

Unsaturated Soil Mechanics
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Week - 09
Lecture - 27
Extended M-C Criterion – II

Hello everyone, we have been discussing the interpretation of suction controlled direct shear test data using Extended M-C Criterion. So, let us discuss how to interpret the suction controlled triaxial test data using extended MC criterion in this lecture.

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SHEAR STRENGTH

o Extended MC criterion:

Test-I : $u_a - u_w = 0 \text{ kPa}$, $\sigma'_2 - u_w = \sigma'_3 - u_w = 10 \text{ kPa}$, $\sigma'_1 - u_w = 60 \text{ kPa}$ } $\rightarrow c', \phi'$

Test-II : $u_a - u_w = 0 \text{ kPa}$, $\sigma'_2 - u_w = \sigma'_3 - u_w = 30 \text{ kPa}$, $\sigma'_1 - u_w = 100 \text{ kPa}$ } $\rightarrow c', \phi'$

Test-III : $u_a - u_w = 60 \text{ kPa}$, $\sigma'_2 - u_w = \sigma'_3 - u_w = 10 \text{ kPa}$, $\sigma'_1 - u_w = 100 \text{ kPa}$ } $\rightarrow c', \phi'$

Test-IV : $u_a - u_w = 60 \text{ kPa}$, $\sigma'_2 - u_w = \sigma'_3 - u_w = 30 \text{ kPa}$, $\sigma'_1 - u_w = 142 \text{ kPa}$ } $\rightarrow c', \phi'$

Test-I :
$$\begin{bmatrix} \sigma'_1 - u_w & 0 & 0 \\ 0 & \sigma'_2 - u_w & 0 \\ 0 & 0 & \sigma'_3 - u_w \end{bmatrix} = \begin{bmatrix} 60 & 0 & 0 \\ 0 & 10 & 0 \\ 0 & 0 & 10 \end{bmatrix}$$

Extended MC criterion:

$$(\sigma'_1 - u_w) = (\sigma'_3 - u_w) \tan^2(45 + \phi'/2) + 2c' \tan(45 + \phi'/2)$$

$$c' = c' + (u_a - u_w) \tan \phi'$$

$$60 = 10 \tan^2(45 + \phi'/2) + 2c' \tan(45 + \phi'/2)$$

$$100 = 30 \tan^2(45 + \phi'/2) + 2c' \tan(45 + \phi'/2)$$

$$\phi' = 2 \left[\tan^{-1} \left(\frac{40}{20} \right) - 45 \right] = 19.47^\circ; c' = 14.14 \text{ kPa}$$

So, in triaxial test setup, which is a suction controlled, here we independently controls u and w . So, therefore, you can controlled the suction and you can also control the all round pressure and the deviatoric stress. So, then you apply the deviatoric stress, so that you get the shear failure and you get the deviatoric stress at failure and pore water pressure etcetera. Then you can strength of the soil at different suction values using extended MC criterion so, let us solve one simple problem.

So, here we have some synthetic data that is a data representative of extended triaxial test suction controlled triaxial test. So, you have series of tests conducted say test I, test 1 consists of u minus w , so where the suction is maintained to be 0 kilo pascal. So, essentially the test is conducted at fully saturated state. And the all round pressure, so

there is a $\sigma_2 - u_a$, which is nothing but which is equals to $\sigma_3 - u_a$, because this is radial applied in triaxial setup, so which is equals to 10 kilo pascal. And then the failure of the soil took place, when the major principal stress value of 60 kilo pascal.

Similarly, another test is conducted test II. Here the test is again conducted at saturated state $u_a - u_w$ is equals to 0 kilo pascal and the similarly the σ_2 and σ_3 or $\sigma_2 - u_a$ and $\sigma_3 - u_a$ both are increased. So, then the failure took place, when $\sigma_1 - u_a$ of 100 kilo pascal. So, this is one set of data conducted at saturation state, so that we can get up you can obtain C and phi values.

Another set of test data conducted say test III, where $u_a - u_w$ is maintained to be 60 kilopascal and $\sigma_2 - u_a$ and $\sigma_3 - u_a$ both are at 10 kilo pascal. So, this is similar to the first test, except that the suction value is a non-zero or it has some suction value, the test is conducted at unsaturated state. So, obviously we expect that when the suction is increased, even though all round pressure is same, you expect to fail at higher load, so this would fail at 100 kilo pascal.

Similarly, another test is conducted test 4, where the suction is maintained to be constant 60 kilo pascal, but the all round pressure is changed to same value as test 2, so that is 30 kilo pascal. So, as the all round pressure is increased by keeping the suction value constant, then definitely the $\sigma_1 - u_a$ increases, so this value becomes 142 kilo pascal. So, it failed at higher major principle stress. So, higher deviatoric stress it fail so, this is another set of data.

