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ADVANCED GEOTECHNICAL
ENGINEERING

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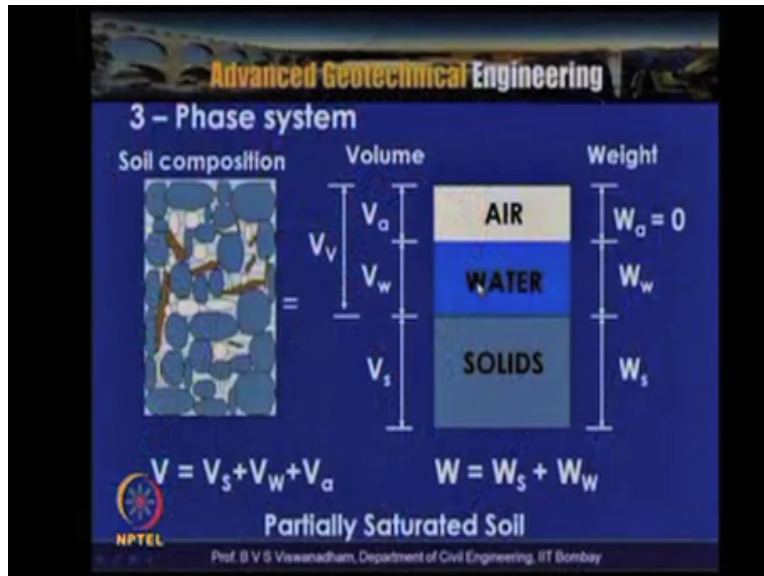
Department of Civil Engineering

IIT Bombay
Lecture No. 02

Module-1
Three Phase Soil System,
Volumetric ratios and
Basic relationships

Welcome to lecture 2 of module 1 for the course advanced geotechnical engineering in the previous lecture we have understood about origin of soils and their type of soils in this lecture we would like to understand about phase properties phase relationships using phase diagrams and we will try to solve some couple of problems so as we discuss soil is a three phase material where you have a soil composition which consists of solids which consists.

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Of air water and solids so if you separate the soil composition into three predominant phases they are a water and solids and if you put weights on the left hand side as on the weights on the right hand side where weight of air is equal to zero and weight of water and weight of solids and if you put volume on left hand side the volume occupied by voids which comprises of volume of a and volume of water and volume of solid.

So volume of voids plus volume of solids is total volume so $V = V_a + V_s + V_w + V_a$ and weight total weight is equal to weight of solids plus weight of water so this is this is a case for a partially saturated soil.

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Volumetric ratios

Volumetric ratios commonly used in soil mechanics are:

- Void ratio e
- Porosity n
- Degree of Saturation S_r
- Air content a_c
- Air void ratio or Percentage air voids n_a

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Now based on the phase diagram whatever we have discussed we can deduce number of volumetric ratios and these are used in soil mechanics and geotechnical engineering particularly they are known as void ratio e porosity n degree of saturations S_r air content a_c air void ratio or percentage void percentage air voids n_a so from the subsequent slides we will try to understand about these definitions of these and inter relations among these properties.

So void ratio is defined as the ratio of the volume of voids to the volume of solids which can be written.

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Void ratio e is defined as the ratio of volume of voids to the volume of solids

$$e = V_v / V_s$$

- Volume of voids V_v refers to that portion of the volume of the soil not occupied by solid grains
- Since the relationship between Volume of air V_a and Volume of water V_w usually changes with ground water conditions as well as imposed loads, it is convenient to designate all the volume not occupied by solid grains as void space Volume of voids V_v

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As e is equal to V_v / V_s as it can be seen here e is equal to V_v / V_s so volume of voids V_v suffix refers to that portion of the volume of the soil not occupied by the solid grains since the relationship between volume of air and volume of voids volume of water usually changes with around with groundwater conditions as well as imposing loads suppose if the groundwater table changes or fluctuates or because of the imposed loads.


