

Unsaturated Soil Mechanics
Dr. T.V. Bharat
Department of Civil Engineering.
Indian Institute of Technology, Guwahati

Week – 07
Lecture – 21
Analytical Methods for Transient Flow-I

Hello everyone in the previous lecture we discussed Transient flows through horizontal and vertical columns or the transient flows without influence of elevation head and with the influence of elevation head we have considered and we got analytical solutions.

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TRANSIENT FLOWS

o Green – Ampt (1911) Equations:

Horizontal flow:
$$\frac{x}{\sqrt{t}} = \sqrt{2k_s \frac{(h_0 - h_i)}{(\theta_s - \theta_i)}} \quad \checkmark$$

Vertical flow:
$$\frac{k_s t}{(\theta_s - \theta_i)} = z - (h_0 - h_i) \ln \left(\frac{z + h_0 - h_i}{h_0 - h_i} \right) \quad \checkmark$$

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These equations are called Green and Ampt equations. This is given by the two gentlemen Green and Ampt in 1911 very old literature.

This particular equations this is for the horizontal flow the x by square root of time is equals to square root of 2 k s times h naught minus h i by theta naught minus theta s. Here theta naught or theta s. So, here x is the spatial distance and t is a time and k s is the saturated hydraulic conductivity of the soil h naught is positive head you maintain it could be 0 or more than 0. And h i is initial head and theta s is saturated volumetric water content and theta i is initial volumetric water content.

Similarly for the vertical flows we have this expression instead of x you will have the independent variables z here this expression is implicit and you can obtain for a given time what is waiting front location in the vertical direction can be obtained using this particular equation. Here the z is assume to be positive upward. So, when there is a infiltration against the gravity that is taking place then we have used positive value for z and this is a expression for this. Again once again I remind you that this expressions are divided in 1911.

So, when the computation was nearly a very costly and during that time the analytical expressions for the transient flows were derived. This expressions can be used for the coarse grained soils, but not applicable for fine grained soils because shorb wetting front generally you do not find for most of the soils, but that approximation is can be born for coarse grained soils; for some coarse grained soils.

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TRANSIENT FLOWS

○ Rate of Capillary rise:

- *Bukingham (1907) experimentally studied the movement of moisture in soil*

U.S. DEPARTMENT OF AGRICULTURE
BUREAU OF SOILS—BULLETIN NO. 38
MILTON WINTERKILL, CHIEF

STUDIES ON THE
MOVEMENT OF SOIL MOISTURE

BY
EDGAR BUCKINGHAM

Second Printing 1910

WASHINGTON
GOVERNMENT PRINTING OFFICE

Bukingham, E. (1907). "Studies on the movement of soil moisture." U.S. department of agriculture, Bureau of soils, 38.

Reservoir

Data logger

Sand column

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n, K_s, h_c

(i) Lane & Washburn (1946)

$$h_c = -970 \ln D_{10} - 1540$$

(mm)

(ii) peck et al. (1974)

$$h_c = \frac{C}{e D_{10}}$$

(mm)

C: 10 - 50 mm²

@ IITG

Interestingly capillary rise in soils has been studied slight little earlier by Buckingham in his classical work on studies on the movement of soil moisture which is published in 1907 very old work which is a classical work in soil science literature. In this particular work here done establish the rate of capillary rise he found the capillary rise data with time for several soils.

The experiment involves somewhat like this; this figure is not from this work this is setup from our lab at IITG. And you will have a soil column it could be sand column in

this case and you have a reservoir, water reservoir these two are connected through this pipes.

So, you have a water table maintain at particular level. So, this is the water table that is maintained which can be read on the scale. So, when the water table level in the reservoir is this at equilibrium the water table level in the soil also is expected to be here somewhere here.

So, this is the water table and z is measured upward positive from here onwards and here you can see the capillary rise that is taking place and capillary fringe you can be seen the fringe the way the water moves in sand which is called a capillary fringe you can be seen here.

So, the rate at which the movement of water takes place within the soil mass was obtained for different soils where are the porosity values for different and all compaction densities could be different therefore, the porosity varies or the saturated hydraulic conductivity varies then the at equilibrium what is a maximum a capillary height that was achieved, that was found which is indicated with h_c capillary height.

So, for these three different values for example, the capillary height values for different soils were try to correlated this is the capillary height is correlated with different soil pore size distribution parameters by several researchers afterwards also for example, several works in later on were conducted by Lane and Washburn. This is in 1946 he established relationship between h_c and d_{10} of soil h_c measured in millimetre and here $h_c = 990 d_{10}^{-1.5}$. This is one empirical equation he derived by conducting experiments on several soils and here d_{10} is also substituted in mm.

