

Soil Mechanics
Prof. B.V.S. Viswanathan
Department of Civil Engineering
Indian Institute of Technology, Bombay
Lecture – 44
Shear Strength of Soils
Lecture No.2

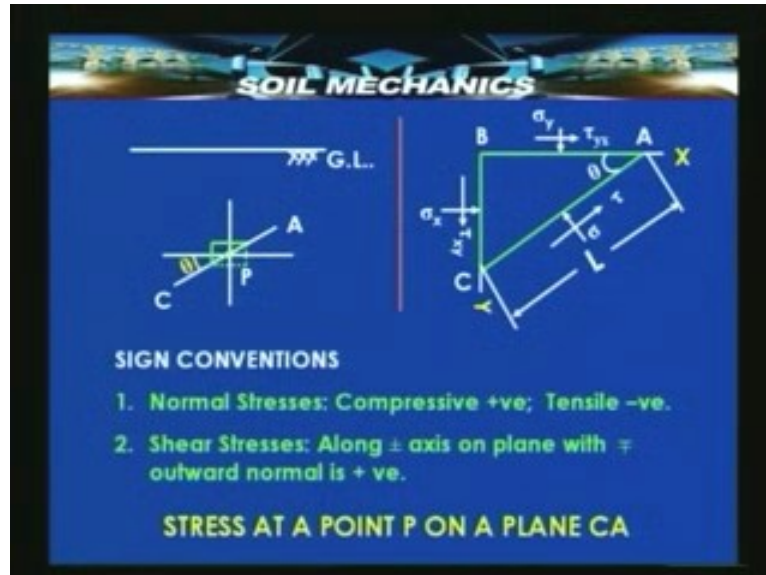
Dear students today we shall go through yet another lecture on shear strength of soil. This is the second lectures in a series of 5 or 6 lectures which I have planned to give. If you remember in the first lecture I gave you an idea about what shear strength is all about, why we need to study that. As usual let us first go through a quick review of what we already know before we go on to learn something more. So let me start with the first slide of today's lecture which is actually a review of what we had seen last time.

Take this slide for example. In the last lecture we had a brief over view of several aspects such as what's the important or significant of shear strength. In order to understand what shear strength is, what shear stresses are, we need to calculate the stress at any point on any plane. This aspect was emphasized in the last lecture and in order to do this computation of stresses at any point on any plane we also briefly looked into a concept known as the concept of Mohr's circle. We shall see a little more of that today. Then I also gave you an idea as to what kind of theories are available in order to see whether a soil will remain stable, will fail under the action of shear or not.

In order to know what is this shear strength of a soil, we need to have methods to measure them not only in labs but also if also directly in the field. So we also had a look at what kind of methods are available for this purpose. This is was what we have covered in the last lecture; a little more detail of what we saw in the last lecture is given in the next few slides. So what's the significant of shear strength for example? It is necessary primarily for the design of geotechnical structure against shear failure. What are the geotechnical structures? There can be many examples for example foundations, excavation.

Even an excavation is rightly called as a geotechnical structure because it is made in earth. Embankments, slopes, retaining walls and any number of earth structures they all come under the category of geotechnical structures and they are likely to fail if the shear stress along any plane at any point exceeds what the strength of the soil against shear is. That's the significance of learning shear strength. In order to know how to compute shear stress at any point so that we will know whether that strength is below or lower than the strength of the soil against shear. We need to have the method to compute the stresses at a point on any plane. So in order to check for shear failure it is necessary to have a method to determine the shear along any plane any point in a soil mass. For example suppose I take a point P in an infinite soil mass, there can be any number of planes passing through that point P, actually infinite number of planes. Take any plane CA and an element at the point P so called parallelepiped in two dimensions it becomes just a rectangle. Let the angle that CA makes with the horizontal B theta. Now this element can be exaggerated for visualization and represented as shown here in this key diagram.

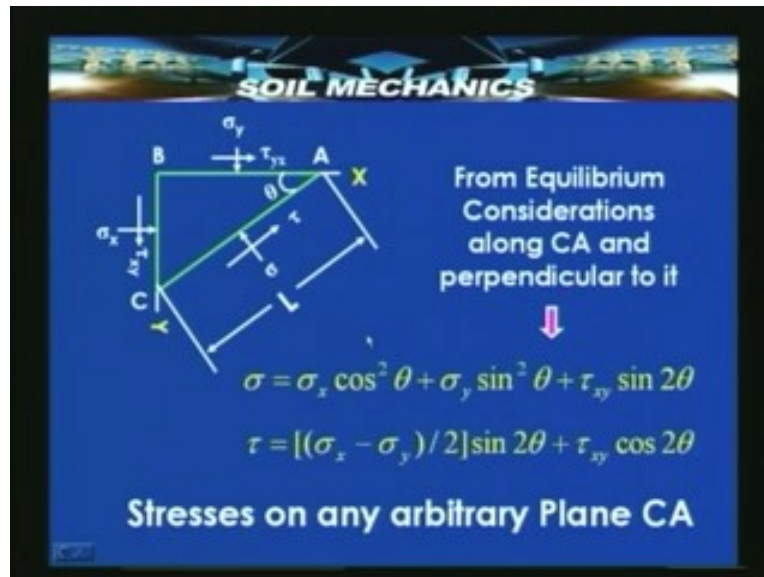
(Refer Slide Time: 04:21 min)



You see here this small element is expressed expanded in a set of 3 planes, actually all the 3 planes pass through this point P but for convenience we take them like this and in the limit when the dimensions of these planes reduced to or tend to zero the elements itself tend to be a point here. Now take this 3 planes AB, BC, CA in general on any plane there will be stresses which can always resolved into normal and shear components. So the normal component on this plane is σ_y , shear stress is τ_{yx} , on the other plane σ_x and τ_{xy} . Now the notation used here follows a certain sign conversions. For normal stresses compressive positive, tensile negative is chosen. So here a compressive normal stress σ_y acts on the plane AB, the normal to the plane AB is in the direction y and according to the chosen coordinate axis and so the stress is called σ_y so also σ_x . The τ_{xy} or τ_{yx} they represents shear stresses according to the sign convention along the positive axis on a plane with negative outward normal, the shear stress is positive.

For example take plane AB, the positive axis is in this directions and it's on a plane with a negative outward normal, the negative outward normal of plane AB is the negative y direction and therefore a positive shear stress acts in the positive direction of this plane, that's why τ_{yx} in the direction x is taken as the positive shear stress so also τ_{xy} . Now with this we will be in a position to calculate the stress on any arbitrary plane CA provided we know the stresses on AB and BC. So stresses on AB and BC are supposed to be known and usually they are known because for example take the plane AB that is horizontal. The stress there is essentially due to the weight of the column soil above it and the stress on the vertical plane is a fraction of that depending upon what's known as the coefficient of lateral earth pressure about which you must have definitely studying in some other lecture.

(Refer Slide Time: 07:11 min)

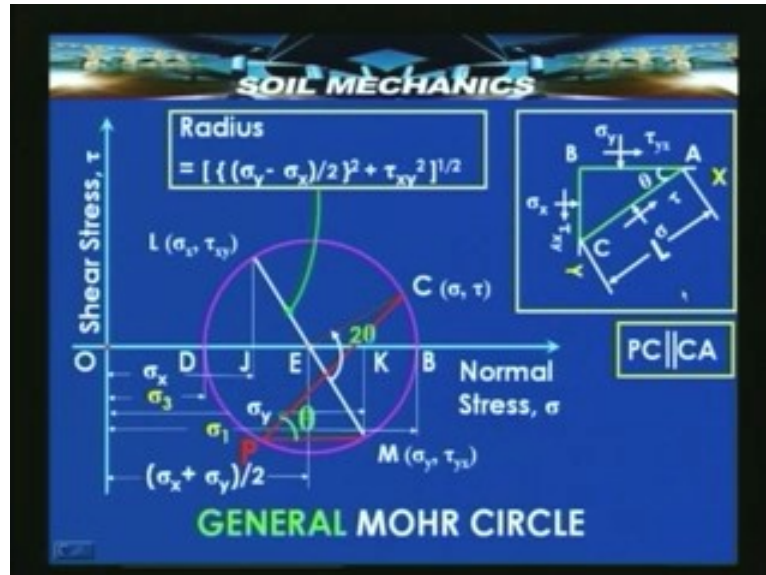


If we have this scheme of stresses on a set of planes, in terms of the stresses on AB and BC we can express the stress on CA from a very simple consideration that this element and therefore that point P must be in equilibrium. Now suppose the length of AC is capital L, correspondingly BC will be $L \sin \theta$, AB will be $L \cos \theta$ and therefore we will be in a position to calculate the total forces on each one of these planes and find out what's the condition required for satisfying equilibrium of this forces. Force on AB for example can be calculated as σ_y into $L \cos \theta$, force on BC can be calculated for example as τ_{xy} into $L \sin \theta$. The force on AC will be σ into L and τ into L .

In all for equilibrium to be maintained on this plane either parallel to it or perpendicular to it, we can show by resolving the forces that this stress σ must satisfy the condition that σ is equal to this and stress τ must be equal to this. This is what we discussed in some detail in the earlier lecture, so for purposes of this lecture, today's lecture we will take for granted that by resolving and satisfying equilibrium, we will be able to get these two stress values. So if all the terms on the right hand side are known and therefore it's possible to calculate the unknown stresses σ and τ , particularly τ which is what we are interested in because that's the shear stress on any plane on CA.

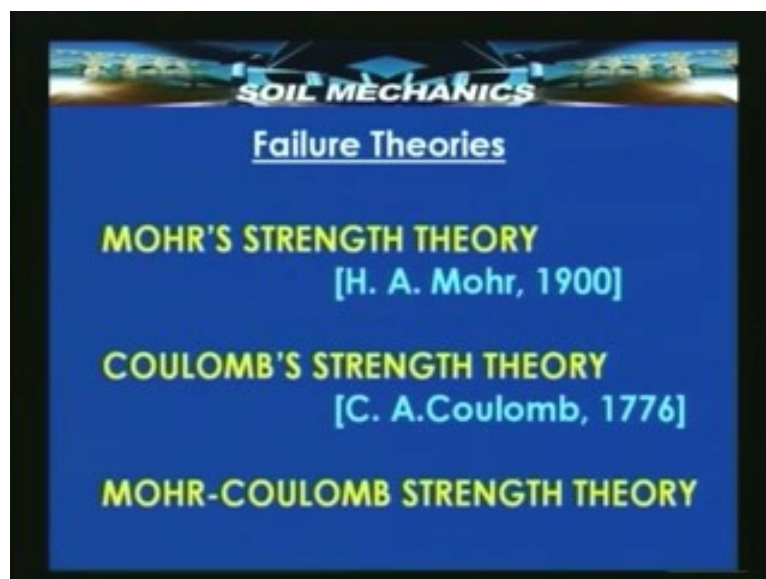
If the shear stress exceeds the strength along this plane then shear failure will take place. Now in order to compute this stresses, there is also a geometrical method available which is based on a concept known as the Mohr circle concept. This is a circle which is devised so ingeniously devised that if you plot the known stresses and follow certain geometrical procedures, you can calculate the unknown stress σ and τ on any arbitrary inclined plane CA. Let us take a look this key diagram once again which we are already familiar with AB BC CA and the stresses are σ_x σ_y τ_{xy} σ and L . Now let us see the geometrical procedure which is used for computing σ and τ on the plane CA. First we choose a set of axis, one axis for all the normal stresses another axis for all the shear stresses. We plot the normal stress for σ_x here as shown point J, σ_y that is point K.

(Refer Slide Time: 08:55 min)



Then we draw so called Mohr's circle whose center will be at the midpoint between σ_x and σ_y and whose radius will be given by $\sigma_y - \sigma_x$ by 2 whole square plus τ_{xy} by 2 whole square. Now with this radius if you draw a circle then this height obviously will be given by τ_{xy} and a point will have a coordinate's σ_x and τ_{xy} , correspondingly a point M will have σ_y and $-\tau_{xy}$. And the center will be having a coordinates $\sigma_x + \sigma_y$ by 2. In addition it can be shown that point B will have the maximum possible normal stress which is called as the major principle stress σ_1 and point D will represent the minimum possible normal stress that is the minimum principle stress σ_3 . And any point C as shown here having stresses σ and τ will represent the stresses on the plane CA.

