

**Geotechnical Earthquake Engineering**  
**Prof. Deepankar Choudhury**  
**Department of Civil Engineering**  
**Indian Institute of Technology, Bombay**

**Module - 5**  
**Lecture - 19**  
**Wave Propagation (Contd...)**

Let us start our today's lecture of this NPTEL video course on geotechnical earthquake engineering. On this course, we are now going through our module five, which is wave propagation.

(Refer Slide Time: 00:44)

Prof. D. Choudhury  
Lec. 19 (A)

Particle velocity,

$$\dot{u} = \frac{\partial u}{\partial t} = \frac{\epsilon_x \cdot \partial x}{\partial t}$$
$$\therefore \epsilon_x = \frac{\partial u}{\partial x} = \frac{\sigma_x}{M} \cdot \frac{\partial x}{\partial t}$$
$$\ddot{\sigma}_x = M \cdot \epsilon_{xx} = \frac{\sigma_x}{M} \cdot v_p$$

NPTEL

DC/1

Let us quickly recap what we have learnt in our previous lecture. In the previous lecture, we have seen for one-dimensional wave propagation in an infinite rod; for the case of longitudinal wave, what is the particle velocity; how we can estimate the particle velocity from the displacement function; and using the strain displacement relationship as well as the stress versus strain relationship, we arrived at this relationship. And the  $v_p$  is nothing but the primary wave velocity or p wave velocity or longitudinal wave velocity, which is expressed as root over  $M$  by  $\rho$ .

(Refer Slide Time: 01:33)

Handwritten derivation on a whiteboard:

$$\dot{u} = \frac{\partial u}{\partial t} = \frac{\sigma_x}{M} \cdot v_p$$

$$= \frac{\sigma_x}{\rho v_p^2} \cdot v_p$$

Since  $\frac{M}{\rho} = v_p^2$ , we have:

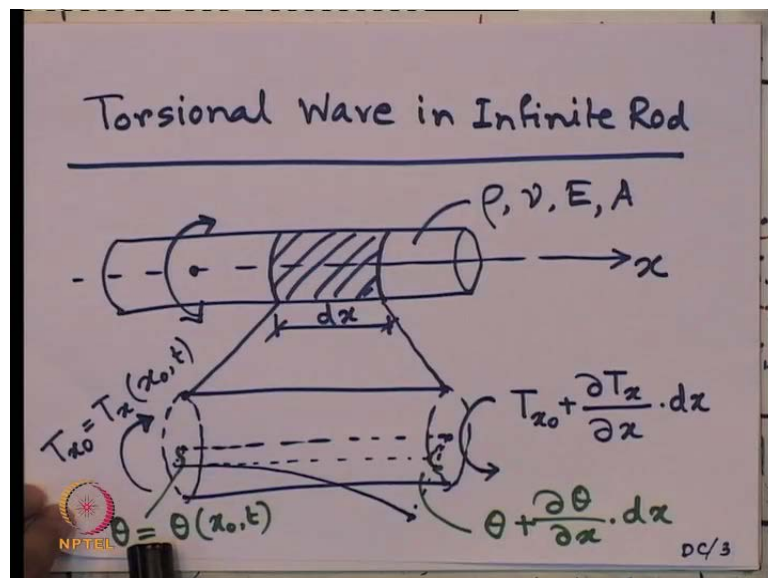
$$\dot{u} = \frac{\sigma_x}{\rho v_p}$$

$\rho v_p \rightarrow$  specific impedance

NPTEL logo and DC/2 are visible at the bottom.

So, this is the relationship as we have seen earlier. As I was mentioning,  $v_p$  is nothing but root over  $M$  by  $\rho$ ;  $M$  is nothing but constrain modulus;  $\rho$  is the density of the material. And this is the relationship between the particle velocity with respect to stresses. And this parameter – the multiplication of density with respect to the velocity of the wave of this  $p$  wave – that function  $\rho v_p$  is known as specific impedance. So, that is the specific impedance of the material.

(Refer Slide Time: 02:14)



Then in the previous lecture, we have derived what will be the governing equation of motion for a torsional wave when it travels through an infinite rod in one dimension. So, this is the derivation we have seen, the difference of torque at both the ends of this infinitesimal element of the rod with the different magnitude of rotational displacement theta at this end and at this end.

(Refer Slide Time: 02:49)

$$\begin{aligned}
 & -T_{x_0} + \left[ T_{x_0} + \frac{\partial T_x}{\partial x} \cdot dx \right] \\
 & = (\rho \cdot J \cdot dx) \cdot \frac{\partial^2 \theta}{\partial t^2} \quad [ \because dx \neq 0 ] \\
 & \boxed{ \frac{\partial T_x}{\partial x} = \rho J \cdot \frac{\partial^2 \theta}{\partial t^2} }
 \end{aligned}$$

↑ Torque
↑ Rotation

DC/4

Then, after considering the equilibrium, we had derived at this relationship of torque versus rotation in this form; where, J is the polar moment of inertia.

(Refer Slide Time: 03:02)

$$\begin{aligned}
 T_x & = GJ \cdot \frac{\partial \theta}{\partial x} \Rightarrow \frac{\partial T_x}{\partial x} = GJ \cdot \frac{\partial^2 \theta}{\partial x^2} \\
 & \text{shear modulus} \\
 \frac{\partial^2 \theta}{\partial t^2} & = \frac{1}{\rho J} \cdot \frac{\partial T_x}{\partial x} \\
 \text{or, } \frac{\partial^2 \theta}{\partial t^2} & = \frac{1}{\rho J} \cdot GJ \cdot \frac{\partial^2 \theta}{\partial x^2} \quad ( \because J \neq 0 )
 \end{aligned}$$

DC/5

After that, we had applied the relationship between torque and rotational displacement through the shear modulus and polar moment of inertia in this form.

(Refer Slide Time: 03:19)

The image shows a whiteboard with handwritten mathematical equations. At the top, the equation is  $\frac{\partial^2 \theta}{\partial t^2} = \left( \frac{G}{\rho} \right) \frac{\partial^2 \theta}{\partial x^2}$ . Below this, it says  $\frac{G}{\rho} = v_s^2$ , where  $v_s \rightarrow$  shear wave velocity. Then,  $v_s = \sqrt{\frac{G}{\rho}}$  is written. At the bottom, the equation  $\frac{\partial^2 \theta}{\partial t^2} = v_s^2 \frac{\partial^2 \theta}{\partial x^2}$  is boxed. In the bottom left corner of the whiteboard, there is a small logo for NPTEL. In the bottom right corner, the text 'DC/6' is written.

And on further simplification, what we had arrived finally, is this one; where, this parameter again we have defined as square of shear wave velocity. So, shear wave velocity is nothing but root over G by rho, where G is the shear modulus of the material and rho is the density of the material. And hence the governing equation of motion in terms of the acceleration, this is the rotational acceleration; and this is the second order differential of the rotational displacement with respect to the x-axis system or space coordinate system related through the shear wave velocity of the media for this torsional wave.



We have also seen for harmonic loading, this is the response what we have obtained; where, we have defined this parameter  $K$  as a wave number, which is nothing but the ratio of the circular frequency to the velocity of wave in that medium.