So, h_c was predicted using such empirical equations. So, this is one such empirical equation, another empirical equation given by Peck et al this is in 1974. So, here the h_c is equals to $c \sqrt{e} / D_{10}$. Here D_{10} is substitute in mm and c is units of mm square and h_c has units of mm, e is a void ratio and c generally it varies between 10 to 50 mm square.

So, such empirical correlations are available for predicting the capillary rise or capillary height for different coarse grained soils generally, because for fine grained soils it takes enormous time to reach equilibrium and moreover the height will be very significantly high.

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TRANSIENT FLOWS

o Rate of Capillary rise:

- Terzaghi (1943):

$$q_r = -k_s i$$

$$n \frac{dz}{dt} = +k_s \left(\frac{h_c - z}{z} \right) \Rightarrow \text{Green-Ampt (1911)}$$

$$- \frac{z - h_c}{(h_c - z)} dz = + \frac{k_s}{n} \int dt$$

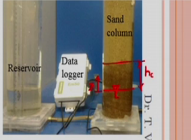
$$\int dz + h_c \int \frac{dz}{(h_c - z)} = + \frac{k_s t}{n} + C$$

$$-z - h_c \ln(h_c - z) = + \frac{k_s t}{n} + C$$

$$C = -h_c \ln(h_c)$$

$$-z + h_c \ln(h_c) - h_c \ln(h_c - z) = + \frac{k_s t}{n}$$

$$-z + h_c \ln \left(\frac{h_c}{h_c - z} \right) = + \frac{k_s t}{n}$$

$$t = \frac{nt}{k_s} \left[h_c \ln \left(\frac{h_c}{h_c - z} \right) - \frac{z}{k} \right]$$


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Terzaghi, K. (1943). "Theoretical Soil Mechanics", Chapman and Hall

Little afterwards in 1943 Terzaghi's in famous book theoretical soil mechanics provides one solution for rate of capillary rise. The rate at which the capillary rise occurs in soils a simple analytical solution is derived which has some interesting aspect which I will discuss after the derivation.

So, which is a again assumes that the Darcy velocity value it is. So, he uses q equals to minus $k_s i$ take also minus $k_s i$ and q we substitute the flux or the Darcy velocity as earlier we substitute as n times dz by dt . So, this is velocity and this is porosity of the soil. So, this is equals to minus k_s he assumes that the saturated hydraulic conductivity still valid for this particular soil because he also assumes that there is a sharp wetting front exist within the soil when the capillary rise takes place which assumption is similar to green and ampt equations.

So here he assumes the gradient that governs for the flow is h_c the capillary height minus z which is a distance for example, this is a water table within the soil mass and z varies upward and which is, now if this is the capillary height. So, this is the capillary height h_c minus z which is the head that controls and distance is z .

So, this is assumed to be the gradient for the flow. So, when z equals to 0 there is this gradient becomes infinity that means, it wants to achieve this value and when z is equals to h_c that means, it reaches the capillary height then gradient become 0.

So, this is a hydraulic gradient he used, this expression is very one similar to the Green and Ampt equations which were given in 1911 and this is 1943.

So, we can understand that rate of capillary rise equations are derived based on existing green and Ampt model for the transient flows through soils during that time. So, he further he simplifies this expression. So, if we simplified. So, this you get this expression $z dz$ by h_c minus z is equal to minus k_s by n integral dt .

So, after rearranging the terms all z terms are taken into one side and time terms are taken into other side and we integrate on both sides then. So, this can be simplified further into if you put minus here and add h_c to it I will put $d z$ somewhere here and minus h_c then this expression becomes minus first two terms if you used this is minus $d z$ plus h_c times $d z$ by h_c minus z is equal to minus k_s by n this is time t plus constant of integration.

So, this is what results and this is minus z plus the integration of dx by a minus x is minus $\log a$ minus x . So, therefore, so, this becomes minus $h_c \log h_c$ minus z there is equals to $k_s t$ by n plus c . If we invoke the initial condition that is at time t equal to 0 z is equal to 0 there is no flow there is no rise. So, then if we invoke that the c is equals to minus $h_c \log h_c$ minus z sorry h_c .

So, this is the value for c if we substitute we get minus z . Then if you bring this value this side plus h_c minus h_c sorry plus $h_c \log h_c$ minus $h_c \log h_c$ minus z minus $k_s t$ by n . So, this can be written as minus z minus of h_c . If you take common or plus h_c if you take common. So, this simply becomes $\log h_c$ by h_c minus z is equal to minus $k_s t$ by n .