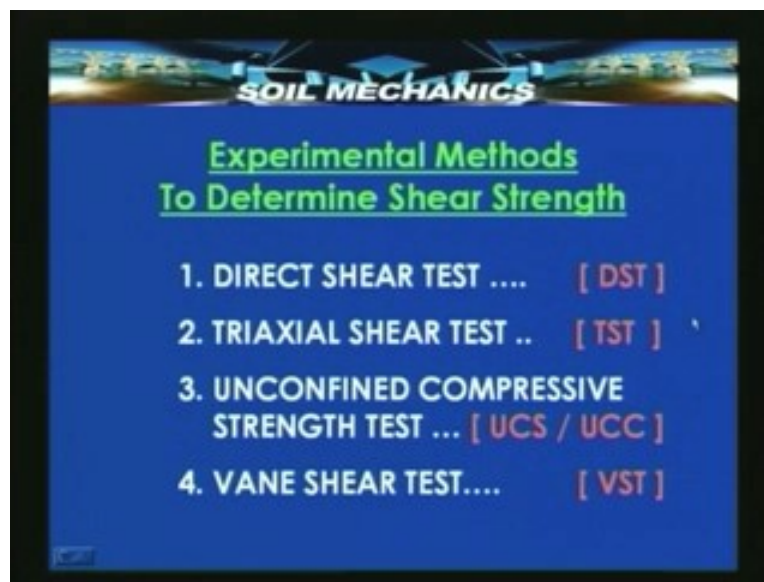
(Refer slide time 11:44 min)



Since CA is at an angle θ with respect to the horizontal, on this drawing if we plot a plane EC at angle 2θ with respect to the line joining L and M whose coordinates are σ_x to τ_{xy} and σ_y to τ_{yx} . Then we get the point C whose coordinates are σ_θ . There is also another method of obtaining a value of C through drawing lines MP and PC which we shall see in a subsequent slide. So this is what is known as the general Mohr circle which helps us to compute the normal and shear stress on any arbitrary plane provided we know stresses on any two perpendicular planes passing through the same point and the angle θ .

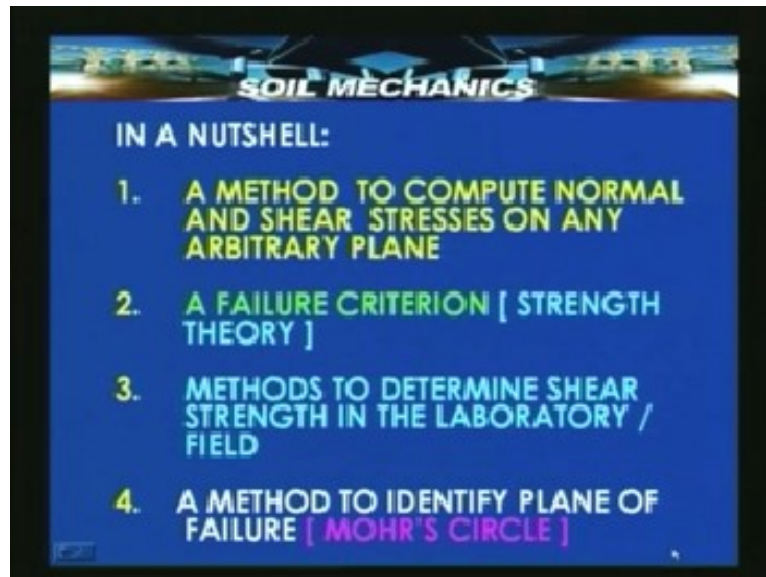
Now having a method to compute C, next step is to know whether failure will take place along particular plane, for this we have several failure theories. An idea about these failure theories had given in my earlier lecture. In today's lecture we shall try to see a little more about these theories. For all practical applications, suffice it to say that a linear relationship between stress and strength is sufficient to determine whether failure takes place or not. This is what is known as the Mohr coulomb failure plane, on the Mohr circle this can be expressed as a line having an equation s is equal to c plus $\sigma \tan \phi$ as we shall be seeing in little later.

(Refer Slide Time: 12:30 min)



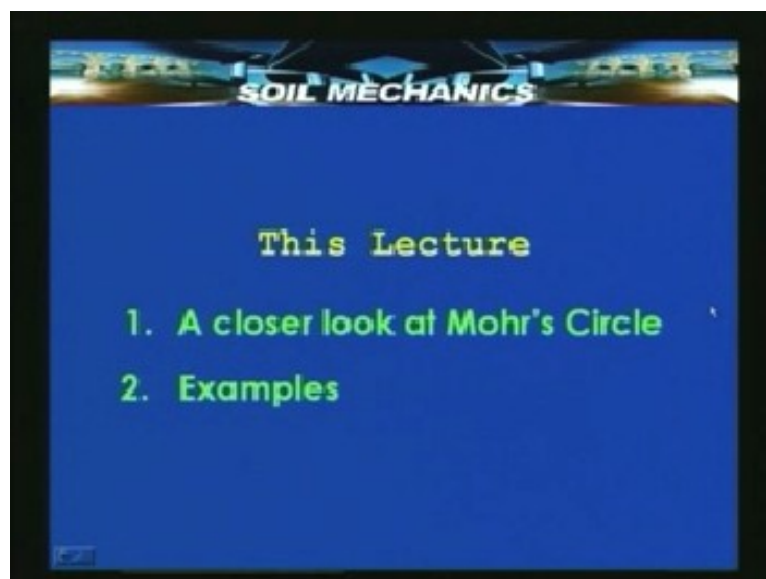
There are many methods to determine this shear strength, direct shear stress is one of them. This is a typically laboratory test although there are also large scale direct shear test meant for field application. Then we have triaxial shear test which is an improved variation of this shear stress called the direct shear test. Then we have certain quick test which can be conducted both in the laboratory and in the field, a little less rigorous than the first two test and they are also very useful and they are known as the unconfined compressive strength test UCS or UCC and the vane shear test VST. In a nutshell what we have seen so far is that there is a method or there needs to be developed a method to compute the normal and shear stresses on any arbitrary plane and it's also necessary to have a failure criterion or a strength theory to know whether failure will take place along any arbitrary plane or not. And then we need to know what the strength is in order to access whether failure will take place and for that we have methods.

(Refer Slide Time: 13:12 min)

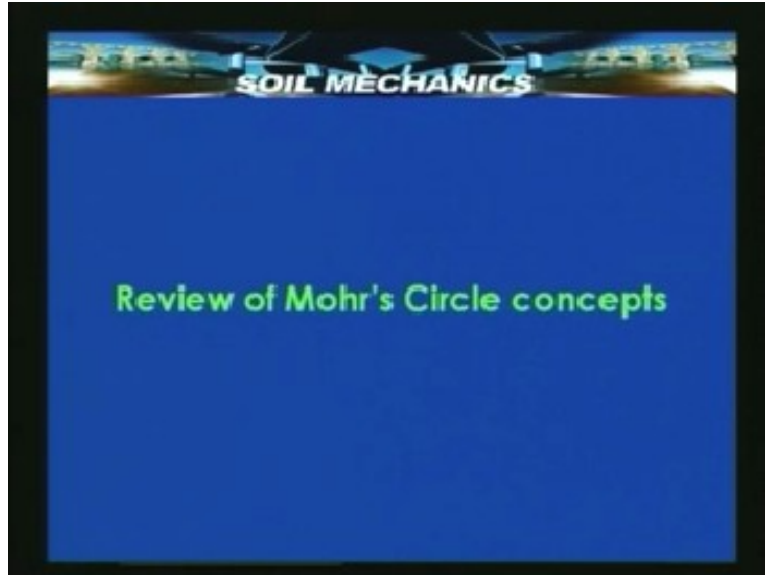


And then in order to compute the stress finally we have a nice method geometry based method known as the Mohr circle. Incidentally the Mohr circle itself is based on certain equations which we had developed rigorously and therefore either we can use the geometrical technique or directly the equations. We will see a few examples a little later and there both the concepts will be used to show that certain quantities which are associated with shear strength determination can be computed either from Mohr circle or by direct equations. In today's lecture what we shall do is to take a closer look at this Mohr's circle how it is constructed, different variation of this Mohr's circle for different conditions and also a couple of examples to see how Mohr's circle can be applied to compute certain parameters which are associated with shear failure.

(Refer Slide Time: 14:21 min)

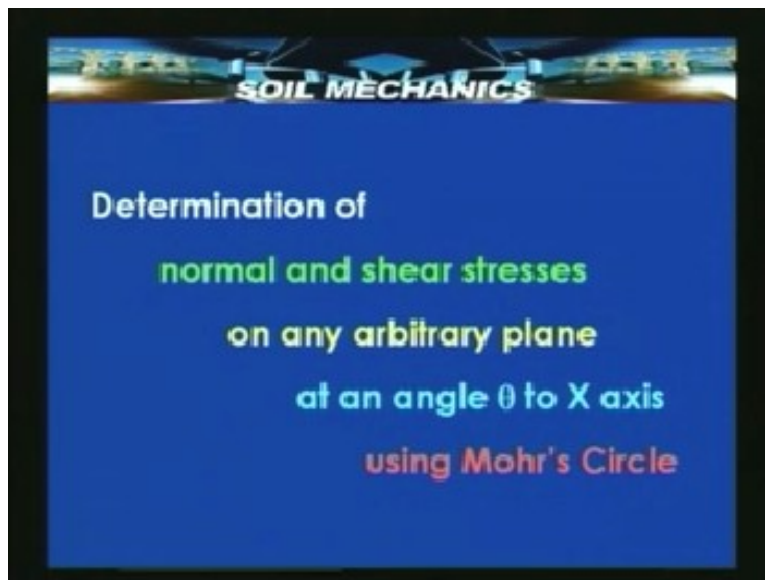


(Refer Slide Time: 14:15 min)



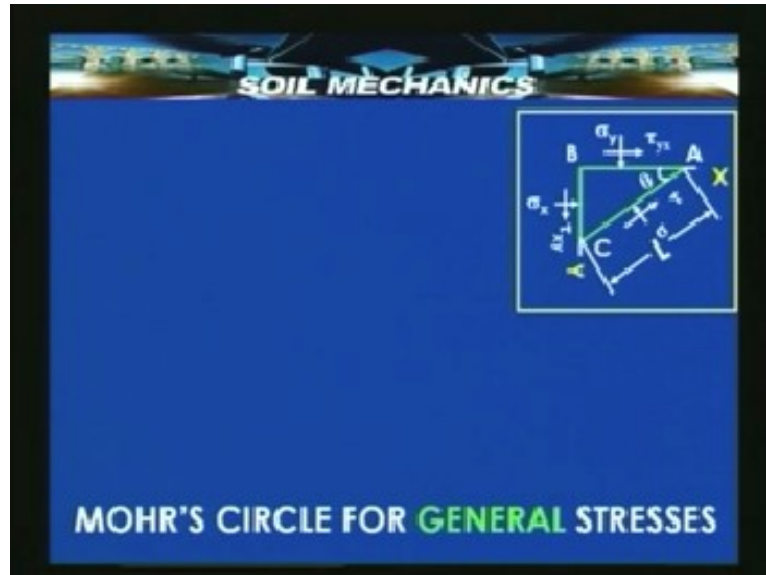
So first a review of the Mohr's circle concepts. What's Mohr's circle meant for? It's meant for determination of normal and shear stresses on any arbitrary plane at an angle θ to x axis and it makes use of the geometrical concept of the circle.

(Refer Slide Time: 14:52 min)



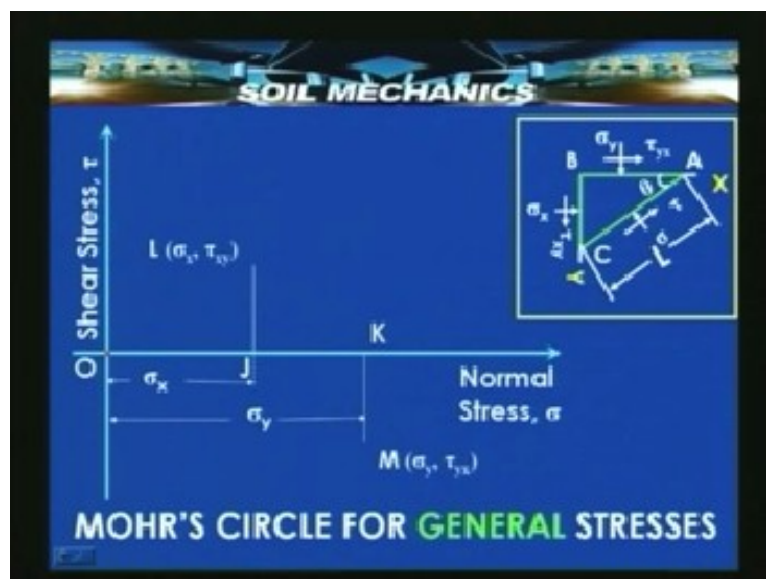
Let us see how a Mohr circle is constructed for a general set of stresses. What is meant by a general scheme or general system of stresses? Take a look at this familiar key diagram which is on the right. This shows as we have already seen 3 planes passing through a point but slightly exaggerated to indicate an element in equilibrium.