(Refer Slide Time: 05:24)

The image shows a whiteboard with handwritten mathematical derivations. At the top, the equation  $\lambda = vT = \frac{v}{f} = \frac{2\pi \cdot v}{\omega}$  is written. An upward-pointing arrow from the word "wavelength" below points to the  $\lambda$  in the equation. Another upward-pointing arrow from the word "wave number" below points to the  $k$  in the equation  $= \frac{2\pi}{k}$ . Below this, two more equations are shown:  $\lambda = \frac{2\pi}{k} \rightarrow$  'x' co-ordinate system and  $T = \frac{2\pi}{\omega} \rightarrow$  't' co-ordinate system. The NPTEL logo is visible in the bottom left corner, and "04/2" is written in the bottom right corner.

$$\lambda = vT = \frac{v}{f} = \frac{2\pi \cdot v}{\omega}$$

↑  
wavelength

$$= \frac{2\pi}{k}$$

↑  
wave number

$$\lambda = \frac{2\pi}{k} \rightarrow \text{'x' co-ordinate system}$$
$$T = \frac{2\pi}{\omega} \rightarrow \text{'t' co-ordinate system}$$

NPTEL 04/2

We had also defined another parameter, which is known as wavelength – lambda, which is related to the wave number in this format. And, we have seen the similarity between the relationships of this wavelength lambda with respect to wave number  $K$ . And, in the time co-ordinate system, we have seen the relationship between time period with respect to the natural circular frequency.

(Refer Slide Time: 05:59)

The slide is titled "Boundary Effects". It features a diagram of a horizontal line representing a centerline. A vertical line intersects this horizontal line at a central point, labeled  $u = 0$ . To the left of the centerline, there is a grey rectangular pulse with a left-pointing arrow above it labeled  $\sigma_0$ . To the right of the centerline, there is a grey rectangular pulse with a right-pointing arrow above it labeled  $\sigma_0$ . Below the diagram, the text reads: "At centerline, displacement is always zero" and "Stress doubles momentarily as waves pass each other". The NPTEL logo is in the bottom left, and "IIT Bombay, DC" and the number "6" are in the bottom right.

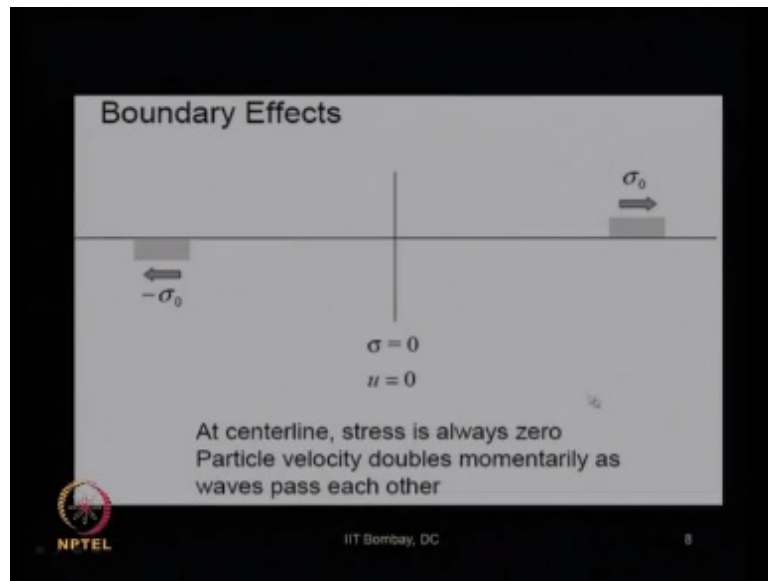
After that, what have seen in the previous lecture, we can see the slide here; we have considered various boundary conditions like this, where centerline, where displacement will be 0.

(Refer Slide Time: 06:09)

The slide is titled "Boundary Effects (Fixed End)". It features a diagram of a horizontal line representing a centerline. A vertical line intersects this horizontal line at a central point, labeled  $u = 0$ . To the left of the centerline, there is a grey rectangular pulse with a left-pointing arrow above it labeled  $\sigma_0$ . To the right of the centerline, there is a dark grey rectangular pulse with a right-pointing arrow above it labeled  $\sigma_0$ . Below the diagram, the text reads: "Response at boundary is exactly the same as for case of two waves of same polarity traveling toward each other" and "At fixed end, displacement is zero and stress is momentarily doubled. Polarity of reflected wave is same as that of incident wave". The NPTEL logo is in the bottom left, and "Reference : Kramer (1996)", "IIT Bombay, DC", and the number "7" are in the bottom right.

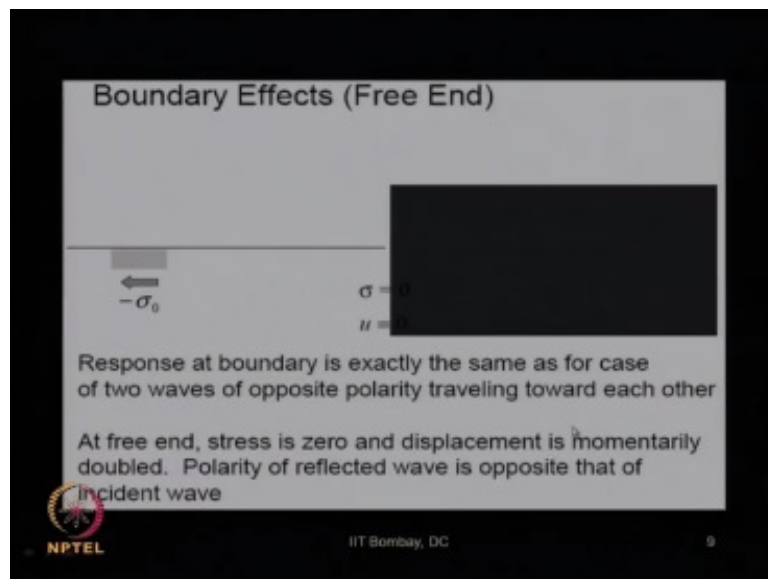
Then at the fixed end, where the displacement has to be 0.

(Refer Slide Time: 06:16)



Also, a boundary effect, where both stress and displacement has to be 0.

(Refer Slide Time: 06:22)



Then at the free end, where stress has to be 0.



(Refer Slide Time: 06:27)

**Boundary Effects (Material Boundaries)**

At material boundary, displacements must be continuous

$$A_i + A_r = A_t$$

equilibrium must be satisfied

$$\sigma_i + \sigma_r = \sigma_t$$

Reference : Kramer (1996) IIT Bombay, DC

NPTEL 10

Then, we have seen the boundary effect, that is, material boundary when there is a layered body, how this incident wave; then, transmitted wave and reflected wave – they are related to each other through this example.

(Refer Slide Time: 06:40)

**Waves in a Layered Body**

One-dimensional case: material boundary in an infinite rod

Incident, Reflected, Transmitted

One-dimensional wave propagation at material interface. Incident and reflected waves travel in opposite directions in material 1. The transmitted wave travels through material 2 in the same direction as the incident wave.

Reference : Kramer (1996) IIT Bombay, DC

NPTEL 11

That is, for one-dimensional case, when wave is travelling in this one dimension in a layered body like this, we have seen this is material 1; this is material 2. We have defined the x co-ordinate system in such a way that this is the positive x-direction and this boundary itself is the x equals to 0. And, incident wave comes from material 1. Some of

that goes in the material 2 as transmitted wave; and, some of the wave comes back in the same medium as reflected wave.