If the soil undergoes some compression are some expulsion of water there is a possibility that volume of air and volume of water can change it is convenient to designate all the volume not occupied by the solid grains as wide space that is volume of whites so the volumes which are not occupied by the solid grains is indicated as the volume of whites so white ratio is defined as V_v / V_s now if you look into for any solid material a is equal to zero that means this absence of whites.

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$e = 0 \Rightarrow$ absence of voids (solid material)
 $e > 1 \Rightarrow V_v \gg V_s$ in the soil mass

<u>Soil type</u>	<u>void ratio e</u>
<i>Uniform sand, loose</i>	0.85
<i>Mixed-grain sand, dense</i>	0.43
<i>Soft glacial clay</i>	1.20
<i>Soft highly organic clay</i>	3.00
<i>Soft Bentonite</i>	5.20

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And e greater than one that means that the volume of voids is militants more than the volume of solids for a typical soil the void ratio ranges from say 0.8 to or 0.4 to around five point two in this case in the table here the typical soils are even say for example you know what is uniform sand and which is in the loose state which we are going to discuss in subsequent lectures so if you have got a uniformly graded sand particles the void ratio is about 0.85.

If you have got mix it grain sand that means that and it is in dense condition in that case the void ratio can be as low as 0.43 if you have got a glacial clay the void ratio can be up to 1.2 and soft highly organic clay that that means the soil which actually has got organic matter the clay which actually has got organic matter can have void ratio up to 3.0 in the case of soft Bentonite the void ratio can be as high as 5.2 so the void ratio for sandy or a sandy soils is relatively low and a clay soils is relatively high.

So if you make this observation here the void ratio of sandy soils is on the lower side compared to wide ratio of the clay soil so in subsequent lectures we will understand why this you know particular observations.

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➤ In nature, even though the *individual void spaces* are larger in coarse-grained soils, the void ratios of fine-grained soils are generally higher than those of coarse-grained soils.

The ratio of volume of voids V_v to total volume V is defined as Porosity n .

$$n = V_v/V$$

But $V = V_v + V_s = (1 + V_v/V_s) V_s = (1 + e) V_s$

$$\Rightarrow n = e/(1 + e)$$

The porosity provides a measure of the permeability of a soil

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So in nature even though the individual wide spaces are larger in coarse granular soils that that means that in nature if you look into this even though the individual wide spaces are larger in coarse-grained soils the void ratios of fine-grained soils are generally higher than those of coarse-grained soils so in the previous slide we observed that the void ratio of sandy soils are relatively lower compared to clay soils that means that if you put as the coarse-grained soil.

The void ratios are of the individual grains the pore spaces in nature even though the individual void spaces are pore spaces are larger in coarse-grained soils the void ratio of fine-grained soils are generally higher than those of coarse-grained soils then we define one more parameter called porosity this is nothing but is a ratio of volume of voids to total volume this is generally expressed as a person in a percentage so if you take $V_v + V_s$ is equal to total volume if you write V is equal to $1 + V_v/V_s$ into V .

So you can write that as $1 + V_v/V_s$ that means that you can actually get an interrelation between n porosity and void ratio which is nothing but n is equal to $e/(1 + e)$ if you look into this the porosity provides a measurement measure of the permeability of a soil if the soil is more porous the permeability is high that means that the porosity provides a measure of the permeability of a soil permeability is a property of a soil which is defined as it is ease with which the water can flow through the soil

The porosity indicates here a large porosity provides a measure of the permeability of a soil so this porosity n of soil.

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➤ Porosity n of soil cannot exceed 100 %

$$0 < n < 100$$

Porosity n of a natural deposit =

f (Shape of grains, uniformity of grain size, and the conditions of sedimentation.

➤ $n = 25 - 50$ % (natural sands)

➤ $n = 30 - 60$ % (soft natural clays)

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Cannot exceed 100 percent that means that it ranges between 0 and 100 porosity n of a natural deposit can be a function of the shape of grains that means that the type shape of the grain uniformity of grain size whether it is same size or different sizes and the conditions of sedimentation so for natural sands the porosity is in the range of n is equal to 25 to 50 percent and for a soft natural clays the porosity can be in the range of 30 to 60 percent.