So, therefore, the t is equals to n by k_s . If I take common h_c also then \log see if I put k_s plus k_s here then everything should be fine I think then this is simply. So, this is plus k_s then everything is fine then. So, t is equals to minus (Refer Time: 14:33) $k_s n t$ by $k_s k_s$ times $h_c \log h_c$ by h_c minus z minus z by h_c .

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TRANSIENT FLOWS

o Rate of Capillary rise:

- Terzaghi (1943): *Gen. Darcy's*

$$q_r = -K_s i$$

$$n \frac{dz}{dt} = +K_s \left(\frac{h_c - z}{z} \right) \Rightarrow \text{Green-Ampt equation (1911)}$$

$$-\int \frac{h_c - z}{h_c - z} dz = \frac{K_s}{n} \int dt$$

$$- \int dz + h_c \int \frac{dz}{h_c - z} = \frac{K_s t}{n} + C$$

$$-z - h_c \ln(h_c - z) = \frac{K_s t}{n} + C$$

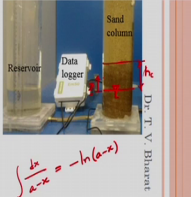
Initial cond. at $t=0, z=h_c$

$$C = -h_c \ln(h_c)$$

$$-z + h_c \ln(h_c) - h_c \ln(h_c - z) = \frac{K_s t}{n}$$

$$-z + h_c \ln \left(\frac{h_c}{h_c - z} \right) = \frac{K_s t}{n}$$

$$t = \frac{nh_c}{K_s} \left[\ln \left(\frac{h_c}{h_c - z} \right) - \frac{z}{h_c} \right] \checkmark$$



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Terzaghi, K. (1943). "Theoretical Soil Mechanics", Chapman and Hall

So, it assumes the validity of Darcy's law general Darcy's law that is we consider total head here. So, if we consider that this can be written as minus k_s the hydraulic gradient he considers is h_c that is capillary height minus z divided by z . This form is a very much similar to the green and Ampt equations which are derived or given in 1911.

If you notice the time interval where the Terzaghis expression is given in 1943 for the capillary rise rate and by then already the Green and Ampt equations are available for transient flows through unsaturated soil. So, therefore, it is very much clear that Terzaghi has considered the grant green and Ampt equations for the derivation of capillary rise rate in unsaturated soils.

So, here this expression q is equals to minus k_s time this one if is it written and here the q is similar to the green and Ampt their θ_s minus θ_i it is written. So, here initially the soil is assumed to be completely dry then θ is equals to 0. Then that is θ_s which is nothing, but n times dz by dt there is a velocity term.

So, then this is flux is equals to minus k_s again he assumes that the saturated hydraulic conductivity provides because he assumes that there is a wetting front; sharp wetting front exist during the capillary rise which is similar to what shown here. If this is a water table location and above this the z the elevation is shown with z and if this is the capillary height h_c then the gradient is considered to be h_c minus z that is a head that governs the flow divided by the z at any given location for example, at z equal to 0. So, it has it becomes infinity that means.

So, the gradient is with full capacity it has infinite gradient or large gradients. So, when z is equals to h_c that means, when the flow reaches the estimated capillary height. So, because the capillary height is should be known in this particular equation if it reaches the capillary height then the gradient is 0 and the flow stops.

So, that is expression for when we simplify this here we need to observe one thing here as we are writing the flow is taking place from higher head to the lower head because this is the higher head and it is taking towards the lower head this should be positive value and then you will get positive quantities and it is solvable.

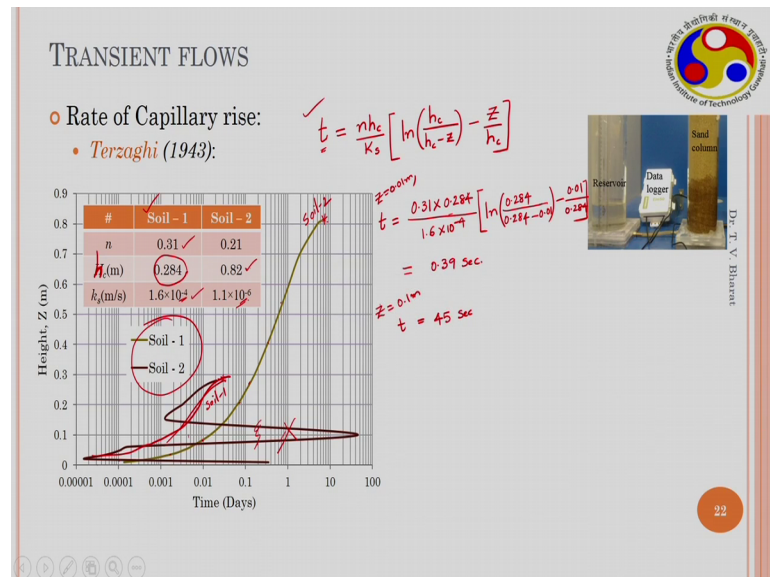
Now, when you simplify this particular expression by rearranging the terms on both sides you get $z \frac{dz}{dt} = \frac{k_s}{n} (h_c - z)$ and integrate on both sides. So, you get and if I take it on the other side $\int \frac{dz}{h_c - z} = \int \frac{k_s}{n} dt$ and integrate the dt term. So, then if we integrate this we can simplify this a priori. So, you can taking negative here and a put negative here and then you can $\int \frac{dz}{h_c - z} = -\ln|h_c - z| + C$ I will write here then I can add h_c and deduct h_c .