(Refer Slide Time: 07:22)

**Waves in layered body (contd.)**

The incident wave can be described by,

$$\sigma_I(x, t) = \sigma_i e^{i(\omega t - k_1 x)}$$

The transmitted and the reflected wave can be described by

$$\sigma_T(x, t) = \sigma_t e^{i(\omega t - k_2 x)}$$


$$\sigma_R(x, t) = \sigma_r e^{i(\omega t - k_1 x)}$$

Assuming that the displacements associated with each of these waves are of the same harmonic form as the stresses,

$$u_I(x, t) = A_i e^{i(\omega t - k_1 x)}$$

$$u_R(x, t) = A_r e^{i(\omega t - k_1 x)}$$

$$u_T(x, t) = A_t e^{i(\omega t - k_2 x)}$$

 Reference : Kramer (1996) 12

Then, we have seen how these stresses of incident transmitted and reflected wave can be represented in a harmonic fashion; also, how the displacement functions can be represented through this form.

(Refer Slide Time: 07:38)

Stress-strain and strain-displacement relationships can be used to relate the stress amplitudes to the displacement amplitudes

$$\sigma_I(x, t) = M_1 \frac{\partial u_I(x, t)}{\partial x} = -ik_1 M_1 A_i e^{i(\omega t - k_1 x)}$$

$$\sigma_R(x, t) = M_1 \frac{\partial u_R(x, t)}{\partial x} = +ik_1 M_1 A_r e^{i(\omega t - k_1 x)}$$

$$\sigma_T(x, t) = M_2 \frac{\partial u_T(x, t)}{\partial x} = -ik_2 M_2 A_t e^{i(\omega t - k_2 x)}$$

From these the stress amplitudes are related to the displacement amplitudes by

$$\sigma_i = -ik_1 M_1 A_i$$

$$\sigma_r = +ik_1 M_1 A_r$$


$$\sigma_t = -ik_2 M_2 A_t$$

At the interface, both compatibility of displacements and continuity of stresses must be satisfied. The former requires that

$$u_I(0, t) + u_R(0, t) = u_T(0, t)$$

And the later one,

$$\sigma_I(0, t) + \sigma_R(0, t) = \sigma_T(0, t)$$

 Reference : Kramer (1996) 13

Then, applying the known stress-strain relationship through this modulus; that is, when we are talking about longitudinal wave, we have to take constraint modulus like this. So,

we have obtained what are the relationships between amplitudes of these stresses and displacement. So, this is the relationship of amplitude only – stress amplitude with respect to the displacement amplitude.

Also, we had seen in the previous lecture that, at the interference of two materials, this boundary condition has to be satisfied; that is, the displacement compatibility and equilibrium of stress. So, these two conditions has to be maintained. Hence, this incident wave displacement at  $x$  equals to 0 plus the reflected wave displacement at  $x$  equals to 0 should be equals to the transmitted wave displacement at  $x$  equals to 0. So, that is from the displacement compatibility. Whereas, from equilibrium of stress condition, we will get stress due to that incident wave at  $x$  equals to 0 plus stress due to the reflected wave at  $x$  equals to 0 should be equals to the stress due to the transmitted wave at  $x$  equals to 0.

(Refer Slide Time: 09:05)

$$A_i + A_r = A_t$$

$$\sigma_i + \sigma_r = \sigma_t$$

At the interface, using the relationship  $kM = \omega \rho v$ , gives

$$-\rho_1 v_1 A_i + \rho_1 v_1 A_r = -\rho_2 v_2 A_t = -\rho_2 v_2 (A_i + A_r)$$

Above equation can be rearranged to relate the displacement amplitude of the reflected wave to that of the incident wave

$$A_r = \frac{\rho_1 v_1 - \rho_2 v_2}{\rho_1 v_1 + \rho_2 v_2} A_i = \frac{1 - \rho_2 v_2 / \rho_1 v_1}{1 + \rho_2 v_2 / \rho_1 v_1} A_i$$

And knowing  $A_i$  and  $A_r$  to determine  $A_t$  as

$$A_t = \frac{2 \rho_1 v_1}{\rho_1 v_1 + \rho_2 v_2} A_i = \frac{2}{1 + \rho_2 v_2 / \rho_1 v_1} A_i$$

NPTEL Reference : Kramer (1996) IIT Bombay, DC 14

Now, let us continue this in today's lecture further. So, what we can write from the previous known relationship that, if we equate the amplitudes, the displacement amplitude of incident wave plus displacement amplitude of reflected wave should be equals to the displacement amplitude of transmitted wave. In the similar form, this is from the displacement compatibility relationship. And, from the equilibrium of stress relationship, we get the amplitude of incident wave plus amplitude of reflected wave should be equals to the amplitude of this transmitted wave.

Now, at the interface, using the known relationship of this one, what we can write further? By putting the values of this  $A_i$  and  $A_r$  with  $A_t$ , we will get this relationship. So, on further rearrangement, if we do the further rearrangement of this relationship of displacement amplitude to the reflected and incident wave with respect to the transmitted wave, what we will get? Suppose this reflected wave amplitude we are interested to know, because in most of the cases, what will be known? The incident wave properties will be known. From a media, we will know what is the stress and displacement of incident wave. We have to now find out what are the stresses and displacement in the reflected and transmitted wave. That is what we are more concerned about in practice. So, this is the co-relation.

If you put this simple relationship in this equation, then on further simplification, you will get like this. And, knowing this  $A_i$  and  $A_r$  to determine as... So,  $A_t$  also you can determine with respect to  $A_i$  in this form, because  $A_r$  we have represented in this format. Hence, you can represent the transmitted wave amplitude also in terms of this incident wave amplitude, displacement amplitude.

Now, let us look at this parameter very carefully. This is  $\rho v$ ; that is, density times the velocity. What is that? We have already mentioned. That is nothing but in the precious lecture, we discussed. It is to specific impedance. This is nothing but specific impedance of material 2. And, this is the specific impedance of material 1.

(Refer Slide Time: 11:58)

Define impedance ratio as  $\alpha_z = \rho_2 v_2 / \rho_1 v_1$

The displacement amplitudes of the reflected and transmitted waves are

$$A_r = \frac{1 - \alpha_z}{1 + \alpha_z} A_i$$

$$A_t = \frac{2}{1 + \alpha_z} A_i$$

$$A_i = -\frac{\sigma_i}{ik_1 M_1}$$


$$A_r = -\frac{\sigma_r}{ik_1 M_1}$$

$$A_t = -\frac{\sigma_t}{ik_2 M_2}$$

Stresses can be expressed as,

$$\sigma_r = \frac{\alpha_z - 1}{1 + \alpha_z} \sigma_i$$

$$\sigma_t = \frac{2\alpha_z}{1 + \alpha_z} \sigma_i$$

 NPTEL © Kramer (1996)

15

So, that ratio, which is called as impedance ratio; so what is impedance ratio? The definition of impedance ratio is nothing but it is the ratio of specific impedance of two materials. And, when we are taking that ratio, always in the numerator, there will be the material, which is in the transmitted zone; that is, the incident wave, where from the wave is coming that will be in the denominator. So, that is why, you can see this  $\rho_1 v_1$  for material 1 is in the denominator; that is the incident wave region; that is the specific impedance of the material, where the incident wave is coming from. And,  $\rho_2 v_2$  is the specific impedance of material 2, where the transmitted wave goes in. So, that is the definition of impedance ratio. This is a very important parameter because this defines through the material parameter of two materials. This gives us the idea, how much displacement amplitude of transmitted wave and how much stress amplitude of transmitted wave will occur for a particular incident wave. Why this is important? As I am mentioning, because suppose when earthquake occurs at a large depth; so, basically, from base rock level or bed rock level, that wave will start travelling where that earthquake focus is existing through this form of seismic waves.