So, from the first set of data, one can obtain the C value and phi value. And from this data, again you get C 1 dash here C dash and you get phi b. Now, here again you get phi dash so, using these two test data, you can get phi b. So, this is how we get, let us understand how we can solve this problem. This also can be represented in a matrix form, the test results one can be represented as $\sigma_1 - u_a$, when does $\sigma_2 - u_a$, $\sigma_3 - u_a$, this is 0, this is 0 is equals to 60, 10, 10. So, it can be represented in this manner also, this is the test 1, either way we can represent the data in either way.

So, now the extended MC criterion given by Fredlund et al that is a $\sigma_1 - u_a$ is equals to $\sigma_3 - u_a \times \tan^2 45^\circ + \phi + 2 C \tan 45^\circ$

phi dash by 2. Here, this is C 1 dash; this becomes C dash, when matric suction is 0, so, this is the expression we use for saturated soils also. So, when this is when you have suction in the soils, when this is defined for unsaturated soils, this is C 1 dash, where C 1 dash is equals to C dash plus u a minus u w times tan phi b fine.

Now, first test data we can substitute the values of sigma 1 minus u a that becomes 60 kilo pascal and sigma 3 minus u a is 10 kilo pascal, then tan square 45 plus phi dash by 2 plus 2 c. Here, anyways the u a minus u w is 0, so I substitute the value of 0, then it C 1 dash is nothing but C dash. So, this is 2 C dash tan 45 plus phi dash by 2.

From the 2nd test data, again we can write this one as. Solving these two equations, we get if you subtract the 1st equation from the 2nd equation, then we get phi value. Phi dash value is nothing but 2 times tan inverse square root of 40 by 20 minus 45, then this is equals to 14 19.47. So, phi dash is 19.47, if we substitute this value in this, we get C dash is equals to 14.14 kilo pascal, of course this can be substituted into the other equation also. Either way you get the C dash the cohesion intercept, which is 14.14.

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SHEAR STRENGTH

- Extended MC criterion:

$$100 = 10 \tan^2(45 + \phi/2) + 2 C_1' \tan(45 + \phi/2)$$

$$142 = 30 \tan^2(45 + \phi/2) + 2 C_1' \tan(45 + \phi/2)$$

$$42 = 20 \tan^2(45 + \phi/2) \Rightarrow \phi' = 20.7^\circ \text{ \& } C_1' = 27.8 \text{ kPa}$$

$$\phi' = \frac{19.47 + 20.7}{2} = 19.8^\circ; \quad C_1' = 14.14 \text{ kPa}$$

$$C_1' = c' + (u_a - u_w) \tan \phi'$$

$$27.8 = 14.14 + 60 \tan \phi' \Rightarrow \phi' = 12.8 \text{ kPa}$$

$$\tau_f = (\sigma - u_a) \tan(19.8) + (u_a - u_w) \tan(12.8) + 14.14$$

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Similarly, we can use the other test data test 3 data when we use, so we get 100 is equals to 10 tan square 45 plus phi by 2 plus 2 C 1 dash, here you have an intercept, because the suction value is non-zero tan 45 plus phi dash by 2. Similarly, the 4th data here this is 142, which is equals to 30 tan square 45 plus phi dash by 2 plus 2 C 1 dash, these are

same C_1 , because at the same suction value, the tests are conducted plus ϕ by 2.

When this is solved, you get this expression so, when you solve for ϕ , ϕ is 20.7 degrees. And when you substitute either of these equations, then you get C_1 , which is equal to 27.8 kilo pascal. So, if you observe, if you notice the ϕ value from the first two test data, you got 19.47 so, from the last two test data, you got 20.7.

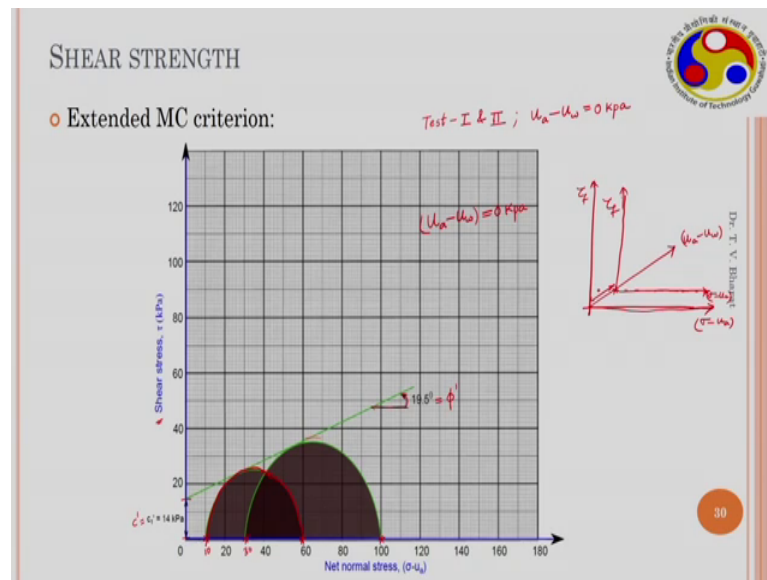
So, when we take average value, so this is 19.47 plus 20.7 divided by 2, this gives a value of around 19.8 degrees and C anyways we have 14.14. So, we got two of the strength parameters is estimated. We can estimate ϕ_b by using those two expressions, where the C_1 is equal to $C + u_a - u_w \tan \phi_b$.