So, then I can simplify this as $-\ln|h_c - z| = \frac{k_s}{n} t + C$. If I write the first two terms then this is 1. So, therefore, $-\ln|h_c - z| + \ln|h_c| = \frac{k_s}{n} t + C$ which is equals to $\ln\left(\frac{h_c}{h_c - z}\right) = \frac{k_s}{n} t + C$ plus an integration constant. So, this expression when you solve here this is $h_c - z = h_c e^{-\frac{k_s}{n} t - C}$ plus h_c .

So, the integration for this is $\int \frac{1}{a - x} dx = -\ln|a - x| + C$. So, therefore, the integration for this is you get negative here $-\ln|h_c - z| + \ln|h_c| = \frac{k_s}{n} t + C$ is equal to $\ln\left(\frac{h_c}{h_c - z}\right) = \frac{k_s}{n} t + C$ plus C . If we invoke the initial conditions, the initial condition is at time t equal to 0 z is equals to 0 there is no flow, the capillary rise is 0.

So, therefore, C becomes $-\ln\left(\frac{h_c}{h_c}\right) = 0$. So, if I substitute this constant here you get $-\ln|h_c - z| + \ln|h_c| = \frac{k_s}{n} t$ and take the C on other side $-\ln|h_c - z| = \frac{k_s}{n} t - \ln|h_c|$ plus $h_c \log h_c$ minus $h_c \log h_c$ minus z is equal to $\ln\left(\frac{h_c}{h_c - z}\right) = \frac{k_s}{n} t$ and if I simplified this $-\ln|h_c - z| + \ln|h_c| = \frac{k_s}{n} t$ is equal to $\ln\left(\frac{h_c}{h_c - z}\right) = \frac{k_s}{n} t$ by n . So, this can be further written as t is equals to $n \ln\left(\frac{h_c}{h_c - z}\right) / k_s$. If I take the h_c common this is $\ln\left(\frac{h_c}{h_c - z}\right) = \ln\left(\frac{h_c}{h_c - z}\right) = \ln\left(\frac{h_c}{h_c - z}\right)$ minus z by h_c . So, this is the expression given by Terzaghi in 1943 in theoretical solve mechanics book and this expression is an implicit expression describing the rate of rise.

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So, there are some problem with this figure here. So, ignore this soil to figure it should be somewhat like this.

So, there is some issue and then you can ignore this soil to data you can only assume the soil one data now let us see. So, the expression for time t is equals to $n h c$ by $k s$ times $\log h c$ by $h c$ minus z minus z by $h c$. Here for a given soil see you take soil one the porosity of the soil is known $k s$ of the soil is known $H c$ should be determined or $H c$ should be known the capillary rise at equilibrium should be known for a given soil then if such information is known one can predict the rate at which the capillary rise takes place through partially saturated soils can be determined.

So, for example, if this data is given this particular data is given we substitute for time equals to at any given says space. Here it is an implicit form therefore, what is a time taken for reaching the capillary rise to given a elevation can be formed. So, for example, the time required to reach 0.1 metre 0.01 metre can be obtained.

So, which is n equals to 0.31 times $H c$ is 0.284 this is substituted in metres $k s$ again substituted in metres metre per second this is 1.6 into 10 to the power minus 4 the data is taken from here times $\log H c$ is 0.284 this should have been smaller letter small $h c$ 0.284 divided by 0.284 minus z I said 0.01 metre minus 0.01 metre divided by $H c$ 0.284.

So, this comes out to be a very small value 0.39 seconds. So, to reach 0.01 metres it hardly takes 0.39 seconds for this particular soil having these properties. Similarly when

z is equals to, here z is equals to 0.01 metre if z is equals to 0.01 metre, but time required is 45 seconds.

So, in this manner we are using different data points at different heights or elevation the time we have found and drawn in this manner. So, this is a data somehow there is error in plotting. So, dimensions. So, this is somehow distorted, but the data should have been like this.