Now, when this wave travels and finally, comes close to the ground surface, we should know that after travelling through various layers of material, that is, from rock to stiff soil to soft soil typically, what will be the changes in the displacement amplitude and stress amplitude of the stresses, because that is most important; we need to find out. So, there this use of impedance ratio is very important, which we will be easily able to obtain from knowing the density and that particular velocity of the wave. Suppose we are talking about shear wave velocity, we have to take shear wave velocity of two materials. If we are talking about the primary wave velocity, we have to take corresponding primary wave velocity in the two medium like that. So, we have to find out the impedance ratio of the two materials. But, remember, the impedance ratio for primary wave or shear wave, etcetera will remain same for between two given materials. Why it will not vary between primary and secondary? Because if we have remembered the relationship between primary wave and secondary wave that is, through a constant relationship between the ratio. So, it is the ratio of two similar parameters. So, that is why, it gives the similar value of impedance ratio whether it is a p wave or s wave.

Now, what we can see from this (Refer Slide Time: 15:38) slide, the displacement amplitude of the reflected and transmitted waves – they can be represented through this

impedance ratio in this simplest format. So, these two equations are very important; we should be able to find out what is the displacement amplitude of the reflected wave with respect to the incident wave amplitude, displacement amplitude over this impedance ratio; and, what is the displacement amplitude of the transmitted wave related to this displacement amplitude of the incident wave through this impedance ratio. That is why impedance ratio is so important.

And, how to estimate this incident wave displacement amplitude? If you know the loading condition or the stresses, which are getting developed for the incident wave, this is the stress amplitude. This is how the displacement is related to the stress amplitude in this format; that we have already seen the derivation in the previous lecture. Also, for the reflected wave, displacement amplitude is related to the reflected wave stress amplitude in this form. And, the transmitted wave displacement amplitude is related to the transmitted wave stress amplitude in this form through their modulus.


Now, stresses – they can be expressed in the similar fashion like through this impedance ratio like this; that is, the reflected wave stress can be obtained using this relationship for a given stress amplitude of incident wave. Also, the transmitted wave stress amplitude can be obtained from the incident wave stress amplitude through this impedance ratio. So, what does it mean? If suppose we know the stress amplitude of the incident wave of any particular earthquake and if we know the material property of two different layers of media, we can easily estimate all other parameters; can you see? So, only parameter we should know is this  $\sigma_i$ . So, if  $\sigma_i$  is known, you can compute from  $\sigma_i A_i$ . Once you know  $A_i$ , you will get  $A_r A_t$ . Once you know  $\sigma_i$ , you will get  $\sigma_r$ ,  $\sigma_t$ .

(Refer Slide Time: 18:21)

**Table 5-1** Influence of Impedance Ratio on Displacement and Stress Amplitudes of Reflected and Transmitted Waves

Impedance Ratio, $\alpha_z$	Displacement Amplitudes			Stress Amplitudes		
	Incident	Reflected	Transmitted	Incident	Reflected	Transmitted
0	$A_i$	$A_i$	$2A_i$	$\sigma_i$	$-\sigma_i$	$\frac{0}{\sigma_i}$
$\frac{1}{3}$	$A_i$	$3A_i/5$	$8A_i/5$	$\sigma_i$	$-3\sigma_i/5$	$2\sigma_i/5$
$\frac{1}{2}$	$A_i$	$A_i/3$	$4A_i/3$	$\sigma_i$	$-\sigma_i/3$	$2\sigma_i/3$
1	$A_i$	0	$A_i$	$\sigma_i$	0	$\sigma_i$
2	$A_i$	$-A_i/3$	$2A_i/3$	$\sigma_i$	$\sigma_i/3$	$4\sigma_i/3$
4	$A_i$	$-3A_i/5$	$2A_i/5$	$\sigma_i$	$3\sigma_i/5$	$8\sigma_i/5$
$\infty$	$A_i$	$-A_i$	0	$\sigma_i$	$\sigma_i$	$2\sigma_i$

Reference : Kramer (1996)

 NPTEL IIT Bombay, DC 16

Let us look at this table. This table is taken from Kramer's book – Table 5.1. What are the influences of this impedance ratio value – that alpha z value on this displacement and stress amplitude of the reflected and transmitted waves. If you see, the impedance ratio equals to 0. What does it mean – impedance ratio equals to 0? Let us go back to the definition. Impedance ratio 0 means this is 0. So, in that case, incident wave displacement amplitude and reflected wave displacement amplitude – they remain same. Transmitted wave displacement amplitude is double of this. That you can easily get by putting alpha z equals to 0 in this equation. And, incident wave alpha i will be reflected wave minus alpha i; and, transmitted wave stress amplitude will be 0. What is this situation? Can you guess? It is nothing but a free boundary, where the stress at the boundary condition has to be 0. That is what it shows.

Suppose if we are talking about shear wave velocity. When this becomes 0, when v comes close to the ground surface, because it cannot travel in the air media. So, in that case, you will get this  $\rho_2 v_2$  equals to 0. So, that is nothing but when you come to a boundary and end of a material, there you will get... displacement amplitude gets doubled. But, stress amplitude – there will be 0; that is what.

(Refer Slide Time: 20:25)

The slide is titled "Boundary Effects (Free End)". It features a diagram of a horizontal line representing a boundary. To the left of the boundary, a grey rectangular pulse is shown moving to the left, indicated by a left-pointing arrow. Below this pulse is the label  $-\sigma_0$ . To the right of the boundary, the stress is labeled  $\sigma = 0$  and the displacement is labeled  $u = 0$ . Below the diagram, the text reads: "Response at boundary is exactly the same as for case of two waves of opposite polarity traveling toward each other". Further down, it states: "At free end, stress is zero and displacement is momentarily doubled. Polarity of reflected wave is opposite that of incident wave". The slide includes the NPTEL logo in the bottom left corner, "IIT Bombay, DC" in the bottom center, and the number "9" in the bottom right corner.

If you remember in our previous lecture, we have discussed this. We have mentioned that boundary effect of free end... What happens at the free end? Stress becomes 0, but displacement is doubled. Can you see this? So, that is what now we have seen through this mathematical expression also, which is valid.

Now, let us check another case. Impedance ratio equals to 1. What does it mean impedance ratio equals to 1? That means this material and this material are same; that is, there is no in fact, any material boundary. The wave is travelling through the same material. So, what is expected? Same incident wave only will continue. So, let us look at the displacement amplitude of incident wave  $A_i$  reflected is 0, which is quite obvious. Nothing should get reflected. Entire thing should continue as a transmitted. So, that is, the same wave will continue. So,  $A_i$  will continue. And, let us look at the stress conditions. Incident wave stress amplitude is  $\sigma_i$ . There is nothing called reflected. And, entire thing gets transmitted; that is, the same wave continues; so  $\sigma_i$ . So, these are the cross check whether this derivations of equations are correct or not through this common known conditions.