(Refer Slide Time: 15:11 min)



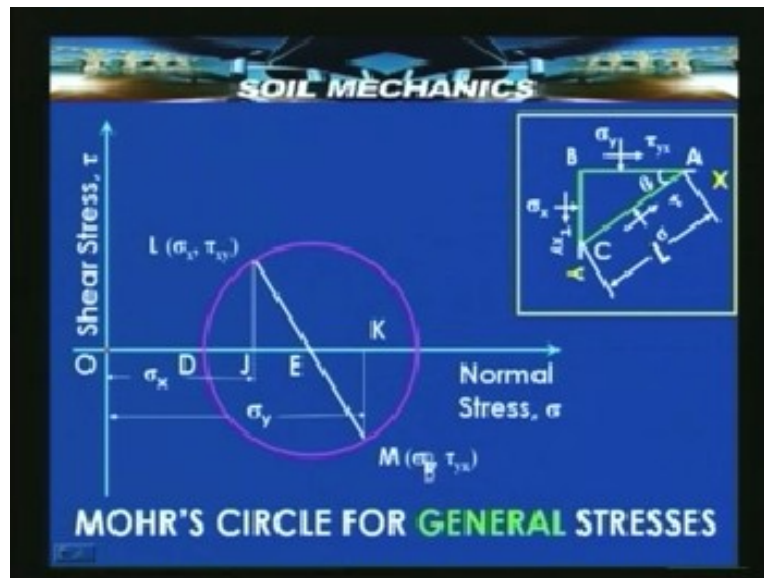
These set of stresses shown here is what I am referring to as the general set of stresses or general scheme of stresses. In the general case we have a normal and shear stress on each one of these planes. Now how do we construct a Mohr's circle and what do we construct it for? We are constructing the Mohr circle to determine sigma and tau on a plane AC in terms of sigma x sigma y tau x y and the angle theta. So how does this go? Step by step let us see. First step is to choose an x axis for normal stresses, all these normal stresses sigma sigma_y sigma_x they will be plotted along the x axis. Then we choose an origin O and a y axis meant for shear stresses. Having chosen this set of axis, now we choose the center of the Mohr's circle for that we start with plotting the known stresses.

(Refer Slide Time: 16:58 min)



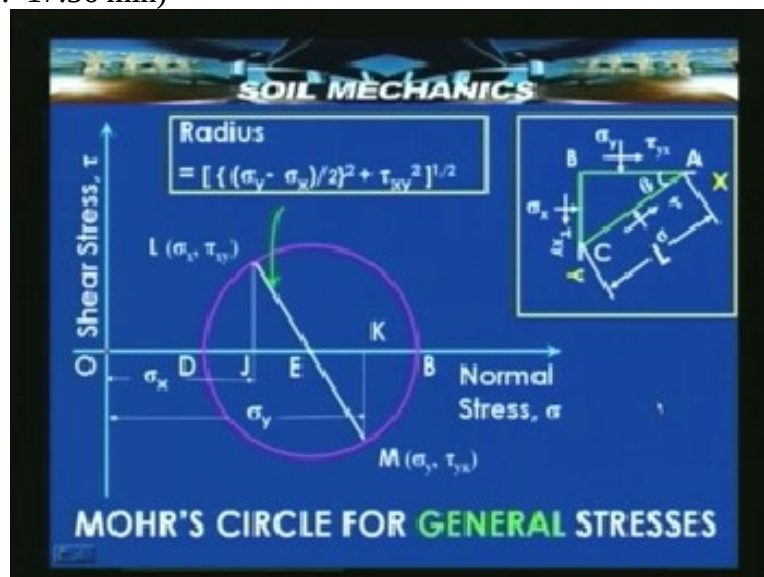
Take σ_x for example, find out the point J whose x coordinate will be σ_x , σ_x is a known normal stress and at J plot the shear stress τ_{xy} which is also known. Then you get the point L, similarly plot σ_y and get the point K and the point M whose coordinates will be σ_y to τ_{yx} . So now we have 2 extremities L and K of a typical diameter of the Mohr's circle LK. Now that intersects the x axis at point E and obviously E is the centre of the circle which will pass through both L and K. So now construct that circle.

(Refer Slide Time: 17:17 min)



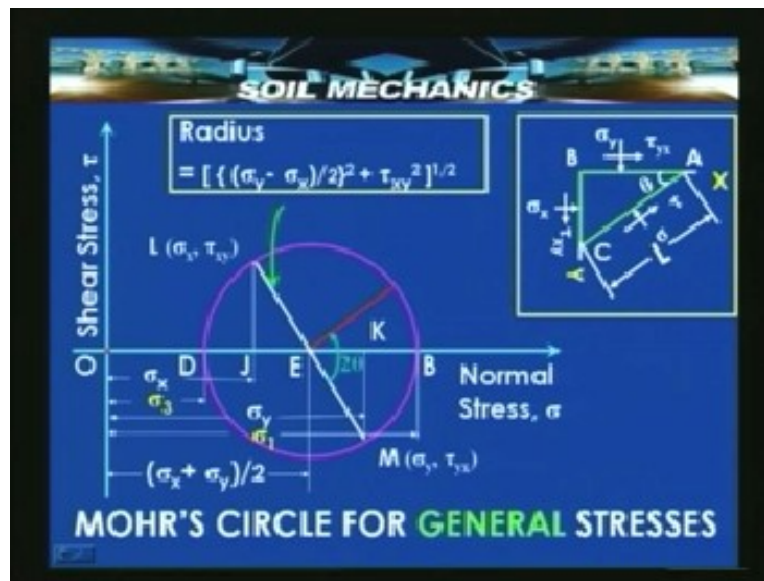
This is a circle, this circle intersects the x axis at point D and E, they have a significant as I briefly mentioned in 2, 3 slides ago. Point D has a least possible normal stress here and that's known as σ_3 .

(Refer Slide Time: 17:36 min)



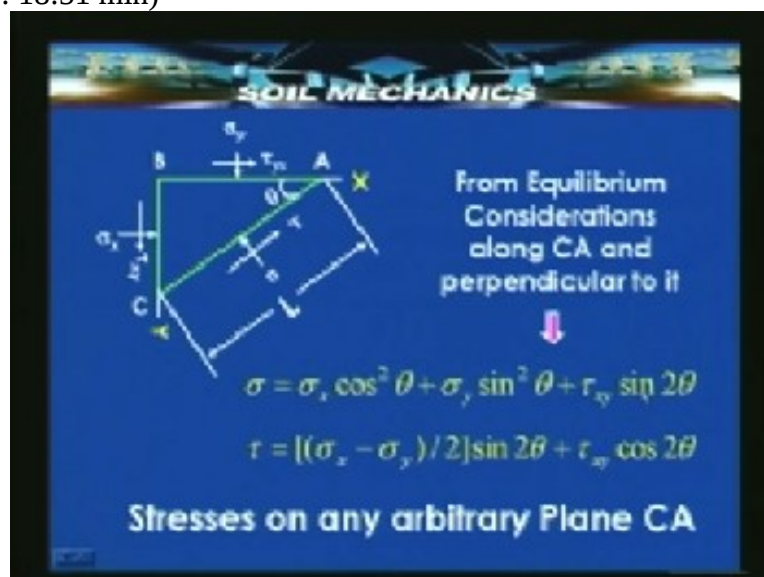
Now see here the radius of the Mohr's circle which we plotted with E as a centre and EL as the radius. The magnitude of the radius is $\sigma_y - \sigma_x$ by 2 whole square plus τ_{xy} whole square raise to the power half. The point D will have a coordinate σ_3 which can be either calculated or measured from the Mohr's circle. Similarly the point B will have the coordinate σ_1 which can be measured or computed and this completes the Mohr's circle for a general stress of stress.

(Refer Slide Time: 18:18 min)



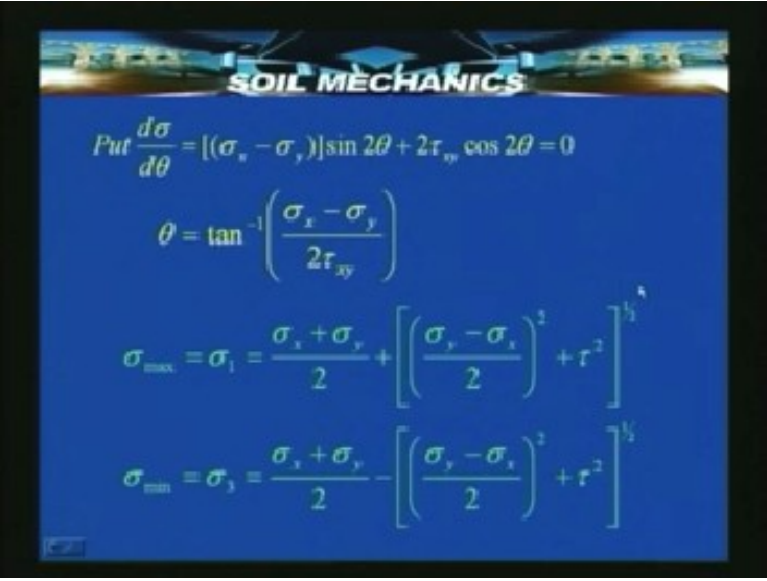
Now if we want the stress on the plane like CA, all that we need to do is to know the angle theta and then from this reference diameter draw a line at angle 2 theta taken in the anti-clockwise direction. And this red line typically represents the plane which we will get corresponding to the point C whose coordinates will be sigma and tow. So point C represents the stress level on the plane AC.

(Refer Slide Time: 18:51 min)



Now it's also possible to compute these stresses by these equations as we have seen earlier. If we compute them from the Mohr's circle or measure them off from the Mohr's circle. As the coordinate of the point C, the horizontal coordinate and vertical coordinate we will get the same values of C sigma and tau as from the numerical computation. So these expressions will be obtained also from the Mohr's circle and these are what are the stresses which are unknown on the plane AC.

(Refer Slide Time: 19:29 min)



SOIL MECHANICS

$$\text{Put } \frac{d\sigma}{d\theta} = [(\sigma_x - \sigma_y)] \sin 2\theta + 2\tau_{xy} \cos 2\theta = 0$$

$$\theta = \tan^{-1} \left(\frac{\sigma_x - \sigma_y}{2\tau_{xy}} \right)$$

$$\sigma_{\max} = \sigma_1 = \frac{\sigma_x + \sigma_y}{2} + \left[\left(\frac{\sigma_x - \sigma_y}{2} \right)^2 + \tau^2 \right]^{1/2}$$

$$\sigma_{\min} = \sigma_3 = \frac{\sigma_x + \sigma_y}{2} - \left[\left(\frac{\sigma_x - \sigma_y}{2} \right)^2 + \tau^2 \right]^{1/2}$$

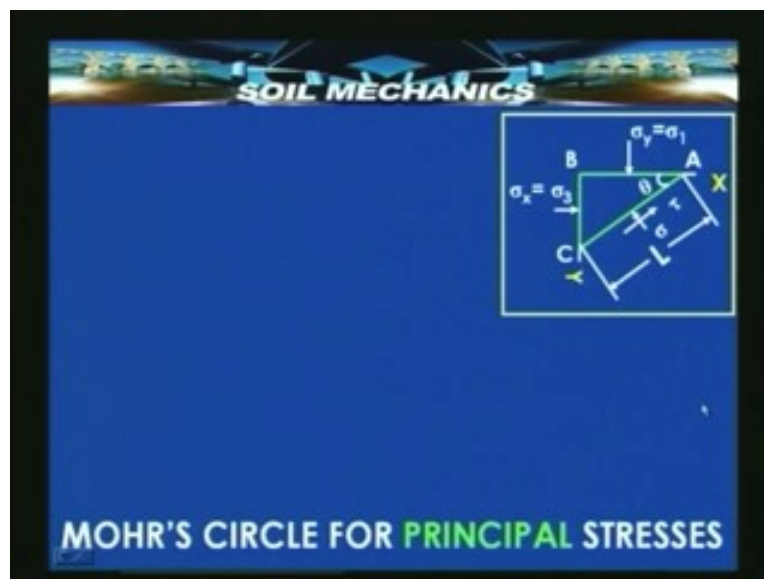
In addition now as we have seen, there are 2 points D and B which have the minimum and maximum stresses. How to compute these stresses? We can of course measure them off the Mohr which is plotted to scale but we can also compute that. For computing this we use the simple notion that sigma₃ is the minimum possible stress. Sigma₁ is the maximum possible stress and we already know what sigma is we have an expression for sigma. This is the expression for sigma and if sigma has to be minimum or maximum it's simply means mathematically that d sigma by d theta must be zero. So take d sigma by d theta, put it equal to zero and this is the condition that you get.

