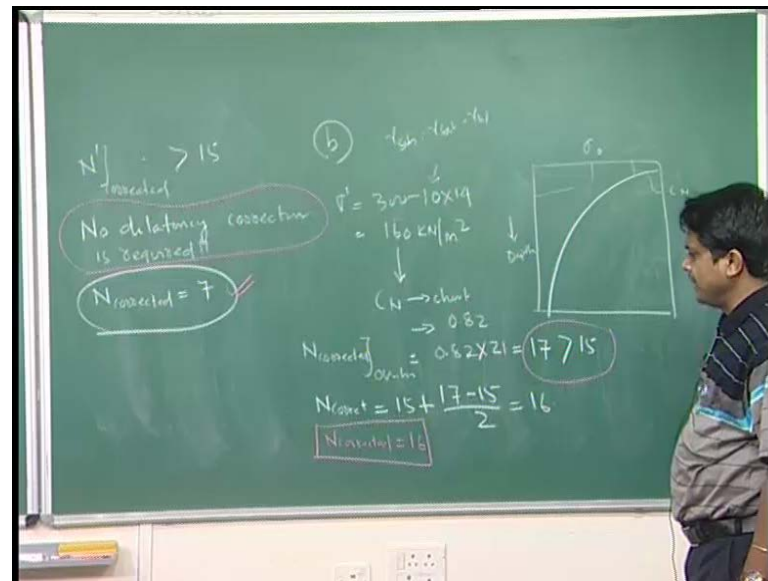
chart, with this overburden **with this over burden**, there is a chart, earlier I have shown, what is the value of sigma C N? We can get it from the chart.

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So with respect to sigma for sigma prime or effective stress over burden 30 kilonewton per meter square C N is equal to 1.4, correction factor C N is correction factor for over burden C N is for overburden. Now with this N corrected is equal to 1.4 into 5, which is equal to 7. Now next question is whatever part is this is your overburden correction. Now second part is your dilatancy correction. So, if there is an water table; is there any dilatancy correction? Yes. Then provide... At what condition dilatancy condition to be applied?

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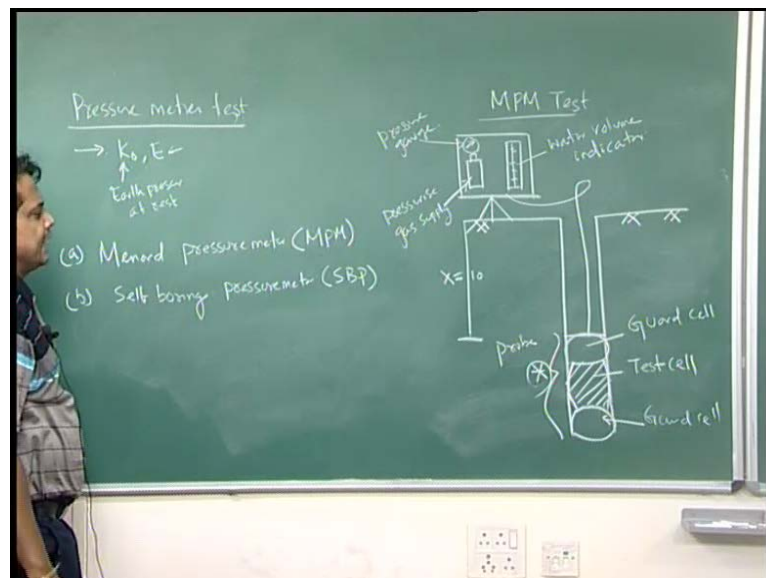
Dilatancy correction to be applied when N corrected after overburden, N corrected after overburden is greater than 15. So in this case, N corrected after overburden correction, it is less than 15; that means no dilatancy **dilatancy** correction is required. Now similarly in this case, no dilatancy correction is required, so what is your total correction? N corrected equal to **n corrected is equal to 7**. Now come to part b; part b it is proposed to carry standard sanitation test at 15 meter below the ground surface. This is ground surface that means in that case, overburden pressure is equal to at 15 meter 300 minus 10 into 14, because total is 15 meter; 1 meter you will leave it, below 1 meter there is water table, definitely gamma saturated you can do it gamma saturated value, because gamma is equal to 20, so gamma saturated is equal to gamma submerge is equal to gamma saturated minus gamma w.

With this 20 minus 10, it will be 10; so sigma prime means overburden pressure is equal to 160 kilonewton per meter square. With respect to this, your C_N from chart, it will be 0.82. Now N corrected for overburden is equal to 0.82 into 21, which is equal to 17; as it is greater than 15, as this overburden correction is greater than 15, again condition is that water table is lying below 1 meter, then this dilatancy correction is going to apply. Now for dilatancy correction N corrected, which is equal to 15 plus 17 minus 15 by 2, which is equal to 16. Now final N corrected is equal to 16. So in this example there are two cases one is your SPT has been carried out below 2 meter depth, other is SPT has been carried out below 15 meter depth from ground surfaces.