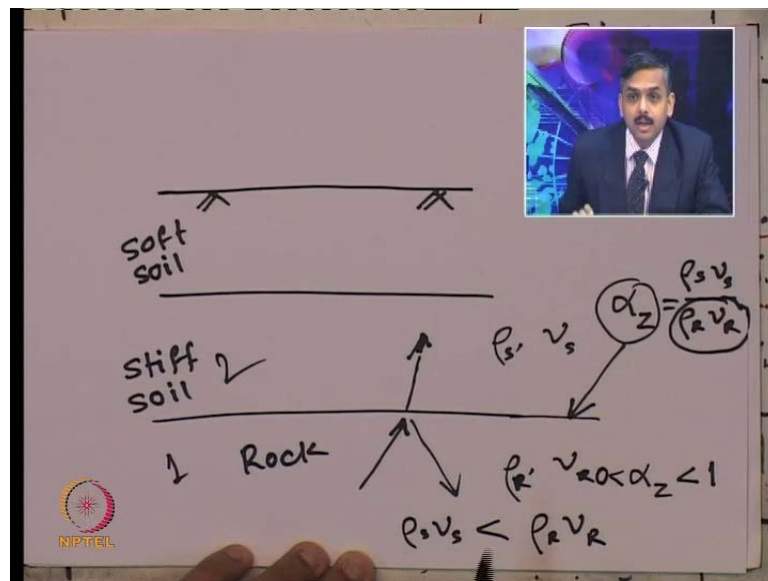
And, what is the impedance ratio of infinity? What does it mean when it will become infinity? Infinity means if this one becomes 0. Suppose there is a wave say p wave which travels in the air also goes to another media. So, we have incident wave displacement amplitude; reflected wave displacement wave is negative of that; that is, in opposite



direction. Negative sign indicates, it goes in the opposite direction. Always remember, all the negative sign indicates the opposite direction. Transmitted – nothing get transmitted. Whereas, for stress amplitude, what happens, the incident wave and reflected wave stress amplitude are same; for transmitted wave stress amplitude, it gets doubled.

Now, what are the common practical forms of impedance ratio for our geotechnical earthquake engineering? If we go back to this definition of impedance ratio, we will see typically when we come from layered soil...

(Refer Slide Time: 23:06)



Let us say here say we are coming from stiff soil, rock, soft soil. This is the typical profile. Now, from rock, your waves are coming – incident wave. So, some of that will get transmitted; some of that will get reflected. Now, depending on the values of this rho and v of this material and this material, rho of rock, v of rock and rho of soil, v of soil – which one will have more density? Of course, the rock compared to soil. Now, which one will have more velocity of wave? Of course, again rock compared to soil. So, which material will have more specific impedance? Of course, rock will have more specific impedance than the soil. So, when we are talking about this alpha z between these two layers... We are considering alpha z between these two layers, which is rho of soil v of soil by rho of rock v of rock, because generally, these waves are coming from here. So, this is our material 1 here and this is material 2. So, that is what we have seen in this

slide, the definition of impedance ratio is specific impedance of material 2 divided by specific impedance of material 1.

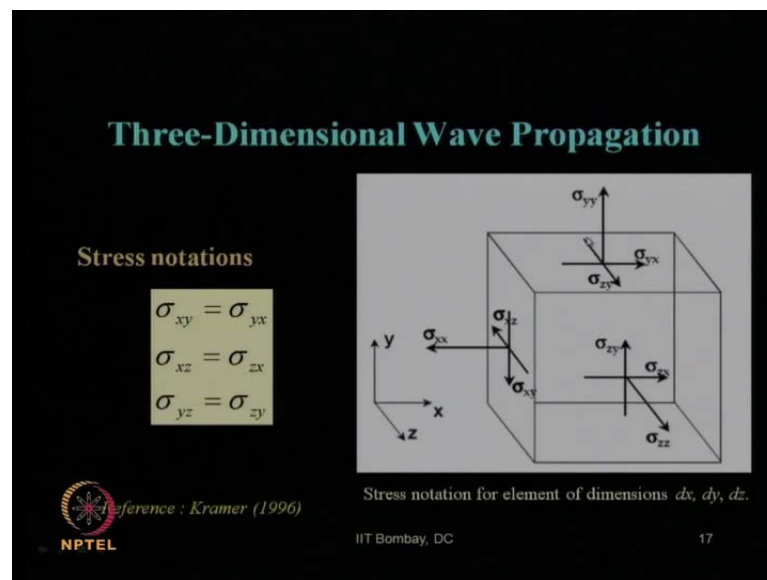
Now, which parameter is more? This parameter is much higher than this. So, now,  $\rho_s v_s$  is much lower than  $\rho_r v_r$ . That is the typical case for our geotechnical earthquake engineering, which practically we will get at site. So, at site, if we get this situation, obviously,  $\alpha_z$  should be... All practical cases will be less than 1. So, that is the practical range of  $\alpha_z$ . It should be between 0 to 1. Equality of 0 also we have seen; equality of 1 also we have seen. So, most practical site condition in terms of our geotechnical earthquake engineering is  $\alpha_z$  should be between 0 to 1. So, if  $\alpha_z$  is 0 to 1, let us see what happens.

Let us look at this table once again. So,  $\alpha_z$  value is between 0 to 1. So, these are the common ranges. As it is given, 0.25, 0.5... It can be different values based on the known soil parameters or rock parameters. That can be any value between 0 to 1. What happens in these cases? Let us say  $\alpha_z$  is 0.5. For that 0.5, what happens? When displacement amplitude incident wave is  $A_i$ , reflected is  $A_i$  by 3. Just putting that  $\alpha_z$  is equals to 0.5, you will get this; will get this also. Similarly, you will get this and this. That is what we are doing here. So,  $\alpha_z$  of 0.5 will give us reflected wave displacement amplitude much lower than what is the displacement wave amplitude for the incident wave. But, let us look at the transmitted wave. It is  $4 A_i$  by 3. What does it mean? That means the displacement amplitude of transmitted wave increased from that was there for the incident wave.

Let us look at the stresses. Stress incident wave is this one. Reflected wave is this minus, that is, opposite direction. And, transmitted wave stress is two-third of the incident wave. So, stress is not increasing, it is reducing for most of our practical geotechnical earthquake engineering problem. But, where the big difficulty arises? Difficulty arises with respect to this displacement. That is the reason an earthquake wave, which is not imperceptible or not that much damaging at large depth in rocks media. When it reaches through various soft soil, there will be many fold increase of this displacement amplitude of that wave. That creates more displacement when it reaches or goes through a stiffer to a softer media. That is the reason.

As I have already mentioned in the introductory lecture, Mexico city earthquake of 1985 is an example of this phenomenon of soil amplification; that amplification mostly occurs in the case of when you have a very soft soil. So, from bed rock to if the wave travels through various soil layers and finally, it goes through a soft soil media, you will see displacement amplitude of the transmitted wave in the soft media gets magnified, gets increased much more than what that incident wave was. So, we have to be very careful about these values, which we can easily estimate in the beginning itself knowing the material property of various layers. So, that is why, knowing the material property is very important to estimate the impedance ratio, which automatically will give us what will be the displacement amplitude of any transmitted wave.