Now under what conditions this expression becomes zero. This expression becomes zero as you can verify easily when theta is equal to tan inverse of sigma_x- sigma_y by 2 tau_{xy}. When theta is equal to this, 2 theta will be equal to tan inverse of sigma_x- sigma_y by tau_{xy} and cos 2 theta can be correspondingly computed, sin 2 theta can be correspondingly computed and then you substitute in this equation you will find that this equation is satisfied. If that is the case then for this value of theta, corresponding to this value of theta what will be the maximum value of sigma and then minimum value of sigma which will correspond to d sigma by d theta equal to zero. Both of them can be computed and it can be shown that sigma maximum is sigma₁ equal to this, sigma minimum is this equal to all this prime quantities which is on the right within the brackets. Now how do we get 2 equations corresponding to theta? What happens is when tan theta is equal to this, it's possible to get this expression satisfied, both when sine 2 theta and cos 2 theta are positive and also when sine 2 theta and cos 2 theta are both negative.

When $\sin 2\theta$ and $\cos 2\theta$ are both positive this equation is satisfied and the corresponding σ value we get from this expression will be equal to σ_1 . On the other hand when $\sin 2\theta$ and $\cos 2\theta$ are both negative and this equation is satisfied then this becomes zero and the corresponding σ becomes the minimum value σ_3 which is given by this expression. So it's possible to calculate the minimum and maximum by a simple differentiation and substitution of condition under which the stress becomes minimum or maximum.

Now let us see once again the construction of a Mohr's circle but this time for a specific case where the principle stresses are known or rather than the principle planes are known. Take the key diagram once again. This key diagram is similar to the earlier one but the only difference is here the horizontal plane AB only has a normal stress. The shear stress is zero on this because AB is a principle plane.

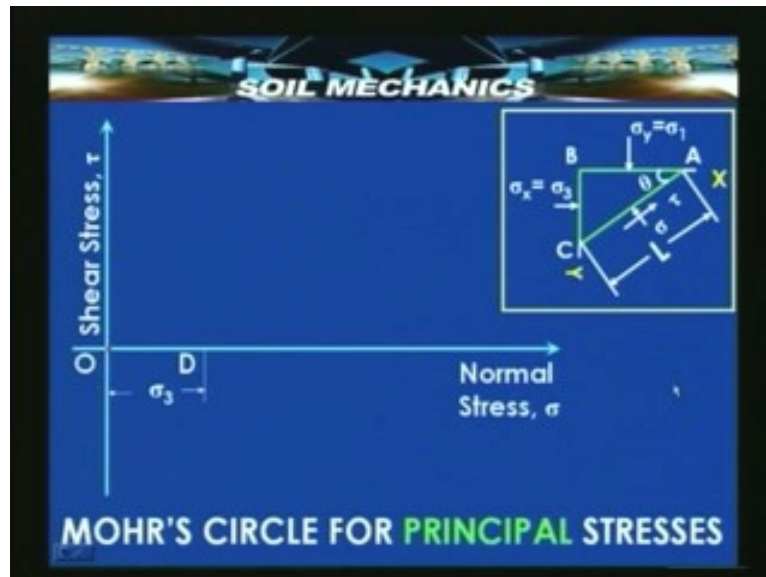
(Refer Slide Time: 22:15 min)



Let us take σ_y to be greater than σ_x and then σ_y becomes the major principle stress. So this is equal to σ_1 . On the other hand BC automatically becomes σ_x or the minor principle stress acting horizontally on the plane BC and correspondingly if you take a plane CA at an angle θ with respect to horizontal or the major principle stress. Then the normal and shear stresses on the plane AC σ and τ can now be computed. In terms of σ_1 and σ_3 either from Mohr's circle or from rigorous expressions.

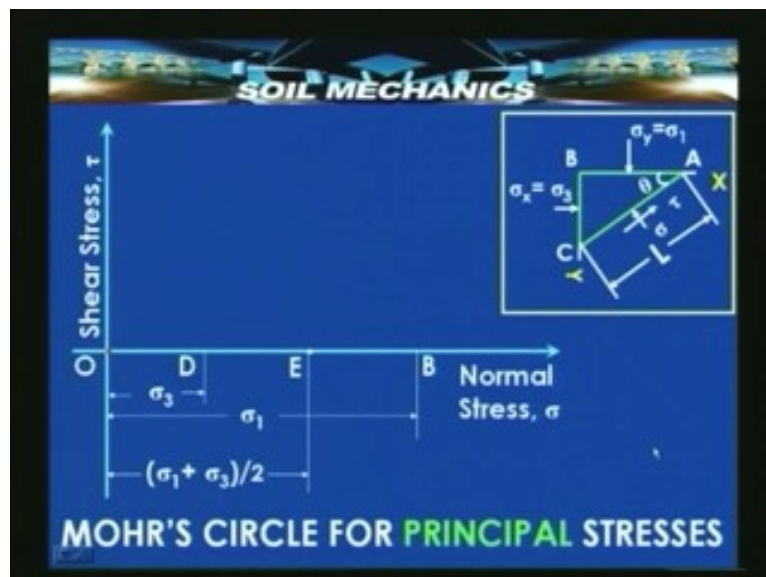
Let us see the construction of a Mohr's circle when the principle stresses are known. As before take the x axis that is normal stress axis, take the origin O then plot the y axis for shear stresses at right angle to the normal stress axis. Choose the point D, earlier D was not known. But now we know D because the value of σ_3 the minor principle stress is known. Then choose the major principle stress as the x axis and get the point B.

(Refer Slide Time: 23:49 min)

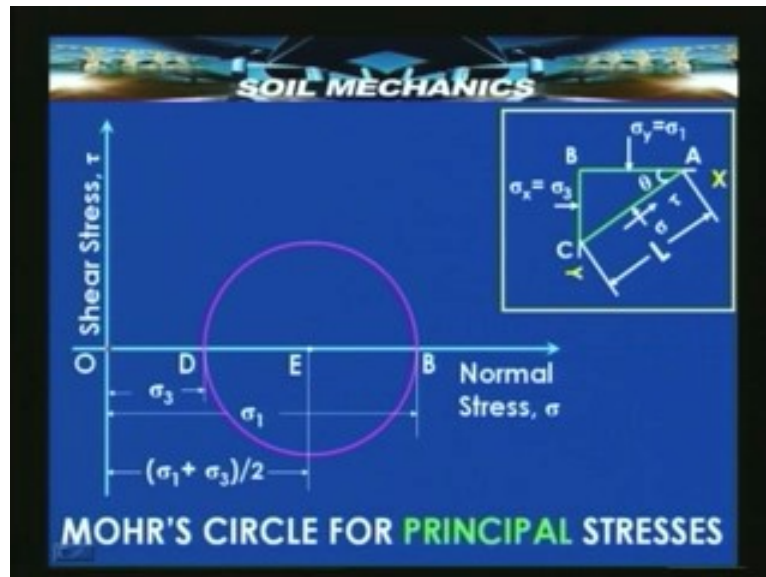


Obviously the center has to lie between D and B at its midpoint. So choose the origin E at the midpoint of the length DB. It will be equal to $\sigma_1 + \sigma_3$ by 2. Plot the Mohr's circle with the radius as $\sigma_1 - \sigma_3$ by 2 then take an angle 2θ with respect to the horizontal that is the line EB.

(Refer Slide Time: 24:11 min)

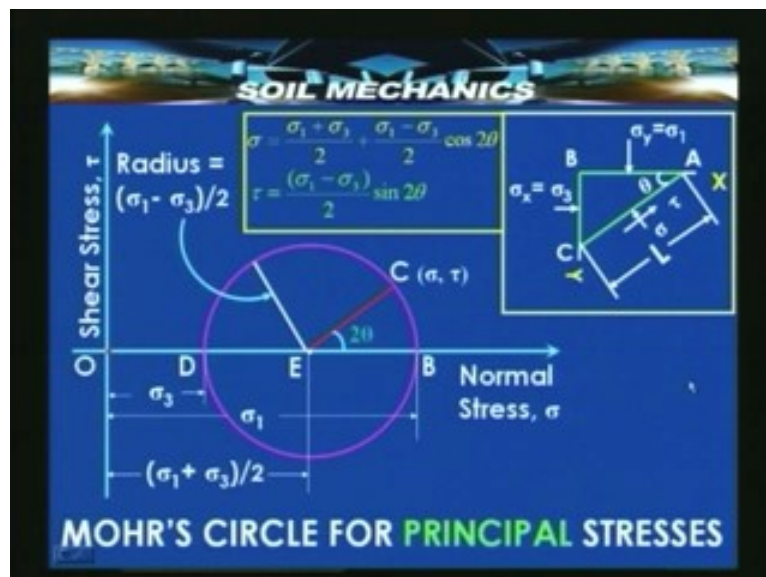


(Refer Slide Time: 24:16 min)



Get the red line, the coordinates of the points where the points intersect the Mohr's circle will be nothing but sigma and tau. So points C will represent the stress state on any arbitrary plane AC at angle theta and it can be shown that sigma will be equal to this, tau will be equal to $\sigma_1 - \sigma_3 \sin 2\theta$.

(Refer Slide Time: 24:42 min)

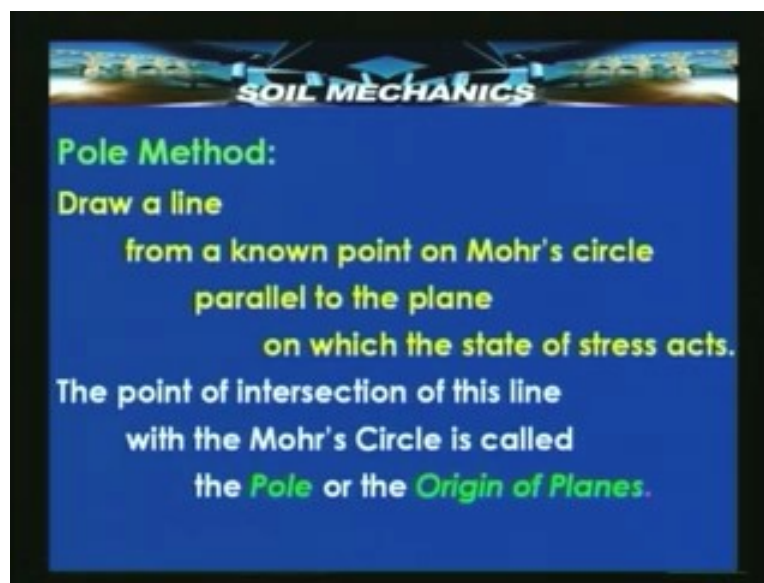


So once we know these 2 expressions we can compute sigma and tau from knowing the angle theta. So looking at this slide we can see that there is a minor difference between general set of stresses and principal set of stresses.

The difference is that when there are principle stresses acting on the known planes there are no shear stresses and to that extent the plotting of the Mohr circle becomes a little easier and computation of this stresses on their plane AC also becomes a little easier. Now do we really need a geometrical method for determining this stresses? Since we know all the parameters on the right hand side we can always compute the stresses σ and τ . Then where do this geometrical methods score? The advantage of geometrical methods as you will see a little later again is that it gives you a very nice concept or ability to visualize how the stresses vary over the plane. A geometrical concept is always easy to visualize and comprehend than an equation. Thus the reason why we have gone to such great length has to understand how a Mohr's circle is plotted. It helps to visualize how the planes are oriented. How the stresses are varying from plane to plane and which is the plane which is critical and why. So let us see further.