So, as the hydraulic conductivity decreased nearly 2.4. So, this is for soil one right. So, this is for soil two this is for soil one. So, the colour also there is some error in this while copying from a spreadsheet there is a error in the error or the entire figure got distorted. As the capillary height is 0.82 for soil 2. So, this is a data for this and 0.284. So, the data somewhat like this it should come here 0.284.

So, this is soil 1 and this is soil 2 for the soil 2 data the capillary height is 0.82 and the rate is very small because it is taking nearly 8 days to reach equilibrium because the hydraulic conductivity is 10^{-6} metre per second. Here the equilibrium time is very less hardly 0.03 days are nothing, but few seconds because the hydraulic conductivity is 10^{-4} metre per second.

So, with this one can used Terzaghis expression, its very simplest expression to find out the capitalize rate. However, researchers have found that the Terzaghis expression over estimates capitalize rate. This is because especially the limitations in the expression, one of the major limitations is that he assumes that the saturated hydraulic conductivity for the soils he uses saturated hydraulic conductivity for k.

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TRANSIENT FLOWS



o Rate of Capillary rise:

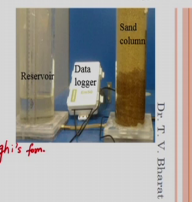
- *Lu and Likos (2004)*:

$$\frac{k_s t}{n} = \sum_{j=0}^m \frac{\alpha^j}{j!} \left[- \sum_{s=0}^j \frac{h_c^s z^{j+1-s}}{j+1-s} + h_c^{j+1} \ln \frac{h_c}{h_c - z} \right]$$

→ $m=0$
→ Terzaghi's form.

$$n \frac{dz}{dt} = \frac{k}{z} \left(\frac{h_c - z}{z} \right)$$

↓
 $K_s \exp(\alpha h)$



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Lu, N., and Likos, W. J. (2004). Rate of capillary rise in soils, Journal of Geotechnical and Geoenvironmental Engineering, 130(6).

So, which is the major limitation and in 2004 after in 1943 and Lu and Likos in 2004. They have given an expression by modifying the hydraulic conductivity function. So, they use the same expression here k times h_c minus z by z .

So, everything else is same instead of k is equal to k_s they use Gardner's expression they use expression for k as k_s time exponential of αh . So, this is what the expression they use and they got a close form analytical solution which is k_s times t by n which is similar to what we got is equals to sigma j equals to 1 to m α^j divide by j factorial times minus sigma s is equal to 0 to j , h_c power s into z power j plus 1 minus s divide by j plus 1 minus s plus h_c power j plus 1 log h_c by h_c minus z .

This expression will be same as Terzaghi's expression if the series form is not considered if j is equal to 0 to m equal to 0 if you substitute m equal to 0 this expression will converge to Terzaghi's form. So, the Lu and Likos in his paper rate of capillary rise in soils published in the *ac* journal. They showed that this particular expression is very good match for coarse grained soils for predicting rate of capillary rise. When compared to Terzaghi's expression Terzaghi's expression over estimates significantly.

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TRANSIENT FLOWS



o Richards (1931):

$$Q_z = q_z \Delta x \Delta y; Q_{z+\Delta z} = (q_z + \frac{\partial q_z}{\partial z} \Delta z) \Delta x \Delta y$$

$$\Delta Q = -\frac{\partial q_z}{\partial z} \Delta x \Delta y \Delta z = \frac{\partial v_w}{\partial t} = \frac{\partial \theta}{\partial t} \Delta V$$

$$-\frac{\partial q_z}{\partial z} = \frac{\partial \theta}{\partial t}$$

$$-\frac{\partial}{\partial z} \left(-K \frac{\partial h}{\partial z} \right) = \frac{\partial \theta}{\partial t} \rightarrow \text{mixed form}$$

$$\frac{\partial}{\partial z} \left(K \frac{\partial h}{\partial z} \right) = \frac{\partial \theta}{\partial t} \rightarrow \text{head form}$$

$$\frac{\partial}{\partial z} \left(K \frac{\partial h}{\partial z} \right) = \frac{\partial h}{\partial t} \frac{\partial \theta}{\partial h} \rightarrow \text{theta form}$$

$$\theta(z,t): \frac{\partial}{\partial z} \left(D(\theta) \frac{\partial \theta}{\partial z} \right) = \frac{\partial \theta}{\partial t}; \quad D(\theta) = K \frac{\partial h}{\partial \theta} \rightarrow \text{hydraulic diffusivity}$$

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So, far we have seen several preliminary expressions for transient flows through unsaturated soils, but then 1931 researcher called a Richards had given an expression for transient flow through unsaturated soils, this expression is even now it is used.