(Refer Slide Time: 29:31)

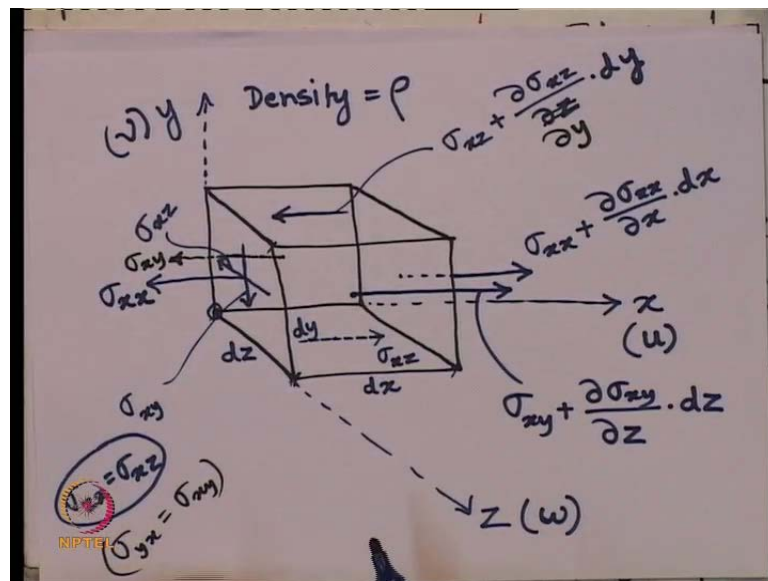


Now, let us come to the three-dimensional wave propagation. So, for three-dimensional wave propagation, let us first take a note of the stress notations. So, this is the three-dimensional element. So far, we have discussed about only one-dimensional wave propagation. Now, we are considering, wave is travelling in all the three directions. So, this is x-coordinate system, y-coordinate system and z-coordinate system. We have the normal stresses: in x-direction, sigma x x; in y-direction, sigma y y; in z-direction, sigma z z; and, the shear stresses like sigma x y. This indicates... As we know, the first parameter indicates the plane; the second one indicates in which direction it works. So, sigma x y means it is acting on x plane in y-direction. This is sigma x z – in x plane again, but in z-direction. So, these two are the shear stresses. Again, on this phase, we

can have  $\sigma_{yx}$ ,  $\sigma_{zy}$ . So, these are... This is actually  $\sigma_{yz}$  it should be; that is on  $y$  plane in  $z$ -direction.

Now, considering the stress notations of equivalence; that is, shear stresses of  $\sigma_{xy}$  should be equals to  $\sigma_{yx}$ . Why? Because to maintain the equilibrium of the body; otherwise, if they are different, there will be a formation of a couple. So, to maintain the equilibrium, the other phase shear stress has to be the same and in opposite direction; like  $\sigma_{xy}$  in the opposite direction should act over here; and, this  $\sigma_{yx}$  should be equals to  $\sigma_{xy}$  to maintain the equilibrium of the cube or of the soil element in three dimension. Similarly,  $\sigma_{xz}$  should be equals to  $\sigma_{zx}$  and  $\sigma_{yz}$  should be equals to  $\sigma_{zy}$ . So, that is why, these shear stresses over here:  $\sigma_{zy}$  in  $y$ -direction,  $\sigma_{zx}$  in  $x$ -direction. This is  $\sigma$  actually  $zy$  – it will be  $yz$ , because in  $z$ -direction. So, direction will come last. But, as we know,  $\sigma_{yz}$  equals to  $zy$ . That is why it is written as  $\sigma_{zy}$ . Similarly, this is  $\sigma_{yx}$  in the  $x$ -direction, which is equals to this  $\sigma_{xy}$ .

(Refer Slide Time: 32:42)



So, knowing this stress condition, let us start deriving; that is, for three-dimensional wave propagation, what will be the basic equation of motion that we are planning to find out. Now, let us take the stresses on the phase. Let us say this is our  $x$ -axis; this is our  $y$ -axis; and, this is our  $z$ -axis. Let us take only one direction first. Then, it will be easy to understand in the similar other directions as well. So, now, in the  $x$ -direction, we have at

this phase... So, this is basically the origin as you can see. From this phase let us say, the normal stress is  $\sigma_{xx}$ . And, what are the dimensions of various sides of this small infinitesimal element? Let us say in x-direction, it is  $dx$ ; in y-direction, let us say it is  $dy$ ; and, in z-direction, let us say it is  $dz$ . So, this to this is  $dx$ ; this to this is  $dy$ ; and, this to this is  $dz$ . These are the dimensions of the material. And, let us say, the density of the material is known as  $\rho$ . Corresponding displacements also, let us define. In x-direction, let us say, displacement is denoted as  $u$ ; in y-direction, let us say, displacement is denoted as  $v$ ; and, in z-direction, displacement is denoted as  $w$  –  $u$ ,  $v$ ,  $w$ . These are the corresponding displacements in the direction of  $x$ , in the direction of  $y$ , in the direction of  $z$ .

Now, if stress when wave is travelling through this infinitesimal element, what is happening? This is the stress – normal stress at one phase; the other phase on this side, the stress will be  $\sigma_{xx}$  plus there will be change of  $\sigma_{xx}$  over  $\Delta x$  into this distance  $dx$ . That is the increment in the change or stress condition of the other side of the element. Why it happens? Because wave travels through this media. So, there is an inertia acting on the system; that inertia will create the stress difference between two phases. Like we have seen in the one-dimensional wave propagation equation also, the similar thing we are observing over here. So, let us look at the derivation over here.

Let us not look at the slide. Here first we will derive; then, we will go the slide. So, now, we are considering the shear stresses. The shear stress in this direction is, this is  $\sigma_{xy}$ ; and, in this direction, we have  $\sigma_{xz}$ . Now, as I said, let us first concentrate only-direction of wave in the x-direction only. So, we will be interested to know all the stresses acting in x-direction. Now, this is not in x-direction; this is not in x-direction. So, we need not to consider these at this moment, because we are only considering the x-direction stresses. So, what are the x-direction stresses? We will see here; x-direction stresses will be on this phase. There will be this direction. This is  $\sigma_{xz}$ . It is in which plane? It is in  $z$  plane in x-direction. So, it will be  $z$   $x$ . If we can the same notation as we have mentioned,  $x$   $z$  also we can write. So, it is basically  $\sigma_{zx}$  in x-direction; so  $z$   $x$ . As we know,  $\sigma_{zx}$  equals to  $\sigma_{xz}$ . That is why we could write this as  $\sigma_{xz}$ . Remember that.

What will be the change over a travel of this distance of  $dy$ ? There will be on this phase in this direction – opposite direction; this should be what?  $\sigma_{xz}$  plus  $\Delta \sigma_{xz}$

changing over the distance of  $\Delta z$  into... It is changing over the length of  $y$ ; it should be  $y$ . It has changed through this distance of  $y$ . So, what it should be?  $\sigma_{xz}$  plus  $\Delta \sigma_{xz}$  that is varying with respect to the distance of  $y$ . So, this should be not  $\Delta z$ ; this will be  $\Delta y$  over the distance of  $dy$ , because see here it is  $\sigma_{xz}$ ; when it goes to on this phase, it moves by a distance of  $dy$ . So, its change will be over the length of this  $dy$ . So,  $\Delta y$  times  $dy$ .

Now, what will be another one in  $x$ -direction? In  $x$ -direction, if I want to find out one more stress, what that should be? On this plane, on this plane, I have taken... Then,  $x$  – another  $x$   $y$  should come from... I have  $\sigma_{xy}$  over here. So, other phase will be on this front phase and back phase. So, this back phase will have this direction; the back phase – back side in the  $x$ -direction. So, what it should be? That is on which plane? That is  $xy$  plane. So, that is  $\sigma_{xy}$ ; I can write  $\sigma_{yx}$ . It is essentially  $\sigma_{yx}$ , because  $\sigma_{yx}$  equals to  $\sigma_{xy}$ . That is the reason why I could write it as  $\sigma_{xy}$ . It is on the back side. So, when it travels through a distance of  $\Delta z$  on this front side, this phase, it will be this one. What it should be? It should be  $\sigma_{xy}$ , which is changing  $\Delta \sigma_{xy}$  over the distance of it travels in the  $\Delta z$ -direction. So, over  $\Delta z$  times  $\Delta z$ .