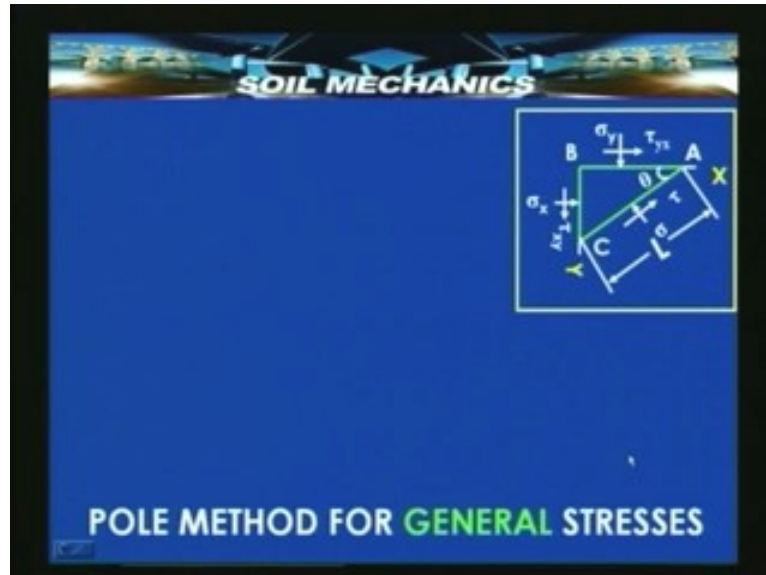
A method is known as the pole method which is also used for plotting the Mohr's circle. This is called a pole method because a specific point on the Mohr's circle which is known as the pole. For example to locate a pole you draw a line from a known point on the Mohr's circle parallel to the plane on which the state of stress represented by that point acts. What I mean is you have a Mohr's circle, you know the state of stress on a particular plane. Choose the point on the Mohr's circle which represents this state of stress and then draw a line parallel to the plane whose state of stress that point represents and then the point of intersection of this parallel line with the Mohr's circle is called the Pole is also sometimes referred to as the Origin of planes.

(Refer Slide Time: 26:41 min)



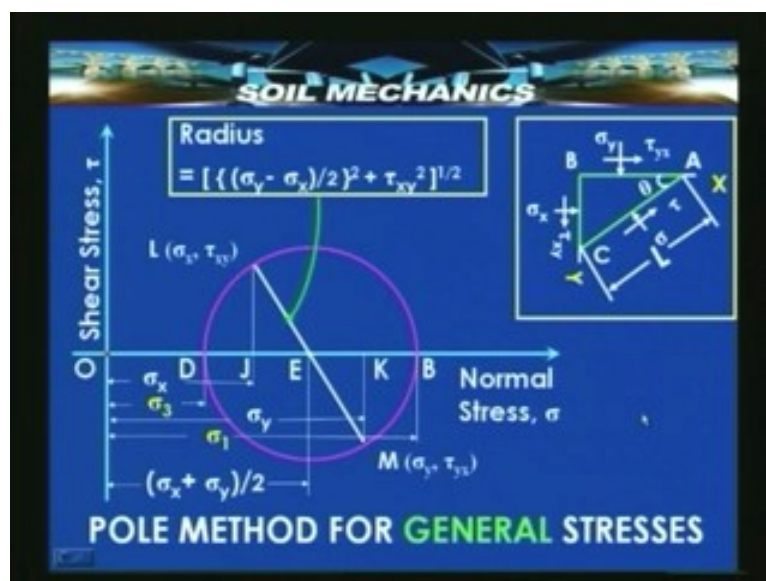
The reason why we see this is very simple. If you see the previous slide, the point C was located by taking an angle 2θ , measuring of an angle 2θ with respect to the normal stress axis is in the anti clockwise direction because the plane CA made an angle θ with respect to the horizontal axis rather than going by this concept. We can also proceed by simply knowing the orientation of the planes that's what the pole method is all about.

(Refer Slide Time: 28:20 min)



Start once again with this index or key diagram for general stresses. So now if we have general stresses how do we use the pole method for determining this sigma and tow? We follow a series of stress always as we have been doing in the earlier instances. First draw the normal stress axis, choose the origin and plot the vertical axis. Plot σ_x , find out the points J and L, plot σ_y find the points K and N, join LM get E, draw a Mohr's circle get the point D and B the radius will be given by this expression. Midpoint E will have coordinates σ_x plus σ_y by 2, D will be equal to σ_3 , B will be representing σ_1 and now comes the determination of the point C on the Mohr's circle which will corresponds to the state of stress sigma tow.

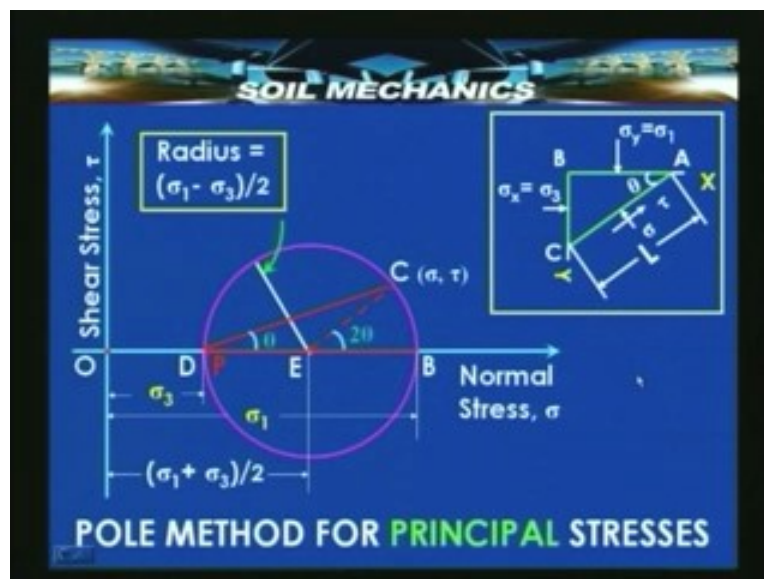
(Refer Slide Time: 29:33 min)



For this rather than plotting a line at an angle 2θ as we did in the previous instance, we will now choose the point M and draw a line MP. Why do we choose point M? Because point M on the Mohr's circle is a point whose stress coordinate are σ_y tow y x. That is nothing but the set of stresses, normal and shear acting on the plane AB which is horizontal. So point M represents the state of stress acting on the plane AB which is horizontal, so from point M if you draw a line MP parallel to the plane on which this state of stress acts, then the point P where it intersects the Mohr's circle is known as the pole. The property of the pole is any line that you draw through the point P the pole will have a corresponding point of intersection on the Mohr's circle and that point of intersection will have coordinates representing stresses which will be acting on the plane passing through the point P.

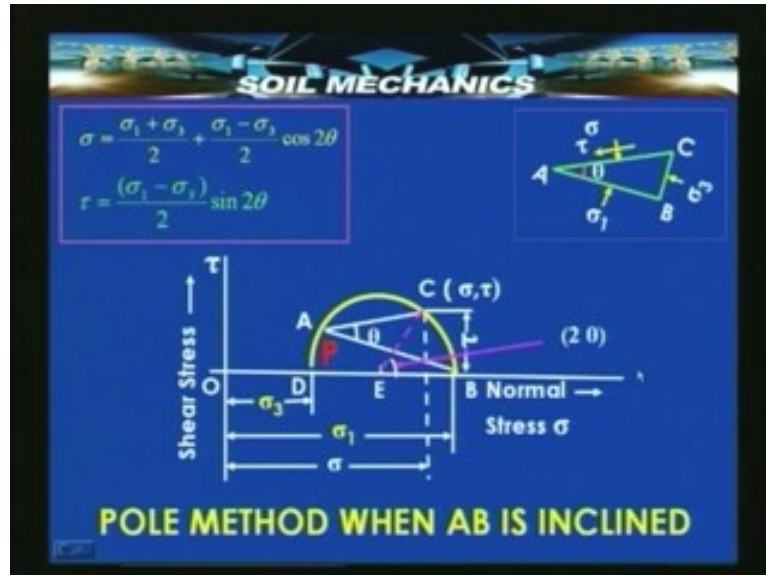
So for example in order to know the stress on the plane AC, from the pole draw line which represents the plane CA that is draw PC parallel to CA. Obviously then C which is the point of intersection of this red line with the Mohr's circle will have the state of stress given by the plane parallel to PC that is plane CA. Then it can also be easily shown that plotting an angle 2θ from here and joining E with C will also give you a same point C as the red line drawn from the point P. So this is an alternative method so called pole method for determining the point C. It can also be applied to a situation where we don't know the general state of stress but we are fortunate enough to know the normal and tangential stresses on the planes AB BC and CA. The value of σ and the value of τ are to be determined in terms of the σ_1 and σ_3 . The plotting of the Mohr's circle does not require any explanation. You can just watch the progression of steps; we have so far plotted the minor principle stress and major principle stress.

(Refer Slide Time: 33:33 min)



We have chosen the centre of the potential Mohr's circle; we have got the Mohr's circle and its radius. We have got the point C and how we have obtained is by choosing the point D which represents the state of stress corresponding to σ_3 . That means the plane DC will have to be a line which is horizontal passing through the point D.

(Refer Slide Time: 33:34 min)



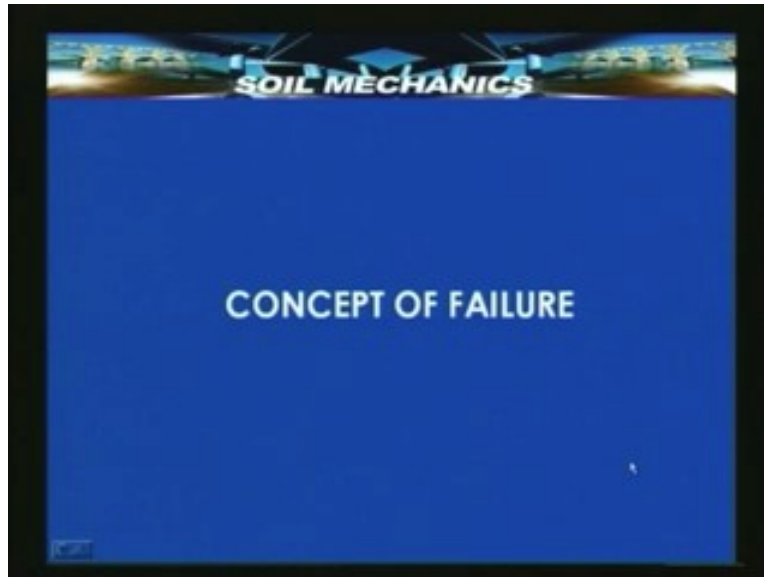
So from point B if you plot a line which is parallel to the plane AB on which this stress σ_{a1} acts then the point P becomes the pole and PC at angle theta will give you σ_{tow} and PC is plotted parallel to CA or alternately plot 2θ with respect to the line DB and get the point C. There is also a possibility that the line AB or the plane AB is not horizontal. We have been assuming that the 2 known planes are horizontal and vertical but that's only a special case. In the most general case the line AB representing the major principle plane could be an inclined plane. A line BC representing the minor principle plane could be a plane which is at right angled to AB and also therefore inclined to the vertical. In such a case if you have a plane AC inclined at angle theta to AB, we can follow the same Mohr's circle concepts and get the stresses on the plane AC, the difference will be that from the point B now rather than drawing a line horizontally to represent the plane on which stress is σ_{a1} .

Now we will draw a line BA which is parallel to the known direction of BA on which σ_{a1} is acting and where it ends that's the point P. And from P if you draw a line PC at angle theta that is parallel to AC then you get σ_{tow} . σ_{tow} will be now equal to O to this point and tow will be from here to C. And this also is possible to obtain by plotting a line to theta through this point E with respect to the horizontal. The corresponding expressions for σ and tow which we have already seen will be satisfied by this point C.

Now having heard an exposure to the method of determining the stresses on any plane let's see what is this concept of failure which will help us to access whether under given circumstances of shear stress and shear strength, failure can take place along a plane in a soil. So what's this concept of failure? Let us go back to physics and try to understand failure along a plane of contact between two bodies which are experiencing friction at the plane of contact. Let's say there is a rectangular block resting on a plane offering friction. This rectangular block let us say has a weight W, corresponding to W there will be a normal reaction N.