If we utilize this expression, C_1 is known that is a 27.8, C is also known that is 14.14 plus $u_a - u_w$, when the intercept is C_1 is 60 kilo pascal that is into $\tan \phi_b$. So, when we solve this, we get ϕ_b , which is nothing but 12.8 kilo pascal. So, ϕ_b is less than ϕ in this case. So, these three are the strength parameters of the extended MC criterion given by Fredlund et al so, this is how we interpret the triaxial test data.

And then you can write the expression that is τ_f is equal to $\sigma - u_a \tan \phi$ is 19.8 plus $u_a - u_w \tan \phi_b$ is 12.8 degrees plus C , C is 14.14. So, this is the modified MC criterion for this particular soil. So, therefore, at any given stress state $\sigma - u_a$ and $u_a - u_w$, the strength of the soil shear strength of the soil can be estimated using this particular expression. So, the stress state of the soil is defined, then we understand the behaviour.

If the stress state changes with time due to some monsoon season, during monsoon season generally the water infiltrates into the ground or infiltrates into the soil slopes, then the suction values decrease. When these suction values decrease, how the strength changes can be evaluated using this particular expression. So, the same thing can be solved graphically similar to what we did for direct shear test data, let us examine, how we do it.

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So, here in the extended MC criterion, we have all the test data given so, this is a using test I and II data sets. This is at one particular u_a minus u_w that is equals to u_a minus u_w is 0 kilo pascal. So, the three-dimensional diagram, there is a three-dimensional surface failure surface can be now approximated τ_f and this is σ minus u_a and other axis is u_a minus u_w . So, this is a three-dimensional thing, we use to plot earlier.

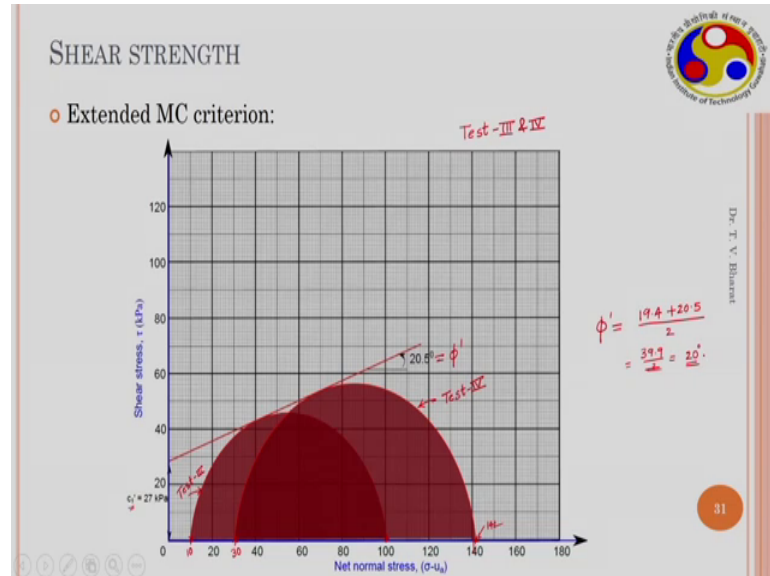
Now, because we are plotting at one particular u_a minus u_w , so you can now consider τ_f here and σ minus u_a here and this is at one particular u_a minus u_w that is u_a minus u_w in this particular case is 0 kilo pascal. So, as is 0 kilo pascal, you can directly plot it here itself, this is 0 now. So, directly you can plot it here, so that is what we have done, the τ_f or the shear stress on y-axis and net normal stress σ minus u_a on x-axis.

So, when this is plotted, now we have two test datasets. So, two test data sets, one is 1 σ_1 minus u_a σ_2 minus u_a is 60 kilo pascal, this is 60 kilo pascal and σ_2 minus u_a and σ_3 minus u_a both are 10 kilo pascal, so this is 10. So, this is one more circle and similarly, the other test data that is a σ_1 minus u_a is 100 kilo pascal and σ_2 minus u_a are σ_3 minus u_a is 30 kilo pascal, so this is second more circle.

So, you draw a line joining to joining these 2 circles so, this is the line you get and the angle of this line is ϕ dash angle of internal friction. The intercept here is c dash itself,

because $u_a - u_w$ is 0 here. So, the C_1 dash is equals to C dash that is a 14.14 or 14 kilo pascals so, this graphically we can directly get this data.