So we have defined porosity and porosity we said that which is ratio of V_v by V that is volume of voids to the total volume and the porosity n of a natural deposit is a function of shape of the grain uniformity of grains and the conditions of sedimentation now if you look into this in soil mechanics or in geotechnical we use wide ratio the reason is out of the void ratio e and porosity what ratios used frequently in soil engineering because of the particular region.

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Out of void ratio e and Porosity n , void ratio is used frequently in soil engineering because:

$$e = V_v/V_s \qquad n = V_v/V$$

Any change in V is a direct consequence of a similar changes in V_v and while V_s remains the same.

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If you see the definition here E is equal to V_v/V_s , n is equal to V_v/V so any change in a volume is a direct consequence of a similar change in volume of voids and while V_s remains constant so because of this if you see the numerator and denominator V_s which is actually remains constant in case of void ratio but in case of porosity both numerator and denominator undergoes changes.

So hence Weight ratio E is frequently used in soil engineering now another definition which is actually called as water content which is very much important the water content of a soil is defined as a weight of water - weight of the solids.

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Water content

The water content w is given as W_w/W_s , expressed as a percentage.

- where W_w = Weight of water
- W_s = Weight of solids (dry)

Natural water content of **fine-grained soils** > **coarse-grained soils**. [No upper limit to w]

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Expressed as a percentage so W_w/W_s that is w suffix W is nothing but weight of water W_s is nothing but the weight of the solids in dry state so natural water content of fine-grained soils is greater. Than coarse grained soils and there is no upper limit for the water content so this water content is also called as moisture content or in case if it is in natural state it is called natural moisture content or in-situ moisture content so the water content W .

Which is defined as weight of water - weight of solids and which is actually not having any upper limit and for some soft soil deposits the water content that can be up to 500 percent and we also said that the natural water content offline-grained soils is high greater than coarse grained soils then there is another one important term.

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Degree of saturation S_r

➤ For a fully saturated soil-water system, since all the voids will be completely filled with water:

$V_v \gamma_w = W_w$ where γ_w = unit weight of water

For partial saturation: $(V_v - V_a) \gamma_w = W_w$


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Which indicates the degree of saturation of SI so which is defined as volume of water to volume of voids that means that volume of water within volume of voids so this how it can be deduced let us see-through this slide for a fully saturated soil water system since all the voids will be completely filled with water we can write $V_v \gamma_w$ as weight of water that means that weight of water is nothing but γ_w that is the unit weight of water.

Which is in SI units can be taken as nine point eight one kilo Newton per meter cube so γ_w , γ_w is equal to $V_v \gamma_w$ so for partial saturation we can write $(V_v - V_a) \gamma_w$ is equal to weight of water that means that what we did is that we have taken that weight of that volume of voids which is completely saturated soil volume of voids is equal to volume of water now for partial saturation.

We can write $(V_v - V_a) \gamma_w$ into γ_w now the relationship between S_r are the degree of saturation can be obtained as S_r is equal to $(V_v - V_a) \gamma_w$ by γ_w .

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Hence the relationship for S_r , the degree of saturation is given as:

$$S_r = [(V_v - V_a)\gamma_w] / V_v \gamma_w = V_w / V_v$$

S_r is the ratio of the volume of water to the volume of void space (Generally expressed as a Percentage)