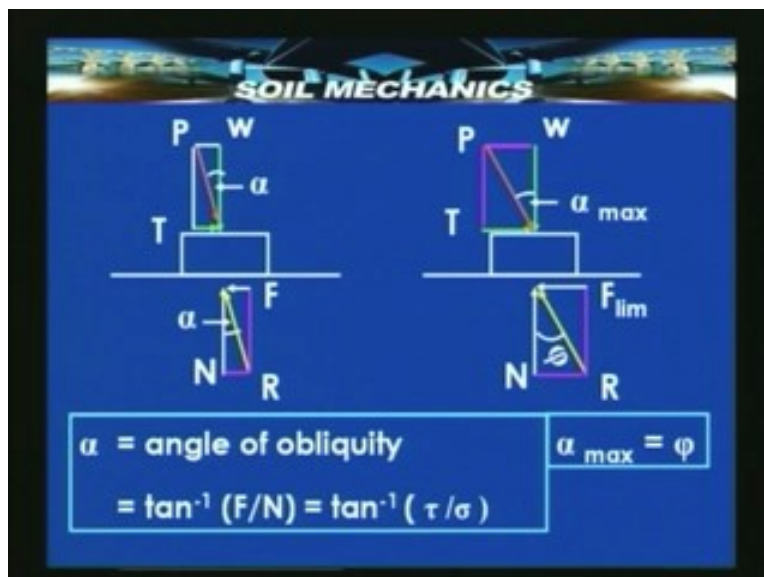
Now suppose you apply a shear force T , a tangential force T shown by this green line because there is friction between the bodies, a force F in the opposite direction will be automatically mobilized and that will resist T and for equilibrium T will be equal to F , N will be equal to W or in other words the resultant R will be equal to the resultant P .

(Refer Slide Time: 35:14 min)



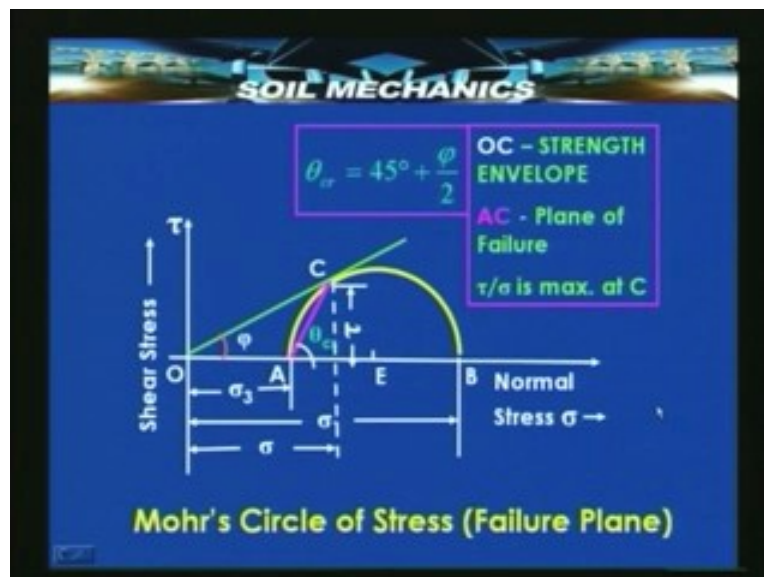
The angle which R makes with a vertical will be the angle which P makes with a vertical that is α . This is the situation when the box is in equilibrium but let us increase the value of T until the block reaches the state of so called incipient seepage. That is T has now achieved its limiting value corresponding to this, this angle α has reached its maximum possible value.

(Refer Slide Time: 35:39 min)



If that plane has a magnitude A for area then shear stress on this contact plane is F limiting upon the area, normal stress on the contact plane is N by area of contact and therefore α is \tan inverse of τ upon σ or ϕ is \tan inverse of τ upon σ . Whereas this expression is valid for all values of α when it is equal to ϕ it becomes valid for the limiting state. And this is what is known as the state corresponding to limit equilibrium and its only possible to know the shear stress τ at this particular state because we know that even a slight increase in τ will cause shear failure at the plane of contact. Whereas at any other value of τ less than the limiting value we only know that equilibrium exists we do not know really what fraction of τ has been really mobilized.

(Refer Slide Time: 39:21 min)



Having decided what criterion is satisfied during failure that is having known that for failure α has to be maximum or \tan inverse of τ upon σ has to be maximum. We can now define failure in terms of the Mohr's circle concept also. For example it is very obvious that failure takes place when τ upon σ is maximum because τ upon σ represents \tan of the maximum angle of obliquity that is ϕ . So we incidentally also have a criterion which is obvious from here that is $\tan \phi$, \tan of the angle of obliquity with its maximum value is equal to τ upon σ .

So $\tan \phi$ equal to τ upon σ or rather τ equal to $\sigma \tan \phi$ becomes the criterion defining failure. Go to Mohr's circle, find out at what point τ becomes equal to $\sigma \tan \phi$. Plot the normal stress, choose the origin, plot the vertical or shear stress, choose the point E which is the centre of the Mohr circle. Plot only half the Mohr's circle for convenience and clarity with EA as the radius, we know σ_1 and σ_3 . So we get the Mohr's circle, σ is the normal stress at any point on the Mohr's circle and τ is the shear stress of any point on the Mohr circle. And if we want to know what's that point on the Mohr's circle which will corresponds to this state of stress during failure. What we need to find out is at which point τ upon σ becomes maximum because that will represent the condition σ equal to or rather τ equal to $\sigma \tan \phi$ which defines failure.

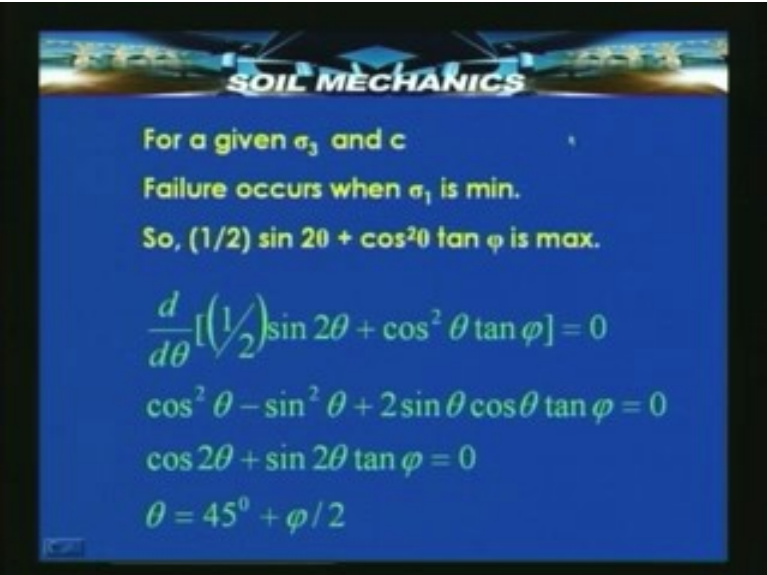
So suppose I choose this pole, we have already seen that this has got to be the pole for a situation where major and minor principle stresses are known and plotted. From the pole draw a line to a point C which will represent the point where τ upon σ will be maximum. Now we know that τ upon σ maximum corresponds to \tan inverse of the angle ϕ . So there exists an angle ϕ whose angle is τ upon σ and τ upon σ has its maximum possible value. Such an angle can only be this angle ϕ which can be obtained by drawing a tangent through the point C from point O. The tangent of this angle ϕ is τ upon σ therefore this angle ϕ is nothing but the angle of maximum obliquity which defines failure. So the plane AC which has been plotted from the pole upto the point C is nothing but the plane on which acts the stresses τ and σ corresponding to failure. So AC becomes the failure plane.

This failure plane AC makes an angle with the horizontal which we now call as θ critical because that corresponds to the critical possible angle. What's the value of θ critical at which the plane of failure will lie? θ critical from simple geometry is obviously equal to half the angle OCA plus ϕ . So 45° plus ϕ by 2 this means that if we know σ_1 , σ_3 and plot a Mohr's circle with E as a center, all that we need to do is go to point A draw a line AC at an angle θ critical equal to 45° plus ϕ by 2 and that point C will be representing the state of stress at the time of failure, incipient failure. Now this means that we need to know the angle ϕ , this means that we need to have a method to determine the angle ϕ . There are laboratory technique to determine the angle ϕ as we will be seeing it later.

Now how do we say that θ is equal to 45° plus ϕ by 2 from theoretical considerations? It's very simple, for a given value of σ_3 and C the failure will occur when σ_1 is minimum. Therefore take the expression for σ_1 which we are already familiar with and maximize that, when that is maximized you will have d by d θ is equal to this, equal to zero. This when you simplify, you will get $\cos 2\theta$ plus $\sin 2\theta \tan \phi$ is equal to zero. That is nothing but θ is equal 45° plus ϕ by 2.

From this we can derive further relationships using the Mohr's circle, suppose this is the Mohr's circle where this angle is defined as the θ critical equal to 45° plus ϕ by 2. We can also use the sine and cosine of this angle ϕ and show that \sin of this angle ϕ is nothing but $\sigma_1 - \sigma_3$ divided by $\sigma_1 + \sigma_3$. That's equal to this or from here we can show that σ_1 by σ_3 is equal to $1 + \sin \phi$ by $1 - \sin \phi$.

(Refer Slide Time: 44:05 min)



SOIL MECHANICS

For a given σ_3 and c
 Failure occurs when σ_1 is min.
 So, $(1/2) \sin 2\theta + \cos^2 \theta \tan \varphi$ is max.

$$\frac{d}{d\theta} \left[\left(\frac{1}{2} \right) \sin 2\theta + \cos^2 \theta \tan \varphi \right] = 0$$

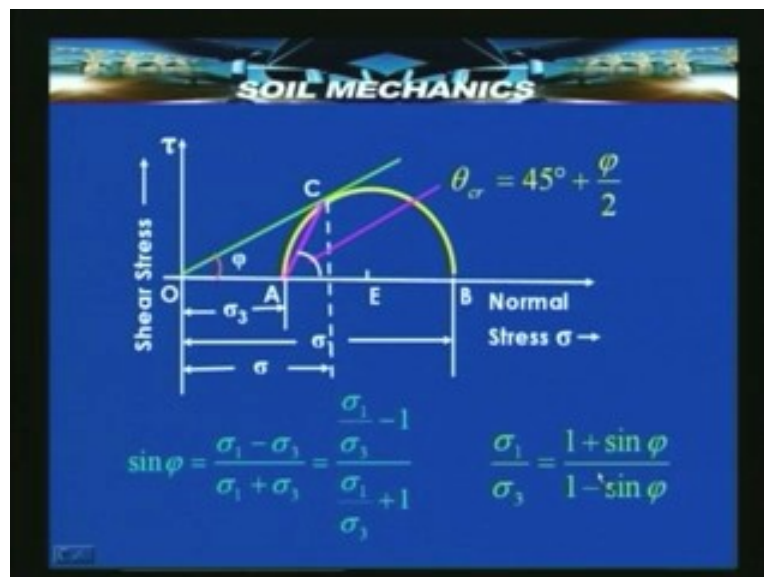
$$\cos^2 \theta - \sin^2 \theta + 2 \sin \theta \cos \theta \tan \varphi = 0$$

$$\cos 2\theta + \sin 2\theta \tan \varphi = 0$$

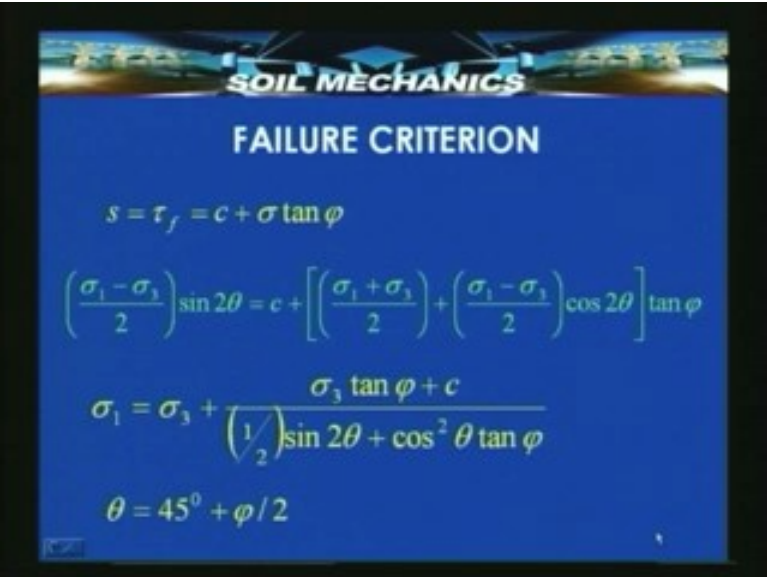
$$\theta = 45^\circ + \varphi / 2$$

All these different expressions are useful in different context to relate the plane of failure and the states of stress σ_1 and σ_3 . The failure criterion will also now look like this. We know that the failure criterion is shear stress is equal to shear strength equal to c plus $\sigma \tan \varphi$. The expression for c plus $\sigma \tan \varphi$ and τ will be equal to this from the Mohr's circle which we have just now seen and this can be simplified into an expression σ_1 equal to σ_3 plus all this in which θ will corresponds to 45 degree plus φ by 2 the angle made by the plane of failure, the critical angle.