This is based on Darcy's generalized Darcy's law and combining with mass conservation expression which I discussed earlier also which involves considering unit volume, considering a representative volume when the flow takes place through this representative volume.

So, Q_z in the upward direction and $Q_{z+\Delta z}$ because this quantity is Δz . And this is a Δx and other direction is a Δy . So, the flux which is this is volumetric flux. So, the volumetric flux which is going into the element is volumetric flow rate Q_z of the water which is entering into the element, elemental volume is there is Darcy's velocity Q_z times cross sectional area $\Delta x \Delta y$.

Similarly, the amount which is coming out is $Q_{z+\Delta z}$ there is some change in the flux due to Q_z by Δz times Δx and Δy . So, this change exits there is what is the difference between these two the difference is $Q_{z+\Delta z} - Q_z$ by Δz times $\Delta x \Delta y \Delta z$.

So, this difference exists if this is a Transient flow. So, in the Transient flows this is equal to what. So, this is Q has units of metre per second. So, this is equal to this quantity is equal to the change in volume of water with time because there is a change in the volume of water, when water content increases in this particular element then the

volumetric flow rate also changes. So, therefore, in Transient flows this is equal to this if you look at the dimensions this has units of metre per second and this has metre and this is metre Q.

So, this has units of metre cube per second and here volume of water is metre cube and t equal to time. So, dimensionally this form is correct this equation is correct, now this can be written as $\frac{d\theta}{dt}$ times volume element because theta is defined as volume of water by total volume.

So, this can be written as this. So, therefore, the expression which you get using mass conversation is $\frac{dq}{dz}$ is equal to $\frac{d\theta}{dt}$ using mass conversation; if you use generalize Darcy's law to express q then this becomes $\frac{dq}{dz}$ of minus k_s gradient. So, there is d or $\frac{dh}{dz}$ this is a head the hydraulic potential varies in the vertical direction that is why the flow is taking place only in vertical direction. So, that is why the gradient is $\frac{dh}{dz}$ is equal to $\frac{d\theta}{dt}$.

So, this expression $\frac{dq}{dz}$ of sorry, this should not be k_s this is k. So, this is unsaturated hydraulic conductivity times k times $\frac{dh}{dz}$ is equal to $\frac{d\theta}{dt}$ this expression is Richards expression Richards equation this is very famous equation which is even now used for solving Transient flow problems.

So, this original expression is in mixed form it is called mixed form because the dependent variables on one side the right hand side is theta volumetric water content and the dependent variable on the left hand side is h suction head or looks are present as dependent variables this form is called mixed form. This can be written in the other form as a $\frac{dq}{dz}$ of k $\frac{dh}{dz}$.

By using chain rule you can write this as $\frac{dh}{dt}$ times $\frac{d\theta}{dh}$. So, this quantity is a slope of your soil water (Refer Time: 34:49) of c theta it is called its quantity is called specific moisture capacity. So, this is head form because when you have a smooth soil water characteristic curve function smooth function of soil water characteristic curve the slope can be determined and then if you solve you will get h of z comma t using this expression so, this is called head form.

Similarly you can write this expression as $\frac{dq}{dz}$ of k this can be modified again using chain rule. This can be written as $\frac{dh}{dt}$ times $\frac{d\theta}{dz}$ is equal to

$\frac{\partial \theta}{\partial t}$. So, this particular quantity is called diffusivity, hydraulic diffusivity $D\theta$ which may be different from diffusion coefficient which is used in and diffusion this has units of metre square per second.

Because it has units of metre per second and h has units of metre this as units of metre square per second this is called hydraulic diffusivity. So, this expression finally, is $\frac{\partial \theta}{\partial z}$ of $D\theta$ times or I will write it here this expression is $\frac{\partial \theta}{\partial z}$ of D of θ $\frac{\partial \theta}{\partial z}$ by $\frac{\partial \theta}{\partial z}$ is equal to $\frac{\partial \theta}{\partial t}$ this form if you solve you get θ of z t . So, this is called θ form.

Many researchers have studied all this by using several numerical techniques to solve these expressions and get the solutions, they found that the gives good solution because it does not approximate any mass conservation principles, but head form was found to be not conserving the mass and which gives erroneous results. And this particular form we have utilized earlier for when we discussed multi step out flow technique this is again simplified further into this particular form.