$$0 < S_r < 100$$

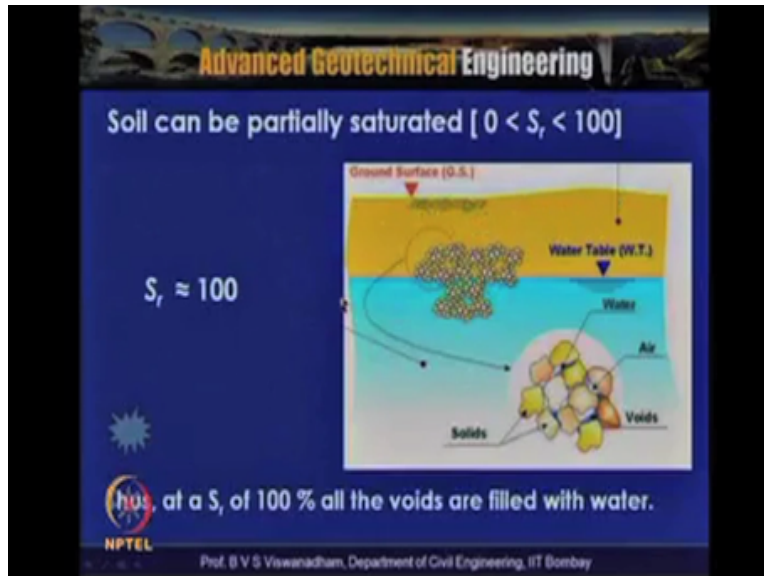
- For completely dry soil $S_r = 0$;
 For fully saturated soil $S_r = 1$ or (100 %)

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Divided by $V_v \gamma_w$ so if you simplify this you will get V_w / V_v so S_r is the ratio of volume of water to volume of void spaces this generally expressed as percentage if this it ranges from 0 to 100 if S_r our degree of saturation is equal 100 that means that the soil is completely saturated if you have a soil with the 80 percent saturation then we can say that the soil is partially saturated so for fully saturated soil S_r that S_r or degree of saturation is equal to 100 percent.

So soil can be partially saturated and if you look into this diagram if this is the ground surface what you see is the ground surface.

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And this is the water table location the hydrostatic groundwater location the soil above this is the main in partially saturated this is because of the temperature fluctuations as well as the partial the water evaporation which takes place in this soil the soil below the water table is said to be completely saturated except some minor occurrence of air bubbles otherwise you can say that the soil is completely saturated it means that all the voids.

Which are actually filled with water so at S_r is equal 100% all voids are completely filled with the water so below the water table we can say that the soil is completely saturated above the water table unless the type of soil is different you actually have a partially saturated soil occurs.

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The slide is titled "Advanced Geotechnical Engineering" and "Degree of Saturation of Sand in various states". It contains a table with two columns: "Condition of Sand" and "S_v [%]". The table lists six conditions: Dry (0%), Humid (1-25%), Damp (26-50%), Moist (51-75%), Wet (76-99%), and Saturated (100%). Below the table, it states "- Valid only for sands". The NPTEL logo is in the bottom left, and the footer text is "Prof. B V S Viswanadham, Department of Civil Engineering, IIT Bombay".

Condition of Sand	S _v [%]
Dry	0
Humid	1-25
Damp	26-50
Moist	51-75
Wet	76-99
Saturated	100

- Valid only for sands

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So if this particular slide shows a typical you know variation of degree of saturation for sands so this is strictly valid for sands based on the condition of sand if it is dry the degree of saturation is equal to zero humid one to 25 % and damp 26 to 50 %moist 51 to 75 % wet 76 to 99 saturated 100 that means that this is a degree of saturation if it is 100 then we can say that the zones of sand deposit is completely saturated so degree of saturation S R fine or silt sands are moist wet are hydrated clays are always completely.

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Degree of saturation S_r

- Fine or silty sands are moist, wet or saturated.
- Clays are always completely or nearly saturated except in the layer of soil subjected to seasonal variation of temperature and moisture.

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Or nearly saturated except .In the layer of soil subjected to seasonal variation of temperature and moisture fluctuations so what we discussed in the previous slide is that clays are always completely are nearly saturated except in layer of soil subjected to seasonal variation of temperature and moisture now we define one more parameter which is called a content air content is called a suffix is indicated by a suffix C.