(Refer Slide Time: 44:48 min)



(Refer Slide Time: 45:33 min)



SOIL MECHANICS

FAILURE CRITERION

$$s = \tau_f = c + \sigma \tan \phi$$

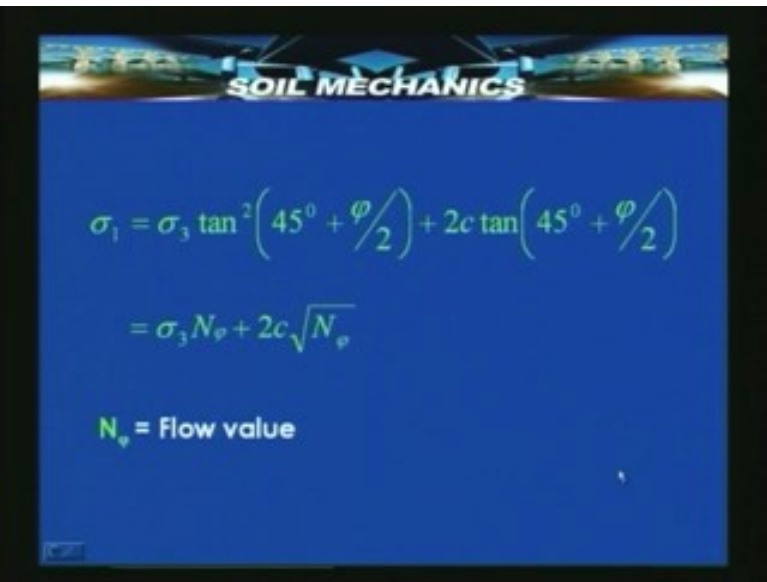
$$\left(\frac{\sigma_1 - \sigma_3}{2} \right) \sin 2\theta = c + \left[\left(\frac{\sigma_1 + \sigma_3}{2} \right) + \left(\frac{\sigma_1 - \sigma_3}{2} \right) \cos 2\theta \right] \tan \phi$$

$$\sigma_1 = \sigma_3 + \frac{\sigma_3 \tan \phi + c}{\left(\frac{1}{2} \right) \sin 2\theta + \cos^2 \theta \tan \phi}$$

$$\theta = 45^\circ + \phi / 2$$

This means that failure will take place along a plane which makes this critical angle 45 degree plus phi by 2 provided the known states of stress σ_1 and σ_3 and the values of c and ϕ at the soil satisfies this relationship. The values of c and ϕ of the soil which are known as the shear parameters can be determined from the laboratory. Then knowing that the plane of failure makes an angle θ equal to 45 degrees plus phi by 2 and knowing σ_1 and σ_3 it is possible to check whether failure will take place along a given plane or not.

(Refer Slide Time: 46:54 min)



SOIL MECHANICS

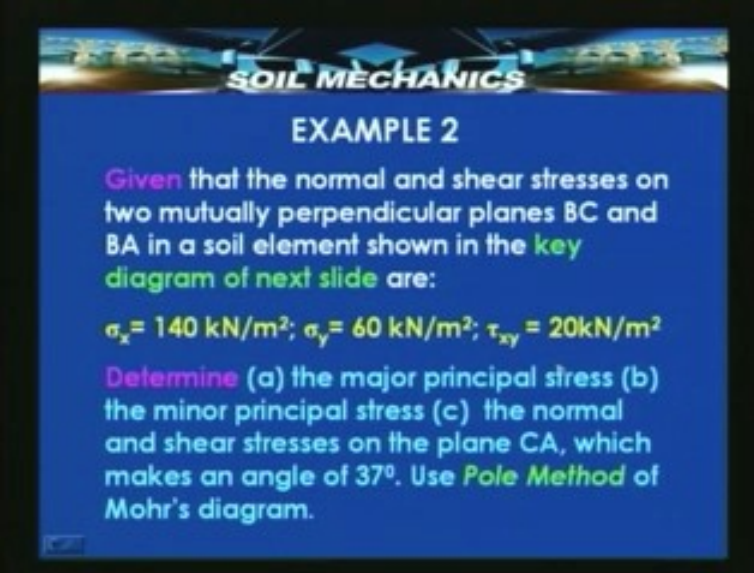
$$\sigma_1 = \sigma_3 \tan^2 \left(45^\circ + \frac{\phi}{2} \right) + 2c \tan \left(45^\circ + \frac{\phi}{2} \right)$$

$$= \sigma_3 N_\phi + 2c \sqrt{N_\phi}$$

N_ϕ = Flow value

That same expression which we saw in the previous slide can also be rewritten in this form where we use now a notation N_{ϕ} known as the flow value to represent $\tan^2 45^\circ + \phi/2$. Let us see a couple of examples now. In the first example we have normal and shear stresses on two mutually perpendicular planes BC and BA on a soil element are given. This are plotted in a key diagram which appears on the next slide. The stresses σ_x , σ_y , τ_{xy} are therefore known. What is required is determine the major principle stress, the minor principle stress, the normal and shear stress on the plane CA which makes an angle of 37° with the horizontal and for this purpose use the pole method and the Mohr's diagram.

(Refer Slide Time: 47:06 min)



SOIL MECHANICS

EXAMPLE 2

Given that the normal and shear stresses on two mutually perpendicular planes BC and BA in a soil element shown in the key diagram of next slide are:

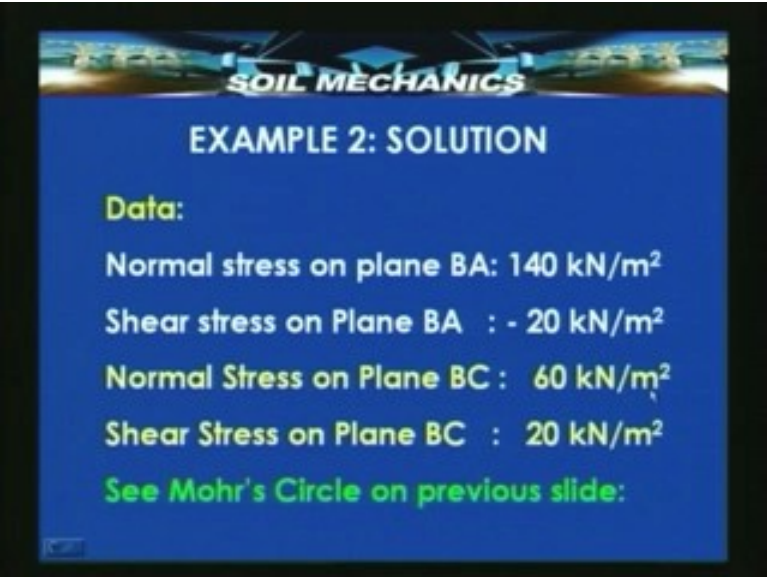
$\sigma_x = 140 \text{ kN/m}^2$; $\sigma_y = 60 \text{ kN/m}^2$; $\tau_{xy} = 20 \text{ kN/m}^2$

Determine (a) the major principal stress (b) the minor principal stress (c) the normal and shear stresses on the plane CA, which makes an angle of 37° . Use Pole Method of Mohr's diagram.

We have seen already in detail the step by step construction of the Mohr's circle, therefore we will not go through that again. All that we need to know is we have a system of general stresses here now where σ_x , σ_y and τ_{yx} are known. So we plot σ_x , σ_y , get the points J and K, points L and M, plot the Mohr's circle and obtained the point M corresponding to the state of stress on the plane AB. Once we know that since plane AB is horizontal, draw the line MP horizontally and determine the pole P. Once you know the pole P you can determine the point C whose coordinates are **C to σ to τ** .

Once you know this point C corresponding to this state of stress, we can easily find out the values of normal stress and shear stress on these planes. Whatever is known is plotted first so normal stress on BA is plotted as 140, normal stress on BC is plotted is 60, shear stress on BA and BC are plotted as -20 and +20 to get the Mohr's circle which was shown on the previous slide.

(Refer Slide Time: 43:46 min)



SOIL MECHANICS

EXAMPLE 2: SOLUTION

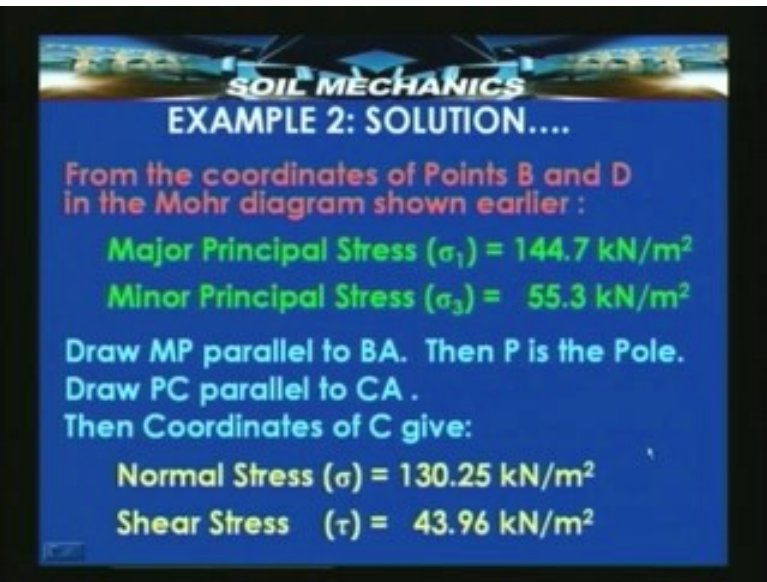
Data:

- Normal stress on plane BA: 140 kN/m^2
- Shear stress on Plane BA : $- 20 \text{ kN/m}^2$
- Normal Stress on Plane BC : 60 kN/m^2
- Shear Stress on Plane BC : 20 kN/m^2

See Mohr's Circle on previous slide:

From the coordinates of the points D and E which correspond to major and minor principle stresses, we can measure off these values. We can also use the pole to get the point C and measure of the value σ and τ from the Mohr diagram. If we calculate it rigorously from the formula also we will get the same stresses. The next example is one in which σ_1 and σ_3 are now known. So this is instead of a state of general stresses this is a state of specific set known stresses.

(Refer Slide Time: 49:11 min)



SOIL MECHANICS

EXAMPLE 2: SOLUTION....