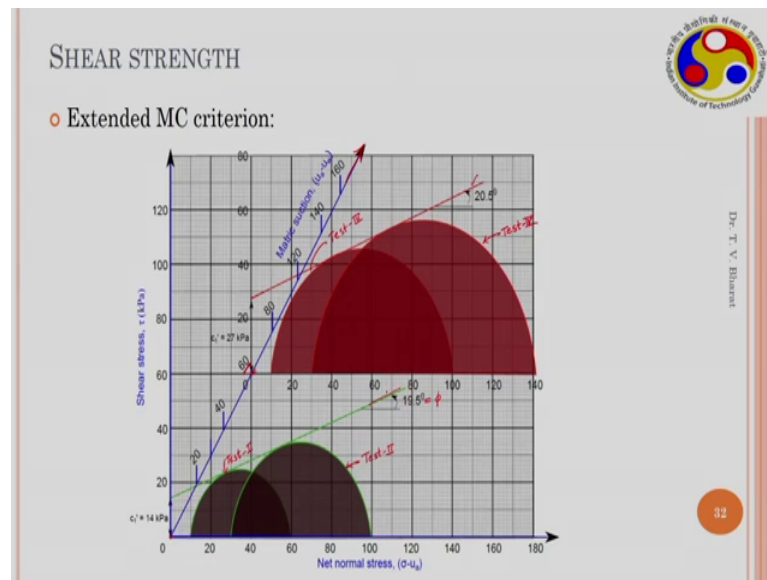
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Second, using 3rd and the 4th test data that is test data III and IV we can obtain shear stress versus net normal stress by recognizing the values of $\sigma_1 - u_a$ and $\sigma_3 - u_a$. $\sigma_3 - u_a$ value is sorry, for this particular data that is test data III, this is a test data 3 test III. Test III data the $\sigma_3 - u_a$ data is 10 kilo pascal and this value is 100 kilopascal, $\sigma_1 - u_a$ is 100 kilo pascal. And similarly, the test IV, the $\sigma_3 - u_a$ is 30 and $\sigma_1 - u_a$ is 140 142 kilopascal so, this is 140 and this is 142.

So, now again we join a line to these two circles so, this is a failure plane. So, this failure plane once it is drawn, the angle is this is angle of internal friction that is 20.5 degrees and the intercept is C_1 dash, this is 27 kilo pascal. So, now you got C_1 dash and ϕ' dash. Now, you can take average value of ϕ' dash, earlier you got a 19.47 or 4, now 20.5. So, we take the average and we get the average value nearly 20 nearly 20 degrees we get.

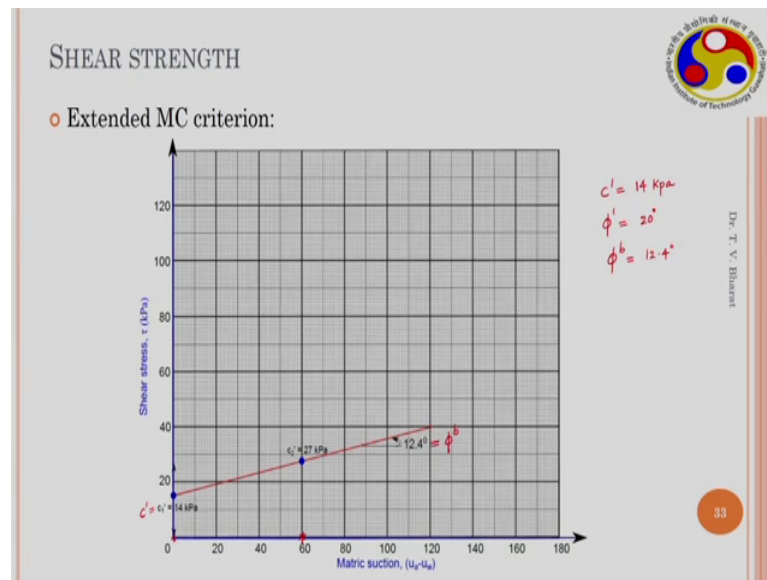
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So, this can be represented in this particular manner. The first test data is this test I and this is test II and this is test III and this is test IV. So, this test I and test II both are conducted at $u_a - u_w$ of 0. So, this is $u_a - u_w$ axis, the z-axis so, when $u_a - u_w$ is 0 on this axis, this particular test data is conducted.

So, τ_f versus σ_a at $u_a - u_w$ is equals to 0 so, this is the data so, this is the failure plane that is angle of internal friction is 19.5. Similarly, at $u_a - u_w$ value of 60 kilo pascal, this tests are conducted. So, this is represented in this manner and this line is the failure plane line that is 20.5 degrees and which is nearly parallel to this line, because the values are very close right.

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Now, as we have the values of C^1 equals to C from the first two test data, which are that are conducted at 0 kilo pascal u_a minus u_w . Though, so the matric suction is 0 correspondingly, the values of value of this one is 14 kilo pascal. So, corresponding to matric suction 0, the C^1 value is C^1 equals to C that is 14 kilo pascal. And corresponding to matric suction 60, the C^2 value is 27 kilo pascal, so another intercept.