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TRANSIENT FLOWS

o Richards (1931):

$$\frac{\partial}{\partial z} \left(D(\theta) \frac{\partial \theta}{\partial z} \right) = \frac{\partial \theta}{\partial t} ; \theta = \theta(h)$$

$$D(\theta) \frac{\partial^2 h}{\partial z^2} = \frac{\partial h}{\partial t}$$

↓ $K(\theta)$

(horizontal flow) $\frac{\partial}{\partial z} \left(K \frac{\partial h}{\partial z} \right) = \frac{\partial \theta}{\partial t} ; h = h_m + z$

$$\frac{\partial}{\partial z} \left(K \frac{dh}{dz} + K \right) = \frac{\partial \theta}{\partial t} \quad (\text{vertical flow})$$

↑ z

Dr. T. V. Bijlavit

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$\frac{\partial \theta}{\partial z}$ of D of θ $\frac{\partial \theta}{\partial z}$ is equal to $\frac{\partial \theta}{\partial t}$. This particular form when we simplified, we assumed that the variation of D with respect to z is ignorable then are we assume that the variation of θ with respect to h is linear. Then we got an expression for $\frac{\partial^2 h}{\partial z^2}$ equal to $\frac{\partial h}{\partial t}$. And then Richards provided analytical solution for this particular expression and we

estimated $\frac{d\theta}{dz}$ and from knowing $D \frac{d\theta}{dz}$ and solving characteristic we estimated $\frac{d\theta}{dt}$ earlier.

So, this Richards expression is a very popular expression popular equation which is used for solving many problems. In this expression $\frac{d}{dz} \left(D \frac{d\theta}{dz} \right) = \frac{\partial \theta}{\partial t}$ is equal to $\frac{d\theta}{dt}$ is a mixed form we wrote, if the flow is taken place in vertical direction this will be transformed into k here h consist of both metric suction head and plus elevation head.

If we considered upward positive then if there is a infiltration taking place in upward direction or evaporation taking place from initially saturated soil then this expression becomes $\frac{d}{dz} \left(D \frac{d\theta}{dz} \right) + k$ is equal to $\frac{d\theta}{dt}$. If you considered this is negative then you get minus k .

Because when we substitute h is equal to $h_m + z$ you get this expression. So, this is expression for vertical flows. Vertical flows or the influence of gravity is considered, this expression is valid for horizontal flows or the influence of gravity is ignored.

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TRANSIENT FLOWS

- Boltzmann transformation (Rumynin, 2012):

$$\frac{\partial}{\partial x} \left(D \frac{\partial \theta}{\partial x} \right) = \frac{\partial \theta}{\partial t}; \quad \text{B.C. } \theta(x=0, t) = \theta_s; \quad \theta(x \rightarrow \infty, t) = \theta_i$$
- $\theta(x, t) \rightarrow \theta(\lambda)$
- where $\lambda = \frac{x}{\sqrt{4Et}} \rightarrow$ Boltzmann transformation
- $\frac{\partial \theta}{\partial x} = \frac{d\theta}{d\lambda} \left(\frac{\partial \lambda}{\partial x} \right) = \frac{d\theta}{d\lambda} \left(\frac{1}{\sqrt{4Et}} \right)$
- $\frac{\partial \theta}{\partial t} = \frac{d\theta}{d\lambda} \left(\frac{\partial \lambda}{\partial t} \right) = \frac{d\theta}{d\lambda} \left(-\frac{x}{2t^{3/2} \sqrt{4E}} \right)$
- $\frac{1}{\sqrt{4Et}} \frac{d}{d\lambda} \left(D \frac{d\theta}{d\lambda} \right) = -\frac{\lambda}{2t} \frac{d\theta}{d\lambda}$
- $\frac{\lambda}{2} \left(\frac{d\theta}{d\lambda} \right) + \frac{d}{d\lambda} \left[D \frac{d\theta}{d\lambda} \right] = 0$
- $D(\theta) = -\frac{1}{2} \frac{d\lambda}{d\theta} \int_{\theta_i}^{\theta_s} \lambda(\theta) d\theta$
- $\theta(x, t) \rightarrow$ measured

Rumynin, V. G. (2012) "Subsurface Solute Transport Models and Case Histories: With Applications to Radionuclide Migration", Springer Science & Business Media

For the expression which I just discussed that is theta form that is $\frac{d}{dz} \left(D \frac{d\theta}{dz} \right) = \frac{d\theta}{dt}$ for this particular theta form researchers derived analytical solution earlier so, which is called Boltzmann transformation.

So, this particular derivation can be found in Lu Likos text book or many other resources like Rumynin which is published in 2012 on subsurface solute transport models and case histories with applications to radionuclide migration and many other researchers in many other literature also this particular formulation is provided. And all us most of the soil science literature we will have the derivation for this particular form of expression form of equation using Boltzmann transformation.

So, because this is a partial differential equation the analytical solution for this particular non-linear form of equation is not possible difficult.

So, therefore, if these two dependent variables can be transformed into z and t for example, this is a horizontal flow then it will be better if you write x that means it is a horizontal flow. So, the influence of elevation are gravity is not there. So, for this particular form you have two independent variables like x and t .