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Advanced Geotechnical Engineering

$$\begin{aligned}
 \text{Air content } a_c &= V_a / V_v \\
 &= [V_a + V_w - V_w] / V_v \\
 &= [V_v - V_w] / V_v \\
 &= 1 - S_r
 \end{aligned}$$

$a_c = 0$ for saturated soil
 $a_c = 1$ for dry soil


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Which is nothing? But volume of air in volume of whites that means that which is ratio of volume of air to volume of whites degree of saturation we defined as volume of water to volume of whites so here the air content is nothing but volume of air in volume of whites so we can write here $V_a + V_w - V_w$ by V_v so if you write $V_a + V_w$ as $V_v - V_w$ we can write this as V_w by V_v is S_r that is degree of saturation.

So we can write V_v by V_v is equal to 1 so the air content is equal to $AC = 1 - S_r$ so if $S_r = 1$ that means that for saturated soil air content AC is equal to 0 that means that for a saturated soil air content a suffix C is equal to 0 for a dry soil air content is 1 because all the voids are filled with our air within the whites.

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Advanced Geotechnical Engineering


Air void ratio or Percentage air voids n_a

$$= V_a/V$$

By writing as $V_a/V_v/(V/V_v)$

$$= n a_c$$

Using $n = V_a/V$
and $a_c = V_a/V_v$

$$= n (1 - S_r)$$


Then another parameter which is actually called as a void ratio or percentage water percentage air voids so another parameter which is actually called as air void ratio or percentage air voids n_a suffix a which is defined as volume of a to total volume so by writing V_a into V/V divided by V_v into V_v/V_v and using n is equal to V_a/V by V which is nothing but porosity we have defined earlier and a suffix C air content is equal to V_a by V_v .

We can write air void ratio or percentage air voids as $n a_c$ that means that if you substitute a_c is equal to $1 - S_r$ we can write percentage air voids n_a is equal to porosity into $1 - S_r$ so again if you see when $S_r = 1$ percentage air voids $n_a = 0$ that means that air void ratio percentage air voids $n_a = n(1 - S_r)$.

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Advanced Geotechnical Engineering

➤ Unit weight $\gamma = (W/V)$ is one of the most important physical properties of the soil

-The unit weight must be expressed with due regard to the state of soil.

$\gamma = f(\text{unit weight of solid constituents, } n, \text{ and } S_v)$

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The another important parameter as far as soil engineering or geotechnical engineering is concerned is unit weight of the soil and this is actually has got number of terminologies depending upon the state of saturation or whether it is in the dry state whether it is in saturated state or whether it is in moist or partially saturated state so the unit weight general definition γ is nothing but weight of the soil to the total volume weight of the soil mass.

To the total volume is one of the most important physical properties of the soil the unit weight must be expressed with due regard to the state of soil so as I said earlier the unit weight must be expressed with due regard to the status soil that means that whether it is a dry or moist or saturated so unit weight is a function of the unit weight of the solid consistent the type of the grains which are is composed of and porosity whether it is closely Platte Packer or lose repack and degree of saturation.

So the application of why do we want the unit weight of the soil suppose if you are what a retaining wall and the soil is you know placed behind the wall the soil exerts the pressure so if you wanted to calculate the lateral pressures you need to calculate what is the earth pressure so in this case in order to compute a pressure you need the unit weight of the soil for example if you wanted to determine a vertical stress or a total stress.

At a particular depth because of the certain depth of the soil or certain state of the soil then you need to know the unit weight of the different layers which are actually above that particular point

of interest so if you wanted to determined you know the stresses in the soil or say earth pressures in behind the walls you need to know what is the unit weight of the particular material.

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Bulk unit weight γ_b (for a partially saturated soil)

= Total weight of soil mass / Total Volume

= $(W_w + W_s) / (V_w + V_s + V_a)$

For a saturated soil $\gamma_b = \gamma_{sat} \Rightarrow V_a = 0$

= $(W_w + W_s) / (V_w + V_s)$

Where γ_{sat} = Saturated unit weight of the soil

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For a partially saturated soil bulk unit weight or γ bulk which is also it called as moist unit weight which is nothing but total weight of soil mass to the ratio of the total rate of soil mass to total volume so γ bulk is equal to we can write as weight of water plus weight of solids divided by volume of water+ volume of solids +volume of a weight of the air is equal to zero that means you are here having only weight of water + weight of solids divided by total volume for a saturated soil γ bulk is equal to γ saturated γ suffix b= γ saturated.