So, now if you join these two lines, you get ϕ^b that is 12.4 degrees. The intercept, in fact is anyways it is starting at matric suction 0, so you got C . So, therefore, C is estimated, which is 14 kilo pascal. And ϕ^1 is estimated, which is 20 degrees and ϕ^b is estimated, which is 12.4 degrees. This data, which is obtained, graphically is very close to the analytical technique. This data is very close to the data obtained from the analytical technique.

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SHEAR STRENGTH


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Extended MC criterion:

Example 2

Test #	$(u_a - u_w)$	$(\sigma_3 - u_a)$	$\sigma_1 - u_a$
1	40	30	100
2	40	80	200
3	120	30	180
4	120	80	280

$\sigma_1 - u_a = (\sigma_3 - u_a) \tan^2(45 + \phi'/2) + 2c' \tan(45 + \phi'/2)$
 $\sigma_1 - u_a = (\sigma_3 - u_a) \tan^2(45 + \phi^b/2) + 2c' \tan(45 + \phi^b/2)$

$\phi^b = 19.47^\circ$; $c' = 42.43 \text{ kPa}$
 $c' = c' + (u_a - u_w) \tan \phi^b$
 $100 = c' + 40 \tan \phi^b$
 $42.43 = c' + 120 \tan \phi^b$

$\phi^b = 19.4^\circ$; $\phi^b = 19.5^\circ$; $c' = 0 \text{ kPa}$

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Let us solve another problem, so, here the data is summarized in this manner. So, test data and $u_a - u_w$, so which is controlled and $\sigma_3 - u_a$, which is maintained and $\sigma_1 - u_a$, which is observed. So, now test 1, the $u_a - u_w$ is maintained to be 40 and the $\sigma_3 - u_a$ is maintained to be 30 and this $\sigma_1 - u_a$ is observed to be 100. For test II, this is again 40 $u_a - u_w$ is 40 and $\sigma_3 - u_a$ is 80, so at higher all round pressure the test is conducted by maintaining the same matric suction, therefore the $\sigma_1 - u_a$ should now increase, this value is 200.

Then now, the matric suction is increased to 120 kilo pascal and the $\sigma_3 - u_a$ is now 30, then $\sigma_1 - u_a$ should be higher than 100, which is observed to be 180. And the IV test with the same matric suction, but increasing the all round pressure 280, the same value as the second test now they should be higher than 200, because the matric suction is higher than the second case, so this is 280 so, these are the observations. Here there is one interesting aspect that is a reason why I am actually discussing this particular example so; here no test is conducted at fully saturated state.

In case if of Bishop's effective stress, Bishop's extended MC criterion, Bishop's modified Mohr Coulomb criterion to estimate c ϕ values and ψ f , one needs to conduct the tests at fully saturated state, at least two tests should be conducted. If you have cohesion intercept also, then minimum two tests need to be conducted, so that you

can estimate cohesion and an angle of internal friction. And when these two values are estimated, other tests can be conducted at higher matric suction values, so that you can obtain ψ value at corresponding matric suction value.

However, in this case, even though you have cohesion intercept, you do not need to conduct the tests at fully saturated state without knowing the test data at fully saturated state, you can obtain the entire Mohr Coulomb failure, you can obtain the failure surface. Let us see how we have obtained? So, briefly I discussed this using these two test data sets using these two test data sets, you can obtain C_1 and ϕ . And using these two test data sets, you can obtain C_2 and ϕ . So, using these two test data, you should be able to get C and ϕ , let us see how we get.

So, from the first two sets of data, we get when we substitute in the MC criterion, there is $\sigma_1 - u_a = (\sigma_3 - u_a) \tan^2 45^\circ + \phi + 2C_1 \tan 45^\circ$. This is at one particular $u_a - u_w$, so, here when we substitute $\sigma_1 - u_a$ from the 2nd test data, this is 200 equals to $80 \tan^2 45^\circ + \phi + 2C_1 \tan 45^\circ$. From the 1st test, this is $100 - 40 \tan^2 45^\circ + \phi + 2C_2$, because this is conducted at one particular suction, which is not same as the suction value, which is used in the first two test data 1st test sorry, here this is C_1 , because this is conducted at the same suction.

So, now when you simplify this simply 100 and this is $40 \tan^2 45^\circ + \phi + 2C_1$. Thus, if you solve for ϕ , you get ϕ equals to 19.47 degrees, which is same value as we obtained in the previous case. So, you can obtain the C_1 . C_1 equals to 14.15 kilo pascal. Just in the previous cases, you can directly substitute ϕ value in one of these expressions, you can get C_1 , which is 14.15 kilo pascal.