Now, if I consider the equilibrium of these forces; that is, on the outer phase or after they travel in the  $x$ ,  $y$ ,  $z$ -directions, the increase in the stresses whether it is normal stress or shear stress in  $x$ -direction. So, all  $x$ -direction forces now if we consider, what we will see?

(Refer Slide Time: 43:31)

$$\begin{aligned}
 & \text{In } x\text{-dir,} \\
 & \left( \sigma_{xx} + \frac{\partial \sigma_{xx}}{\partial x} \cdot dx \right) dy dz - \sigma_{xx} \cdot dy \cdot dz \\
 & + \left( \sigma_{xy} + \frac{\partial \sigma_{xy}}{\partial z} \cdot dz \right) dx dy - \sigma_{xy} \cdot dx \cdot dy \\
 & + \left( \sigma_{xz} + \frac{\partial \sigma_{xz}}{\partial y} \cdot dy \right) dx dz - \sigma_{xz} \cdot dx \cdot dz \\
 & = \rho \cdot dx dy dz \cdot \frac{\partial^2 u}{\partial t^2}
 \end{aligned}$$

Let us write it down further. We can write down that,  $\sigma_{xx}$  plus  $\frac{\partial \sigma_{xx}}{\partial x}$  by  $dx$  into  $dx \dots$  This is working on which plane? This stress is acting on the plane with cross-sectional area of how much? This is  $dz$ ; this is  $dy$ ; multiplied with  $dy dz$ . That is the force. That is the positive force in this direction. Opposite force in the other direction is  $\sigma_{xx}$  times same  $dy dz$ . So, minus  $\sigma_{xx}$  times  $dy dz$ . So, that is the resultant force in positive  $x$ -direction due to the normal stress difference.

Next, I have to write another one, which is  $\sigma_{xy}$  plus  $\frac{\partial \sigma_{xy}}{\partial z}$  by  $dz$  into  $dz$ . This shear stress let us look at here. This shear stress is acting on which plane? The cross-sectional area is  $dx$  times  $dy$ . Look at this plane – front plane. This shear stress is acting on this front plane. So, if I multiply that shear stress with respect to this area – cross-sectional area  $dx$  times this vertical  $dy$ , we will get the net force in the  $x$ -direction. So,  $dx dy$ . In the other direction, what is the force, which is acting? This dotted one in black color on the other side of the phase; on this side; on that side. So, that will be minus  $\sigma_{xy}$  times  $dx dy$ .

What else we have? Another force let us look at here. We have... In this, we have taken actually the direction will be different. In the positive direction, it is increasing. So, it should be the incremental portion; it should always in positive direction; and, this should be in this negative direction. So, that is the direction cosines. Also, we can check why it should be in this direction; because this and this should be in this format. And, this – the

other side should be in this, so that they are in equivalence. So, with this direction, what we can write now? This is the positive direction in x; this is the shear stress incremental part. That is acting on which plane? What is the cross-sectional area over here? That is dx times dz. So, sigma x z plus del sigma x z by del y times dy times dx dz minus... What is this one acting in the other direction? Sigma x z. So, sigma x z times dx dz.

Now, this net force, which we are getting – this should be equals to... It should be equals to the inertia in x-direction. What should be the inertia? Mass times the acceleration. Now, what is acceleration in x-direction? That is, if you differentiate displacement twice with respect to time, displacement in x-direction is u. We have already mentioned. So, del 2 u del t square is the acceleration. And, what is the mass? Mass is density into the volume. What is the volume of that element? dx, dy and dz. So, inertia has to balance that difference in stresses at two phases of the element in x-direction. So, this is the derivation for in x-direction only. So, what we can write? This, this gets cancelled; this, this gets cancelled; this, this gets cancelled.

(Refer Slide Time: 49:00)

The whiteboard shows the following derivation:

$$\frac{\partial \sigma_{xx}}{\partial x} (dx dy dz) + \frac{\partial \sigma_{xy}}{\partial z} (dx dy dz) + \frac{\partial \sigma_{xz}}{\partial y} (dx dy dz) = \rho (dx dy dz) \frac{\partial^2 u}{\partial t^2}$$

[∵ dx dy dz ≠ 0]

$$\therefore \frac{\partial \sigma_{xx}}{\partial x} + \frac{\partial \sigma_{xz}}{\partial y} + \frac{\partial \sigma_{xy}}{\partial z} = \rho \frac{\partial^2 u}{\partial t^2}$$

In x-dir.

So, on simplification of this equation, we can further write that, del sigma x x by del x into dx dy dz plus del sigma x y by del z into dx dy dz plus del sigma x z by del y into dx dy dz. That is equals to rho dx dy dz times del 2 u by del t square. So, this term I can cancel from both the sides, because this is the volume of the element, which is nonzero; has to be nonzero. Therefore, what we can write? del sigma x x by del x plus del sigma x



$\frac{\partial \sigma_{yy}}{\partial y} + \frac{\partial \sigma_{xy}}{\partial x} + \frac{\partial \sigma_{yz}}{\partial z} = \rho \frac{\partial^2 v}{\partial t^2}$ . So, this is our basic governing equation of motion for three-dimensional wave propagating in x-direction. So, this is in x-direction only. Similarly, if I want to find out what will be the equation in y and z-direction, in the similar format, if we want to derive, the form of the equation will be... Let me write it down for y and z-directions.

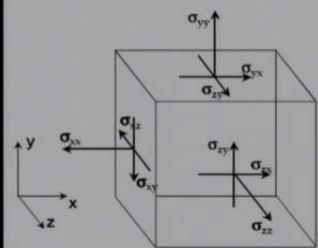
(Refer Slide Time: 51:01)

The image shows a whiteboard with two equations written in black ink. The first equation is under the heading "In y-dir." and is  $\frac{\partial \sigma_{yy}}{\partial y} + \frac{\partial \sigma_{xy}}{\partial x} + \frac{\partial \sigma_{yz}}{\partial z} = \rho \frac{\partial^2 v}{\partial t^2}$ . The second equation is under the heading "In z-dir" and is  $\frac{\partial \sigma_{zz}}{\partial z} + \frac{\partial \sigma_{xz}}{\partial x} + \frac{\partial \sigma_{yz}}{\partial y} = \rho \frac{\partial^2 w}{\partial t^2}$ . In the bottom left corner of the whiteboard, there is a small circular logo with a star and the text "NPTEL" below it.

In y-direction, it should be  $\frac{\partial \sigma_{yy}}{\partial y}$ , because this is the normal stress component plus  $\frac{\partial \sigma_{xy}}{\partial x}$  plus  $\frac{\partial \sigma_{yz}}{\partial z}$  should be equals to  $\rho \frac{\partial^2 v}{\partial t^2}$ . And, we will get here in z-direction, in the similar form,  $\frac{\partial \sigma_{zz}}{\partial z}$ , that is, the normal stress plus  $\frac{\partial \sigma_{xz}}{\partial x}$  plus  $\frac{\partial \sigma_{yz}}{\partial y}$  equals to  $\rho \frac{\partial^2 w}{\partial t^2}$ . So, these are three governing equation of motion, which we will get after solving this three-dimensional wave equation in the solid, which is shown over here.