In case one, calculate your overburden, considering water table, then with respect to your overburden, find the correction **for correction** factor for overburden correction, and the N corrected is coming 7. As water table is lying at the ground surface, so dilatancy correction is required; again N corrected after overburden is less than 15, hence no dilatancy correction is required. So N corrected is 7. In case two, it is proposed to carry out your speed standard penetration test are 15 meter below ground surface; with respect to 15 meter, what is your overburden? Then with respect to overburden, correction factor for overburden has been calculated from this chart, overburden correction has been calculated, it is 17; as water table is lying in the ground, again N correction after overburden is greater 15. Hence dilatancy correction is required; and after the dilatancy correction, N correction is 16.

Now, next phase this is all about there are four examples based on, one is your plate load test two example last class, and one example is also today plate load test, third example is your **...** Fourth example, **four example**, this is your fourth example. So it is based on standard penetration test, how to do this correction, what are the corrections, how to calculate N correction - N corrected value?

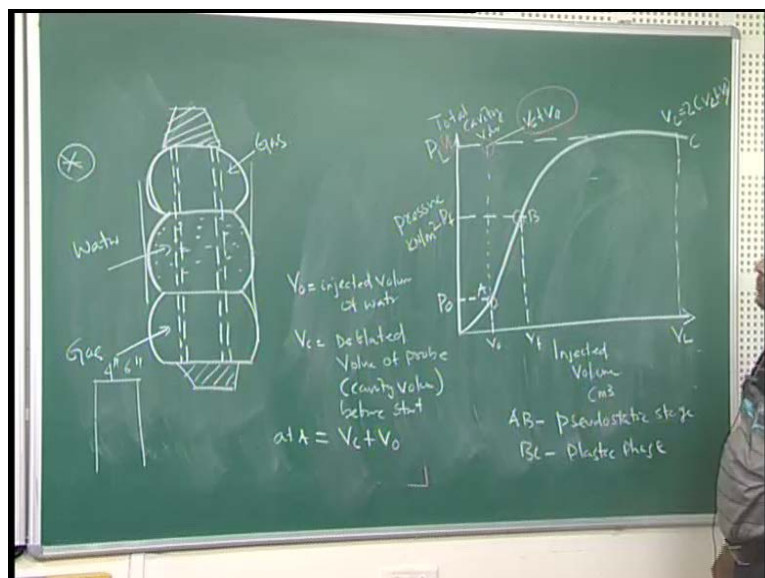
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Now next phase is your pressure meter test. Why we go for pressure meter test? To measure k_0 and E **in** from in situ condition; k_0 is your earth pressure at rest, and this is your modulus of elasticity. So there are two methods; one is your pressure meter test,

other is your hydraulic fracture; it has two **two** test, it has application for different soils. So in this case, the pressure meter test, it will be particularly applied to any kind of soils, and there are two pressure meters; one is your Menard Pressure Meter - MPM; then second is your Self Boring Pressure Meter. Now we will discuss the generally menard pressure meter is widely used; so for this menard pressure meter test, I am just drawing this schematic sketch, where you can understand (No audio from 25:01 to 25:36) pressurized gas supply, this is your pressure gauge, water volume indicator, now this is called simply a probe; this is probe and it is guard cell, test cell probe consist of guard cell, test cell both the ends probe is guard cell in between it is test cell.

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If I make this guard cell in bigger form...

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Now, this is one **one** guard cell one filled by gas, then this is your middle is your test cell, it is by means of water, then again bottom part is also guard cell, it is by means by your gas. So, pressure meter test a tentative sketch, I draw it detail means schematic sketch, generally in pressure meter test, what happen? Bore hole has been made; suppose it has been decided a depth of X, say 10 meter, 20 meter, 15 meter, 5 meter, you go and find it out what is the insitu k_0 and E? So in that case this is called completely **this is call completely** a probe; this probe has been connected, **this probe has been connected** to

outside, the system is there, where we can measure the pressure, where water volume indicator is there, how much water is in there inside and gas supply also there.

So once you enter the probe, what will happen? Guard cells at the bottom and the top, guard cell at the bottom and the top filled by means of gas. Then with the help of water, this is gas, this is gas and the test cell filled up by means of gas; by means of water once you are applying, if this is my soil, corresponding soil here, so what will happen? If this is the bore hole, so it will try to expand both the way, both the direction it will try to expand, so that means the soil will be expanded laterally; once you increase the level of water, so it will try to expand the volume; once it expand, side wall be your, it is your soil; so soil contact this cell, it will try expand laterally. So you know, what is the value of γh overburden here that is your σ_1 .