So because and volume of air is equal to zero in that situation for a γ sat it is defined as weight of water +weight of such weight of solids divided by volume of voids volume of water + volume of solids where γ sat is nothing but the saturated unit weight of this side for example this agitated unit weight of the soil can occur if the if you find a soil strata below the ground water table in another case for example dry unit weight.

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Dry unit weight γ_d
For a dry soil $\gamma_d = (W_s) / (V_a + V_s) \Rightarrow V_w = 0$

$$\begin{aligned} \gamma_d &= W_s / V \\ &= (W - W_w) / V \\ &= [W / V - w W_s / V] \\ &= \gamma_{bulk} - w \gamma_d \end{aligned}$$

$$\gamma_d = \gamma_b / (1 + w)$$

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So dry unit weight which is indicated as γ suffix D for a dry soil $\gamma_D = W_s$ divided by $V_a + V_s$ here because the soil is in dry volume of water occupied in the voids is zero that means that volume of water is equal to zero in that situation it is written as for a dry soil W_s divided by total volume which is nothing but volume of air + volume of solids so γ_D is equal to W_s by V we can write this as $W - w W_s$ so $W - w W_s$ by V we can write as W by $V - W_s$ so if you write W_s by V as γ_D we can write γ_D as $\gamma_{bulk} - w \gamma_D$.

So if you rearrange these terms we can write γ_D is equal to $\gamma_{bulk} / (1 + w)$ so this particular you know relationship is used widely which is nothing but γ_D is equal to γ_{bulk} divided by $1 + w$ water content so here the unit weights of γ_D are in kilo Newton per meter cube γ_{bulk} in kilo Newton per meter cube in water content expressed in percentage but here is indicated in this form w in decimal form.

For example if the soil actually has got say 20 % water content here in this expression you need to use w is equal to 0.2 so typical values of unit weight of weight for soils are given in this particular slide.

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Typical values of Unit Weight for Soils

Soil type	γ_{sat} (kN/m ³)	γ_d (kN/m ³)
Gravel	20 - 22	15 - 17
Sand	18 - 20	13 - 16
Silt	18 - 20	14 - 18
Clay	16 - 22	14 - 21

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So if you see here typical values of unit weight for soils is given for a soil type gravel the saturated unit weight can be 20 to 22 the dry unit weight can be 15 to 17 sand can be 18 to 20 and the dry unit weight can be 13 to 16 silt it can be 18 to 20 and 14 to 18 in dry state for clay the saturated unit weight can be 16 to 22 and the dry unit weight can be 14 to 21 for some non soil like materials like say multiple solid waste they are even lighter than water someday the unit weight ranges from in the loose state for a freshman.

So solid waste it can be as low as 9 kilo Newton per meter cube for industrial waste like coal ash the dry unit weight can be in the range of 12 kilo Newton per meter cube that means that these light materials can be used in soil engineering or geotechnical hearing or some construction purposes if you wanted to do on construction on soft ground.

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Specific Gravity

Specific Gravity is the ratio of the unit weight of a substance to the unit weight of water γ_w at 4°C. In soil mechanics, specific gravity generally refers to the specific gravity of solid particles G_s , and is defined as the unit weight of solid particles to the unit weight of water.

$$G_s = \gamma_s / \gamma_w$$

$$= W_s / V_s \gamma_w$$

Unit weight of solid constituents $\gamma_s = W_s / V_s$

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The another parameter which is called specific gravity the specific gravity is defined as the ratio of the unit weight of the substance to the unit weight of water γ_w at 4 degree centigrade so in soil mechanics are Jordan engineering the specific gravity generally refers to the specific gravity of the solid particles so we use a specific gravity of the solid particles that is G_s suffix s and is defined as the unit weight of the solid particles to that of water that is G_s is indicated.