From the coordinates of Points B and D in the Mohr diagram shown earlier :

- Major Principal Stress (σ_1) = 144.7 kN/m^2
- Minor Principal Stress (σ_3) = 55.3 kN/m^2

Draw MP parallel to BA. Then P is the Pole.
Draw PC parallel to CA .
Then Coordinates of C give:

- Normal Stress (σ) = 130.25 kN/m^2
- Shear Stress (τ) = 43.96 kN/m^2

(Refer Slide Time: 49:28 min)

SOIL MECHANICS
EXAMPLE 3

Given: $\sigma_1 = 200 \text{ kN/m}^2$; $\sigma_3 = 60 \text{ kN/m}^2$

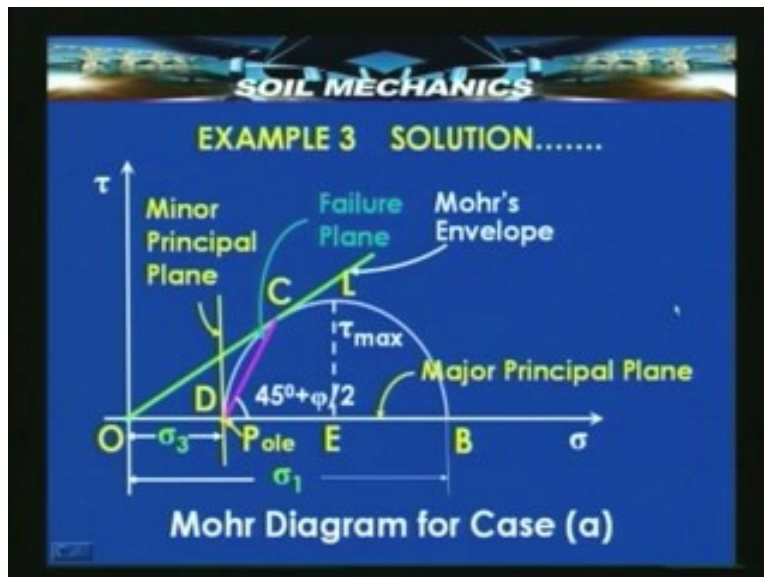
Determine [Using Mohr's Circle & Pole Method]:

- 1) the Pole
- 2) the Major and Minor Principal plane
- 3) the maximum shear stress
- 4) the failure plane and the stresses on it

a) When σ_1 plane is vertical and σ_3 is horizontal ,
b) When σ_3 plane is vertical and σ_1 is horizontal

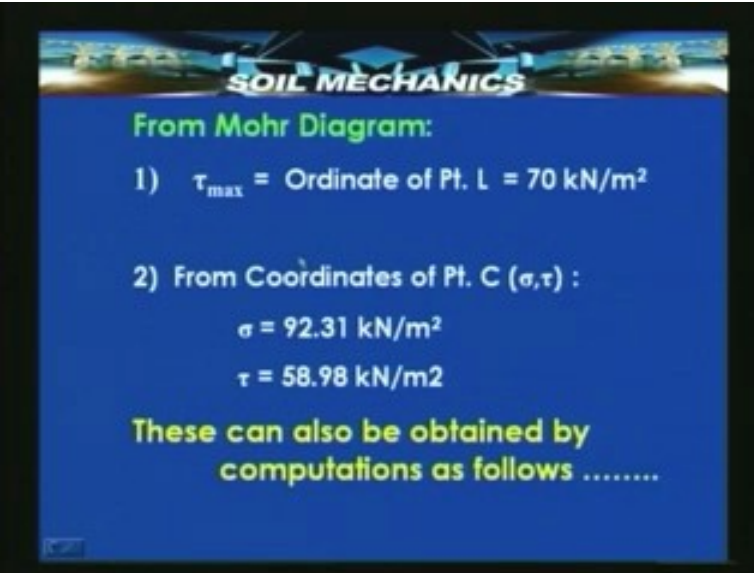
Again using the Mohr's circle and pole method determine first the pole then the major and minor principle planes, the maximum shear stress and the failure plane and the stresses on the failure plane. Under two conditions, one when σ_1 plane that is the major principle plane vertical and the minor principle is horizontal and case b when the major principle is horizontal.

(Refer Slide Time: 50:11 min)



The solution is very simple as before plot the Mohr's circle and also after determining the pole P corresponding to the principle stress σ_1 , get the failure plane corresponding to the point C and get the stresses.

(Refer Slide Time: 50:28 min)



SOIL MECHANICS

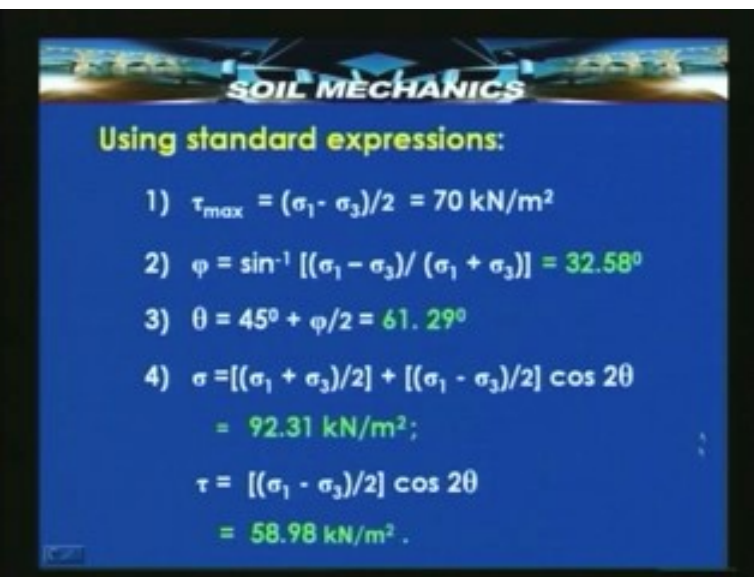
From Mohr Diagram:

- 1) $\tau_{\max} = \text{Ordinate of Pt. L} = 70 \text{ kN/m}^2$
- 2) From Coordinates of Pt. C (σ, τ):
 $\sigma = 92.31 \text{ kN/m}^2$
 $\tau = 58.98 \text{ kN/m}^2$

These can also be obtained by computations as follows

The τ_{\max} can be measured off as an ordinate of the point L that will come to 70. From the coordinates of the point C obtained from the pole method, σ and τ can be calculated as 92 and 58 respectively or from rigorous computation we can get τ_{\max} must be equal to $\sigma_1 - \sigma_3$ by 2, the radius of the Mohr's circle that's again coming to 70. The angle ϕ will be \sin^{-1} of the quantities σ_1 and σ_3 which are known. θ is again known, 45 degrees plus ϕ by 2. We can calculate σ and τ from these quantities once again by using the same expressions.

(Refer Slide Time: 50:44 min)

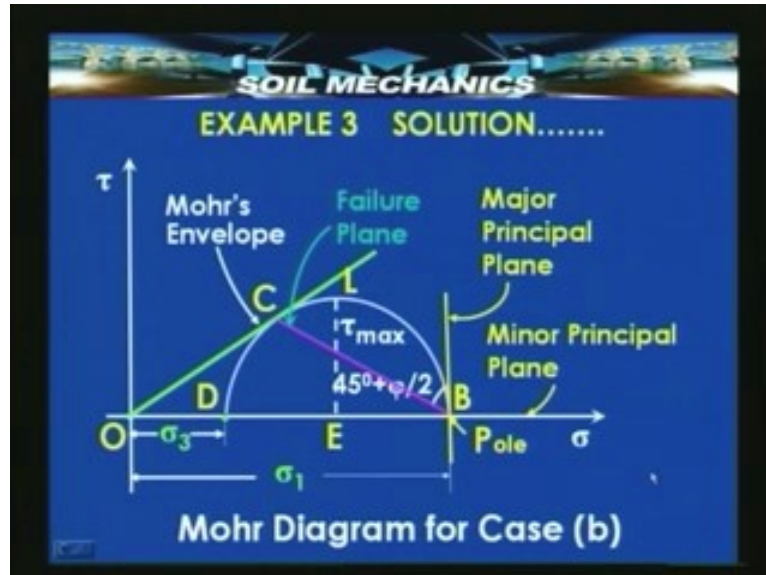


SOIL MECHANICS

Using standard expressions:

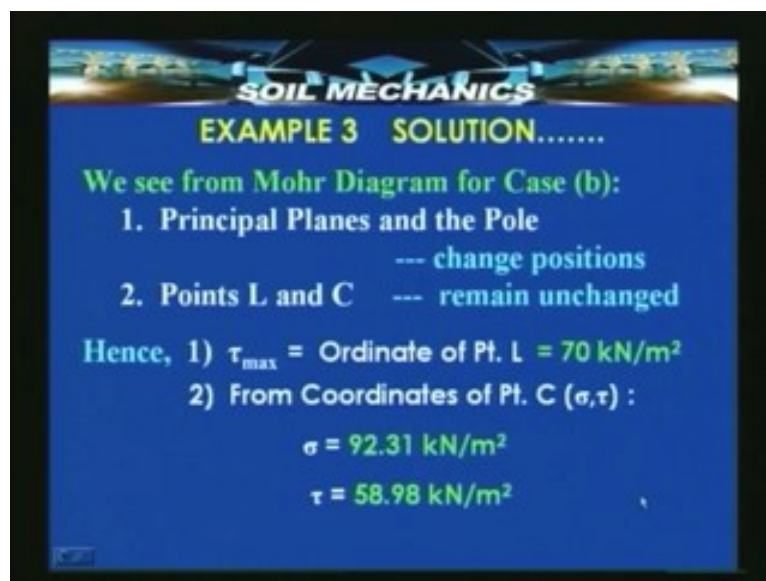
- 1) $\tau_{\max} = (\sigma_1 - \sigma_3)/2 = 70 \text{ kN/m}^2$
- 2) $\phi = \sin^{-1} [(\sigma_1 - \sigma_3) / (\sigma_1 + \sigma_3)] = 32.58^\circ$
- 3) $\theta = 45^\circ + \phi/2 = 61.29^\circ$
- 4) $\sigma = [(\sigma_1 + \sigma_3)/2] + [(\sigma_1 - \sigma_3)/2] \cos 2\theta$
 $= 92.31 \text{ kN/m}^2$;
 $\tau = [(\sigma_1 - \sigma_3)/2] \sin 2\theta$
 $= 58.98 \text{ kN/m}^2$.

(Refer Slide Time: 51:14 min)

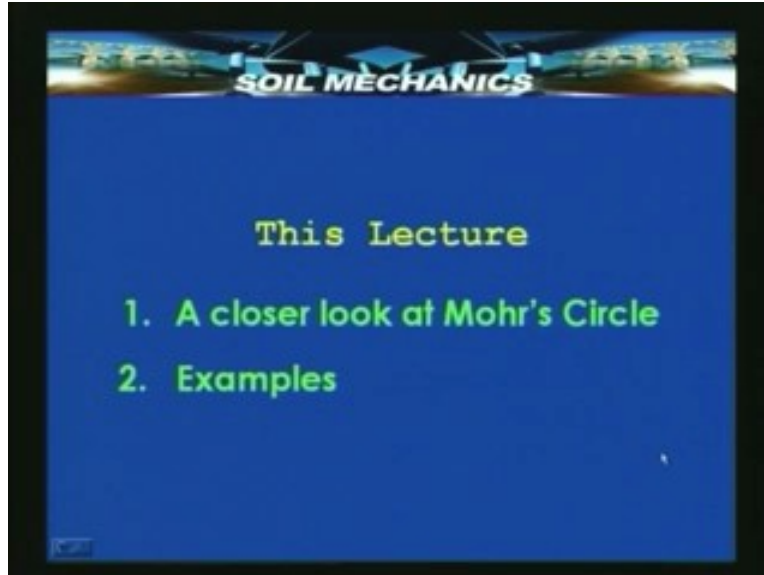


This is possible to compute the complete set of stresses either graphically or from rigorous computations and we will find that all that has happened is the principle planes and the pole have changed positions and points L and C however remain unchanged. And the maximum shear stress and the coordinates of the point C give rise to the values of τ_{max} and σ_{tow} . Now in this lecture we have taken a close look at how to plot a Mohr's circle and what its cable of giving and we have seen a couple of examples to illustrate this concept and reinforce these.

(Refer Slide Time: 51:24 min)

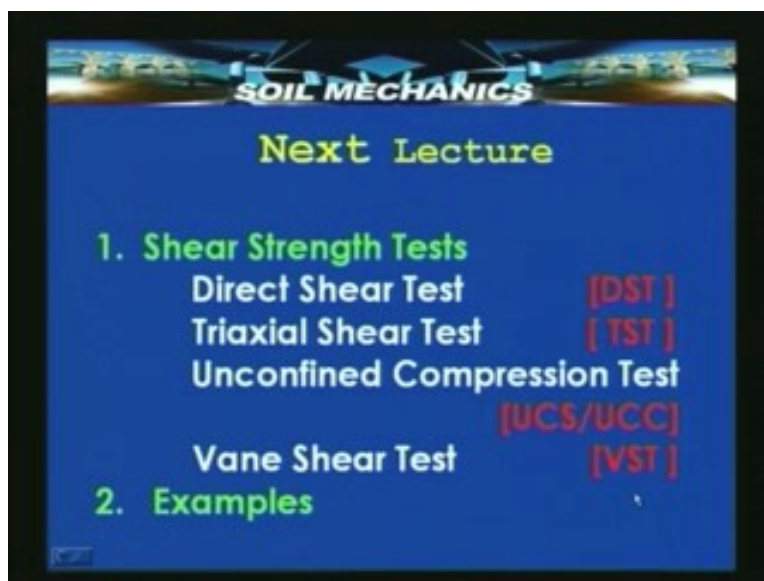


(Refer Slide Time: 51:48 min)



In the next lecture we shall be seeing the methods of determining the shear strength of a soil because that's what is going to help us to check whether the stresses which we have so far calculated are going to cause a failure or not. And we will also see a few examples during the next lecture and this way we will conclude today's lecture by saying that we now have a basis with a help of which we can check whether a given soil along a given plane is likely to undergo shear failure or not.

(Refer Slide Time: 52:00 min)



And this will be useful in many instances such as foundations retaining walls etc.
Thank you