Now, k_0 you can find it out, k_0 is equal to σ_3 by σ_1 ; σ_3 is your lateral pressure, and σ_1 is your over burden or vertical pressure; so σ_1 you can calculate, how do get σ_3 ? So, it will expand, it will try to push the soil both the sides; once soil fails, once soil fails, the expansion will stop or it may goes somewhere else or remains constant; that means we further expansion no change in volume, no change in volume or no change in further addition of water, no change in volume; that case you consider as a σ_3 case. So once you get σ_3 , and with the help of γ and height, at what height you are doing, you can calculate σ_1 , then easily you can get k_0 or pressure at rest.

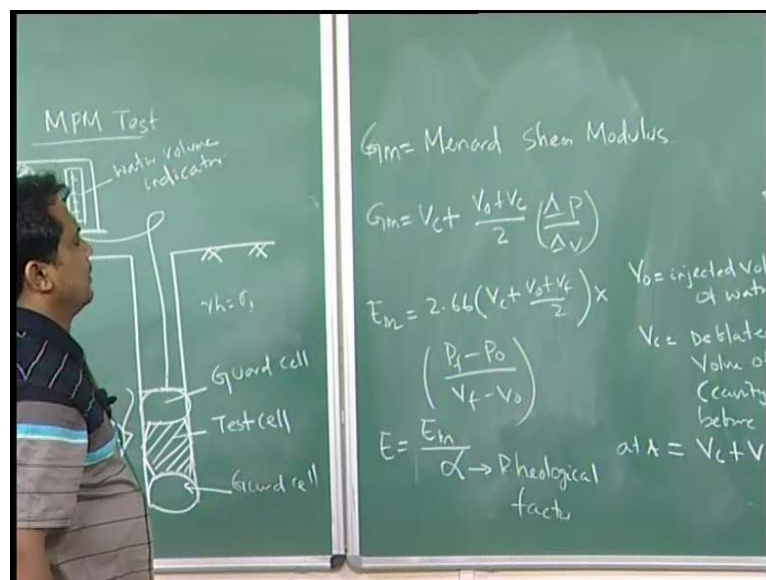
Now, what is the what is the graph it looks like? If I draw pressure verses volume change, this is pressure kilonewton per meter square; this is your injected volume in centimeter cube. How do you get the volume changes? Indicator you are getting, how much you are adding water inside, accordingly volume will be changing. Then with expansion change in pressure inside, you can measure it by the means of pressure gauge. So the curve will go, this is A, this is your B and this is C; P_0 P_f , V_f , this is your P_L and this part will comes somewhere else here, it will be V_L .

So in this case, V_0 is equal to injected volume of water at the beginning; V_c is equal to if I write here in this side total cavity volume, so then it will be V_c , then this will be your V_c plus V_0 , then this will be V_L is equal to $2 V_c$ plus V_0 ; V_c is deflated volume of probe or it is called cavity volume before start. So the volume of cavity at A at

A which is equal to V_c plus V_0 ; A B generally called pseudo static stage; B C is your plastic phase. If we look at here, what is V_c ? V_c is nothing but your plated volume of probe, suppose there is a bore hole has been made, bore hole has been made, suppose diameter is say 400 or may be 4 inch 4 inch bore hole or may be 6 inch bore hole. So what will happen? Initially the probe you have to deflate it, so that is touch, it should touch both the end of this borehole. So, that volume, initial volume before start it is called V_c .

So the V_c is initially, so if I write this is my V_c , then what happen? Once you applied injected volume, so at point A it will be V_0 , so V_0 is your injected volume of water, with the help of water it expand; but initially the volume is V_c , so that is why at this point, the volume will be your V_c plus V_0 . So it will go elastic, then after some time, it reaches plastic deflated volume. So when when volume is equal to V_c plus V_0 twice the volume, at that time general it has been considered the value of σ_3 achieved; or may be to go elastic then plastic, and there is no more further change in volume, that point you can consider as a from the pressure you can take it as σ_3 . So once you get σ_3 , once you get σ_1 , then you can find it out later earth pressure.

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Now, what is the co relation it is here? G_m , suppose G_m is equal to Menard shear modulus. So G_m generally calculated, it is V_c plus V_0 plus V_c by 2 delta P by delta V. E_m is equal to 2.66 V_c plus V_0 plus V_f by 2 into P_f minus P_0 by V_f minus V_0 . So

you can calculate E is equal to E_m by α ; α is equal to rheological factor, α is equal to rheological factor; for different soils, for different soils, there is a chart, α you can get it, from there you can find it out E .