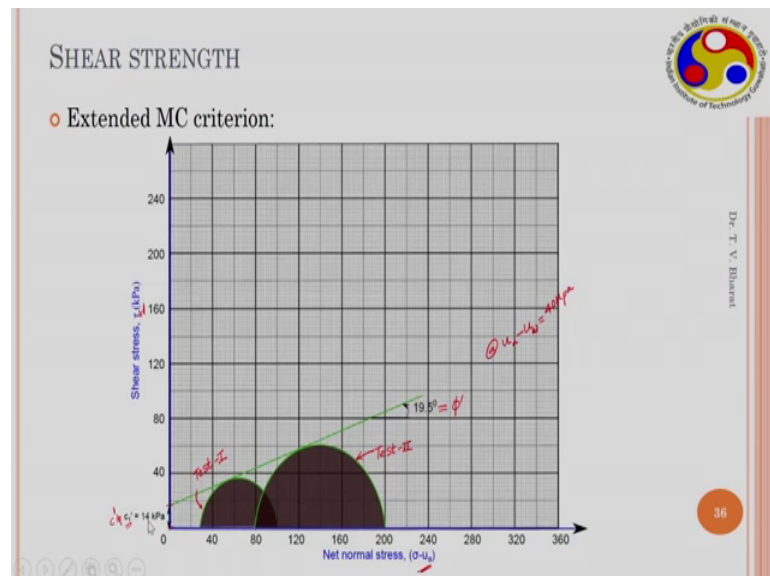
Similarly, this is 1 and this 2, when you use sorry this is 2 and 1 so, this is 2, and this 1. And similarly, when you use 4, 280 equals to $80 \tan^2 45^\circ + \phi + 2C_2$, because these two are conducted at $u_a - u_w$ of 40 kilo pascal. And this one is 180 - 30 sorry this is 30, so this is 50, so that is why this is half, and you get the same value ϕ . So, here this is $30 \tan^2 45^\circ + \phi + 2C$. These two are $u_a - u_w$ of 120 kilo pascal; this is 4th and this

3rd data. So, when we solve this, we get the same phi dash, which is 19.47 degrees, and C 2 dash is 42.43 kilo pascal.

So, from the expression, that C 1 dash is equals to C dash plus u a minus u w times tan phi b. Using this expression, you can substitute C 1 dash that is 14.15 is equals to C dash plus u a minus u w is 40 tan phi b. And in the second expression, C 2 dash is 42.43 is equal to C dash plus 120 tan phi b. When you solve this, you get phi b is equal to 19.5 kilo pascal, which is same as your phi, phi is nearly same and the C is 0 kilo pascal.

So, when the intercept this is intercept is 0 that means, this is sand, the test data is for sand. So, you got phi dash, which is same as 19.45 or 5 kilo sorry degrees, sorry this is degrees. So, you have phi dash, phi b dash and C dash, all these thee strength parameters could be estimated just by utilizing four test data sets. So, these test data sets are essentially we obtained by maintaining one particular suction value, two test data sets could be obtained by changing the all round pressure and another set of data can be obtained by maintaining different matric suction, but changing the all round pressure. So, if we have such data, we can obtain all the strength parameters estimated. Quickly let us see how graphically this could be done.

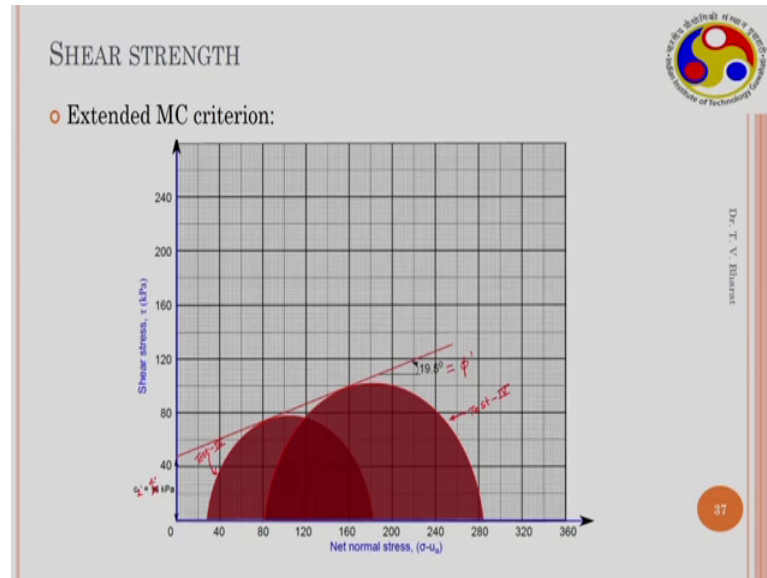
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So, here as we have done earlier, the net normal stress that is sigma minus u a and tau f are plotted, this is at one particular u a minus u w that is u a minus u w is 40 kilo pascal. So, at 40 kilo pascal, when we use the test data 1, this is for test data 1 and this is test