(Refer Slide Time: 52:45)

**Three Dimensional Elastic Solids**



The diagram shows a 3D rectangular element in a Cartesian coordinate system (x, y, z). Stress components are labeled:  $\sigma_{xx}$ ,  $\sigma_{yy}$ ,  $\sigma_{zz}$  (normal stresses) and  $\sigma_{xy}$ ,  $\sigma_{yx}$ ,  $\sigma_{yz}$ ,  $\sigma_{zy}$ ,  $\sigma_{zx}$ ,  $\sigma_{xz}$  (shear stresses).

$$\rho \frac{\partial^2 u}{\partial t^2} = \frac{\partial \sigma_{xx}}{\partial x} + \frac{\partial \sigma_{xy}}{\partial y} + \frac{\partial \sigma_{xz}}{\partial z}$$

$$\rho \frac{\partial^2 v}{\partial t^2} = \frac{\partial \sigma_{yx}}{\partial x} + \frac{\partial \sigma_{yy}}{\partial y} + \frac{\partial \sigma_{yz}}{\partial z}$$

$$\rho \frac{\partial^2 w}{\partial t^2} = \frac{\partial \sigma_{zx}}{\partial x} + \frac{\partial \sigma_{zy}}{\partial y} + \frac{\partial \sigma_{zz}}{\partial z}$$

Reference : Kramer (1996)

NPTEL IIT Bombay, DC 18

You can see in this slide, three-dimensional elastic solid, which we have derived now. So, this is the final derived equation as I have mentioned just now.

(Refer Slide Time: 52:55)

**Three dimensional wave equations (contd.)**

**Strain notations**

$\epsilon_{xx} = \frac{du}{dx}$	$\epsilon_{xy} = \frac{dv}{dx} + \frac{du}{dy}$	$\Omega_x = \frac{1}{2} \left( \frac{dw}{dy} - \frac{dv}{dz} \right)$
$\epsilon_{yy} = \frac{dv}{dy}$	$\epsilon_{yz} = \frac{dw}{dy} + \frac{dv}{dz}$	$\Omega_y = \frac{1}{2} \left( \frac{du}{dz} - \frac{dw}{dx} \right)$
$\epsilon_{zz} = \frac{dw}{dz}$	$\epsilon_{zx} = \frac{du}{dz} + \frac{dw}{dx}$	$\Omega_z = \frac{1}{2} \left( \frac{dv}{dx} - \frac{du}{dy} \right)$

Reference : Kramer (1996)

NPTEL IIT Bombay, DC 19

Now, using the strain notations... The strain notations – what we can use? The normal strain in x-direction, in y-direction, in z-direction – those are nothing but epsilon x x is del u by del x; epsilon y y is del v by del y; epsilon z z is del w by del z. These are normal strain. What are the shear strain? These are the shear strain. Epsilon x y – so, del v by del x plus del u by del y; epsilon y z – del w by del y plus del v by del z; epsilon z x

– del u by del z plus del w by del x. And, what are the torsional strains? Torsional strain in x, y and z-direction can be represented by these equations.

(Refer Slide Time: 54:00)

**Three dimensional wave equations (contd.)**

**Stress-strain relationships**

$$\begin{aligned} \sigma_{xx} &= c_{11}\epsilon_{xx} + c_{12}\epsilon_{yy} + c_{13}\epsilon_{zz} + c_{14}\epsilon_{xy} + c_{15}\epsilon_{yz} + c_{16}\epsilon_{zx} \\ \sigma_{yy} &= c_{21}\epsilon_{xx} + c_{22}\epsilon_{yy} + c_{23}\epsilon_{zz} + c_{24}\epsilon_{xy} + c_{25}\epsilon_{yz} + c_{26}\epsilon_{zx} \\ \sigma_{zz} &= c_{31}\epsilon_{xx} + c_{32}\epsilon_{yy} + c_{33}\epsilon_{zz} + c_{34}\epsilon_{xy} + c_{35}\epsilon_{yz} + c_{36}\epsilon_{zx} \\ \sigma_{xy} &= c_{41}\epsilon_{xx} + c_{42}\epsilon_{yy} + c_{43}\epsilon_{zz} + c_{44}\epsilon_{xy} + c_{45}\epsilon_{yz} + c_{46}\epsilon_{zx} \\ \sigma_{yz} &= c_{51}\epsilon_{xx} + c_{52}\epsilon_{yy} + c_{53}\epsilon_{zz} + c_{54}\epsilon_{xy} + c_{55}\epsilon_{yz} + c_{56}\epsilon_{zx} \\ \sigma_{zx} &= c_{61}\epsilon_{xx} + c_{62}\epsilon_{yy} + c_{63}\epsilon_{zz} + c_{64}\epsilon_{xy} + c_{65}\epsilon_{yz} + c_{66}\epsilon_{zx} \end{aligned}$$


Reference : Kramer (1996)

NPTEL IIT Bombay, DC 21

Now, using the stress strain relationship... How we can use the stress strain relationship for this three-dimensional wave equation? This normal stress in x-direction – sigma x x can be represented through strain through some modulus. So, c 1 1 epsilon x x plus some modulus c 1 2 epsilon y y plus c 1 3 epsilon z z plus c 1 4 epsilon x y – this modulus is related to the shear, because it is relating to the shear strain. These are relating to the normal strain. So basically, what you can see, other two shear strains – all these modulus coefficients, that is, stress versus strain can be easily written in the form of a matrix; that is what it shows. So, what is that form of the matrix?

(Refer Slide Time: 55:11)

$$\begin{Bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{zz} \\ \sigma_{xy} \\ \sigma_{yz} \\ \sigma_{zx} \end{Bmatrix} = \begin{bmatrix} C_{11} & C_{12} & C_{13} & C_{14} & C_{15} & C_{16} \\ C_{21} & C_{22} & C_{23} & C_{24} & C_{25} & C_{26} \\ \vdots & \vdots & \vdots & \vdots & \vdots & \vdots \\ C_{61} & C_{62} & \dots & \dots & \dots & C_{66} \end{bmatrix} \begin{Bmatrix} \epsilon_{xx} \\ \epsilon_{yy} \\ \epsilon_{zz} \\ \epsilon_{xy} \\ \epsilon_{yz} \\ \epsilon_{zx} \end{Bmatrix}$$



In the simplest form, if we want to rewrite it, the stress strain relationship is something like this;  $\sigma_{xx}$ ,  $\sigma_{yy}$ ,  $\sigma_{zz}$ ,  $\sigma_{xy}$ ,  $\sigma_{yz}$ ,  $\sigma_{zx}$ . These are the six stresses: three normal stresses and three shear stresses. They are related through some modulus, which is nothing but a modulus matrix through the strain. What are those strain?  $\epsilon_{xx}$ ,  $\epsilon_{yy}$ ,  $\epsilon_{zz}$ ,  $\epsilon_{xy}$ ,  $\epsilon_{yz}$ ,  $\epsilon_{zx}$ ; so these strains are first three normal strains; and these three are shear strains. They are related through this matrix, what we were showing over here -  $C_{11}$   $C_{12}$   $C_{13}$   $C_{14}$   $C_{15}$   $C_{16}$ ;  $C_{21}$   $C_{22}$   $C_{23}$   $C_{24}$   $C_{25}$   $C_{26}$  so on. This one will be  $C_{61}$ ; this one will be  $C_{62}$ . Similarly, goes over here up to  $C_{66}$ ; like that. So, this matrix is nothing but modulus matrix. So, this stress-strain relationship can be expressed through this modulus matrix. With this, we have come to the end of today's lecture. We will continue further in the next lecture.