If these independent variables can be transformed into one independent variable then this can be this will become simply ordinary differential equation then solutions may be possible. So, this concept is utilized based on Boltzmann transformation. So, here the simple boundary conditions are considered, the boundary conditions are θ_x is equal to 0 and time any given time this is equal to θ_s and θ at x is equal to infinity at any given time this equal to 0.

So, this means that you have a infinite column that means, very long column long soil column which is connected to you may have some porosity or a something here porous plate and which is connected to a reservoir.

So, when the flow takes place through the soil mass you discard the experiment or you stop the experiment before the flow reaches the other boundary this x is equal to infinity boundary and this x is equal to 0, at time t is equal to any given time. So, θ is equal to θ_s all the times.

So, at this point this is a saturated water content this is saturated water content at any given time and the flow does not reach the other boundary. So, now, the independent variables θ of x t is a independent variables the independent variables x t if they can be transformed θ of x t can be transformed to one independent variable that is λ send a where λ is equal to x y square root of t this is called Boltzmann

transformation. This is called Boltzmann transformation now how to convert this. So, the $\frac{d\theta}{dx}$ terms.

So, this can be written as $\frac{d\theta}{d\lambda} \frac{d\lambda}{dx}$ using chain rule, but as the θ is not dependent on only one single independent variable this will be d instead of dx . So, this is $\frac{d\theta}{d\lambda}$ and this is solved $\frac{d\theta}{d\lambda} \frac{d\lambda}{dx}$.

So, if λ is equal to $x + \sqrt{t}$ this is $\frac{d\theta}{d\lambda} \frac{d\lambda}{dx}$ is simply $\frac{1}{\sqrt{t}}$. Similarly $\frac{d\theta}{d\lambda}$ is equal to $\frac{d\theta}{dt} \frac{dt}{d\lambda}$ $\frac{d\theta}{dt}$ is equal to $\frac{d\theta}{d\lambda} \frac{d\lambda}{dt}$.

So, this is equal to $\frac{d\theta}{d\lambda}$ times this one is nothing, but minus x by t root t because the derivation of D by dx of $\frac{1}{\sqrt{x}}$ is $-\frac{1}{2} x^{-3/2}$ and because this is a x power minus half and the derivative is simply minus half and x power minus $\frac{3}{2}$. This can be written as $-\frac{1}{2} x^{-3/2}$.

So, similarly here x is t . So, therefore, $\frac{d\theta}{d\lambda}$ into minus x minus there and you are derivating partial derivative only derivative for t is considered. So, therefore minus x by t square root of t .

So, if you substitute these into the main expressions then $\frac{d\theta}{dx}$ gives $\frac{d\theta}{d\lambda} \frac{1}{\sqrt{t}}$ and $\frac{d\theta}{dx}$ is again $\frac{d\theta}{d\lambda} \frac{1}{\sqrt{t}}$ can be taken out and this expression which is equal to.

So, $\frac{d\theta}{dt}$ is minus x by square root of t is again λ from the Boltzmann transformation. So, this is simply minus λ by t $\frac{d\theta}{d\lambda}$ if you get two here. So, as this t gets cancelled on the either side this expression simply becomes λ by 2 $\frac{d\theta}{d\lambda}$ plus $\frac{d\theta}{d\lambda}$ of $\frac{d\theta}{d\lambda}$ is equal to 0 .

So, this is ordinary differential equations this can be solved by integrating the boundary conditions will be transform to θ of x is equal to 0 means λ is equal to 0 this is equal to θ s. And similarly when x is equal to reaches infinity λ also reaches

infinity which approaches to infinity which is equals to theta initial; initial value could be 0 or anything.

So, then if you integrated then you get value for if you integrate twice then you get d theta. So, then d theta can be written as minus half d lambda by d theta at theta theta initial to theta s lambda of theta times d theta. So, this is the solution for the theta form of Richards equations by considered the Boltzmann transformation.

So, essentially when you conduct such an experiment when you obtain the variation of theta with x and time if you get then this can be transformed to this can be written as theta of lambda if that information is known.

So, how lambda varies with theta or theta varies with lambda that is known. So, because a lambda is simply express square root of time. So, for different values of x and different values of time if you get the data of theta then this can be compiled and theta of lambda or lambda of theta can be written, once the data is substituted here you can get d of theta there is a diffusivity.

How diffusivity changes with volumetric water content can be obtained using this particular expression. So, from the measurements of theta of x t is measured from the data from this particular data which is converted to theta of lambda or lambda of theta. So, then from this using this expression d of theta can be obtained the diffusivity changes with volumetric water content.

Thank you.