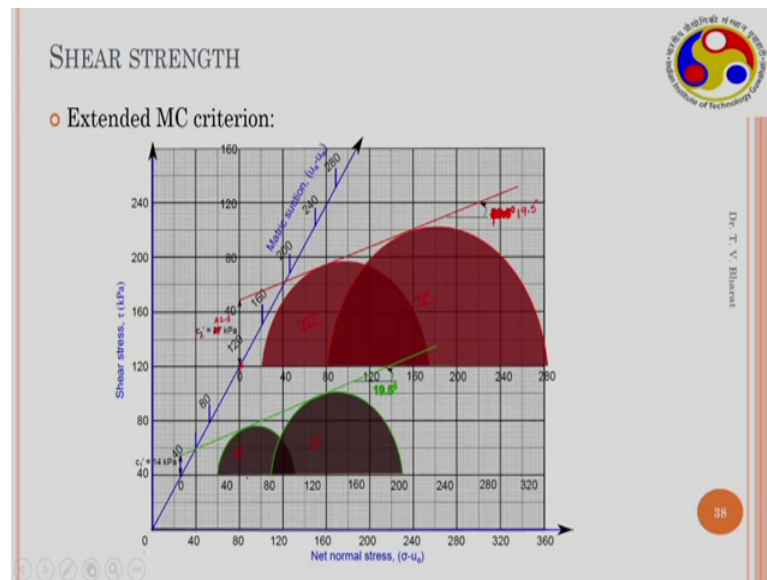
data 2, then we obtained the angle of internal friction that is 19.5 degrees. And the intercept, C 1 dash this is not equal to C dash, because this test is conducted at one particular matric suction value, so this is 14 kilo pascal.

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Then the other two test data were used, where this is test data III and this is test IV. So, the angle of internal friction is same that is 19.5 degrees. This is C 2 dash which is not 14; here we could see that this is 42, so these are around 42.5 so, this is 42.5 kilo pascal. So, the angle of internal friction is the angle of internal friction value is same for both the cases, but the intercept value changes from the first set of data and the second set of data because of matric suction value change changed.

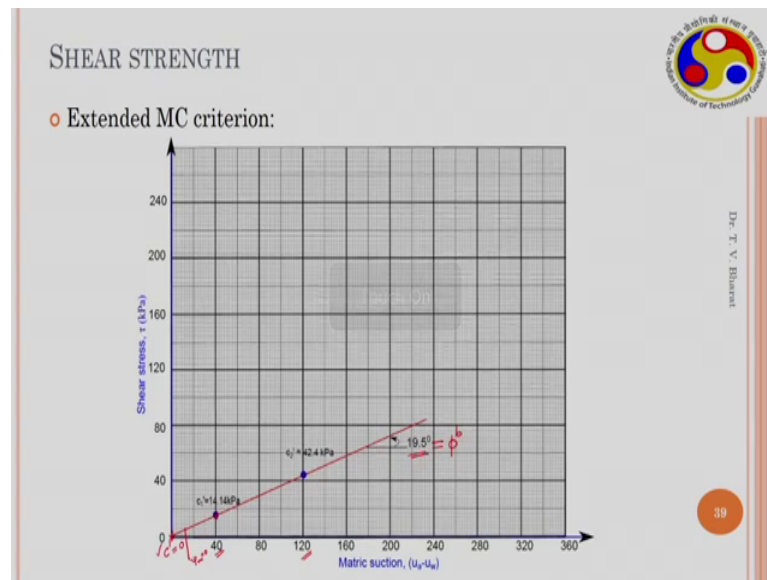
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Now, here this is 14, and this is not 27, this is C 1 dash, and this is C 2 dash, this is 42.5 kilo pascal and here 19.5 and this is also 19.5. Sometimes, the measurement errors would be there while plotting. And here this is a test data I, II, III and IV so, this is if you observe, which is on an axis of this is the matric suction axis.

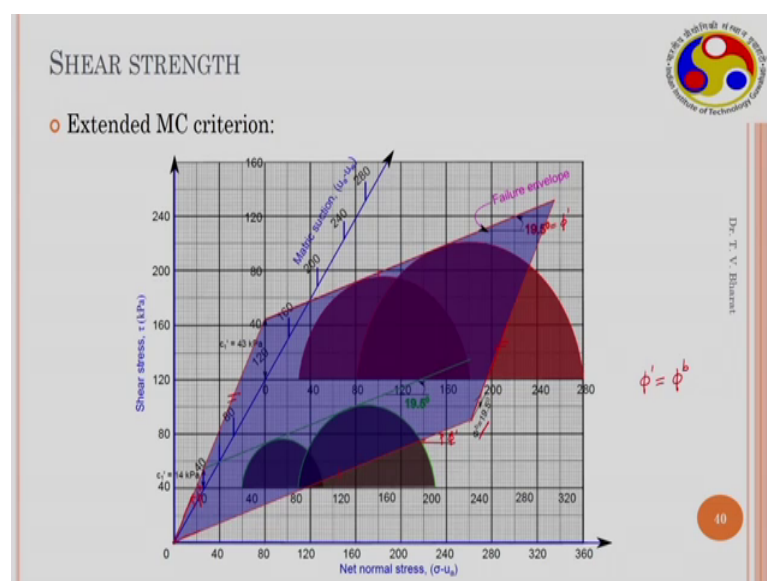
So, this is at a value of 40 kilo pascal so, this is 40 and this is 80, 120, 160. So, here again at 120, so that is another matric suction value, where the tests were conducted at 120, so this is drawn so, this is how now, we locate the test data of all the four test data on the three-axis plot.

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Now, once we have the phi dash, now phi dash in the cohesion intercepts, the first intercept value 14.14 kilo pascal or 14 kilo pascal, which is obtained at matric suction value of 40. And another intercept is obtained that is 42.5 at 120 kilo pascal. So, when these two test data are plotted on tau f versus matric suction plot so, when you draw the straight line joining these two points, the line passes through origin. So, the cohesion intercept C dash is equals to 0, which is at matric suction of 0 so, this we obtain for graphically. Now, the angle of internal the angle, which indicates phi b is 19.5 degrees so, this data is how we get phi b estimated.

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Now, we have all the data ϕ dash, ϕ b, and C dash. So, now this data this line is a this line is with ϕ b sorry this line is with ϕ dash, this line with horizontal makes ϕ dash 19.5. And this line joining these two planes is ϕ b that is also 19.5, for this particular case, in this particular case, the ϕ dash is equals to ϕ b. So, here this line joins from origin and at a 40 kilo pascal of matric suction, the intercept is 14.

And at 120 kilo pascal matric suction, the intercept is 42.5. And this line with respect to horizontal line, with respect to the matric suction line parallel to this. This angle is again ϕ b, in this case, ϕ b is equals to ϕ dash. Therefore, this is a 19.5 and this line is parallel to this line these two are parallel. And again, this and these two are parallel and this is ϕ dash and again this is ϕ dash. So, this represents a failure envelope, which is a surface so, this particular surface can be drawn due to the model given by Fredlund et al.

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SHEAR STRENGTH

Summary of **Modified M-C** and **Extended M-C** criteria:

- (Bishop)**
 - Parameters: $[c', \phi', \psi_f(u_a - u_w)]$
 - Test conditions:
 - $\psi_f = 1 \leftarrow \text{Sat.}, (u_a - u_w) = 0 \rightarrow \text{Test-I, Test-II} \rightarrow c', \phi'$
 - $(u_a - u_w) \rightarrow \text{Test-III} \rightarrow \psi_f$
- (Fredlund et al)**
 - Parameters: $[c', \phi', \phi^b]$
 - Test conditions:
 - $(u_a - u_w) \rightarrow \text{Test-I \& Test-II} \rightarrow c', \phi'$ [vary $(\sigma_3 - u_w)$]
 - $(u_a - u_w) \rightarrow \text{Test-III \& Test-IV} \rightarrow \phi^b$ [vary $\sigma_3 - u_w$]

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So, in summary, if we compare the modified M-C Criterion, which is given by Bishop and extended Mohr Coulomb criterion, which is given by Fredlund et al, there is a slight difference. Here the strength parameters are C dash, ϕ dash and ψ f, which is a functional form, which is function of u minus u w. In the Fredlund et al, the strength parameters are C dash, ϕ dash and ϕ b. So, these are the strength parameters in Bishop and these are the strength parameters in Fredlund et al.

The tests the triaxial tests are direct shear test, we need to conduct these tests sorry to estimate these strength parameters, we require suction control direct shear test data or suction control triaxial test data in a specified manner. For example, in case of Bishop's model, we acquire the test data at $u_a - u_w$ is equal to 0 that is at fully saturated state. At fully saturated state, we require 2 sets of data minimum because, from these 2 test of test data, we can obtain c' and ϕ' . If you have sand, then anyways the cohesion intercept does not exist at saturated state, so one test data would be sufficient.

And then you need to conduct series of tests. Test III, IV like that series of test data to obtain the values of ψ_f . ψ_f in this case is 1, but other cases ψ_f need to be estimated ψ_f can be estimated using the equations expressions for different suction values. When you maintain different suction values in different tests, you can obtain ψ_f value. ψ_f value changes from one at saturated state to nearly 0 at nearly dry state.

In Fredlund et al case, we do not require series of test data, we just require four test data, two test data should be conducted at one particular $u_a - u_w$, two tests need to be conducted test 1 and test 2. So, in these two tests, you can vary all round pressure $\sigma_3 - u_a$, if it is a triaxial test, or you can vary the net normal stress, if it is a direct shear test. And conduct another two tests, test III and IV again by varying $\sigma_3 - u_a$ or net normal stress in direct shear and conduct these two tests.

So, from this test, you obtain the cohesion intercept to one particular cohesion intercept at $u_a - u_w$ and the angle of internal friction. And from these two tests, you get another cohesion intercept and angle of internal friction. So, by solving these two expressions for cohesion intercepts, we get c' , ϕ' and ϕ_b by solving these two expressions for cohesion intercept, we get c' and ϕ_b . Anyways we already got the ϕ' , so we get all the strength parameters.

We do not require to conductive in the test set fully saturated state in case of Fredlund et al model that is extended MC criteria. So, this way, we can obtain the strength parameters using modified MC criterion and extended MC criterion by conducting different set of tests under suction control in suction control triaxial or suction control direct shear test.

Thank you.