As γ_s by γ_w which is nothing but γ_s if you write it as W_s divided by capital V_s that is V_s nothing but volume of the solids into gamma W so G_s we can write it as W_s by V_s gamma W or weight of solids is equal to $G_s V_s \gamma_w$ that means that if you know the specific gravity of the solids in a given soil mass if you know the volume of solids and with the unit weight of water you can determine the weight of the solids.

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The value of the **specific gravity** can be determined from laboratory tests.


W_1

W_2

W_3

W_4

Where W_1 = Wt. of empty sp. gravity bottle
 W_2 = W_1 + dry soil
 W_3 = W_2 + water (without any entrapped air)
 W_4 = W_1 + water

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How to determine the specific gravity say in the laboratory if you see this simple illustration which is shown in this slide let us take you have got a particular material under consideration is filled in a specific gravity button and if you take weight of empty is the gravity bottle and if you indicate that weight as w_1 and if you take the desired soil in dry straight air-cooled and dry state and if you take that weight that is w_2 is equal to w_1 empty bottle + the dry soil and w_3 what you do is that w_2 + water without any entropy air.

So in order to do that you have to continuously shake and subjected to say some sort of boiling we're in the whites are entrapped air in the soil particles can be removed so if you determine that weight is indicated as w_3 is w_2 + water then once after doing that entire specific gravity a specific gravity bottle is filled with water and that way it is indicated as w_4 that is w_1 + water .

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Specific Gravity

$G_s = \text{Weight of soil solids} / \text{Weight of water volume equivalent to that of water}$
 $= (W_2 - W_1) / [(W_4 - W_1) - (W_3 - W_2)]$

For most of soils G_s ranges from **2.5 – 2.9**

$G_s = 2.65$ for sands
 $G_s = 2.2 - 2.4$ for Pond ash
 $G_s = 4.4 - 5.2$ for Iron ore

Mineral	G_s
Kaolinite	2.62-2.66
Illite	2.60 – 2.86
Montmorillonite	2.75-2.78

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Now so we have got w_1 w_2 w_3 w_4 Now we need to find out the specific gravity of the soil under consideration means so you need to take weight of the solids soil solids how do you get that is nothing but W_2 minus W_1 W_2 e is the weight of the soil - W_1 is the empty weight of the bottom then weight of the water volume equivalent to that of water so what in order to get this what you need to do is that you need to subtract weight of water - weight of water occupied that is $W_3 - W_2$.

So what we are doing is that we are actually taking the water volume equivalent to that of soil solids so in order to get that what you need to do is that $W_4 - W_1 - W_3 - W_2$ you need to do if you do that so you will get the expression $G_s = (W_2 - W_1) / [(W_4 - W_1) - (W_3 - W_2)]$ for most soils the specific gravity to the solids ranges from 2.5 to 2.9 here I have given for atypical minerals.

Which we are going to discuss in the next lecture KO light in light and multiple light these are the three predominant minerals in fine grained soils so if you see here for a kiln light base to the soil the specific gravity is around the range of two point six two point six for a light base in soil two point six two point eight six multiple right based soil the specific gravity can be in the range of two point seven five to two point seven eight so normally for sands.

The specific gravity of the solids G_s will be equivalent to two point six five and for say non soil like materials say pond ash has specific gravity in the range of two point two to two point four in some cases for very light coal ash or a pond ash it can be as low as two and for air on more

which is based on hammer tight type mineral where four point four to five point two it can have is a you know very high specific gravity.

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For a partially saturated soil:

Mass Specific Gravity $G_m = \gamma_b / \gamma_w$

G_m (dry) = γ_d / γ_w - for dry soil

G_m (saturated) = γ_{sat} / γ_w - for saturated soil

G_m (dry) = Mass Specific gravity (dry state)

G_m (sat.) = Mass Specific gravity (saturated state)

So for a partially saturated soil the we have discussed that which is γ bulk that is the γ bulk is nothing but the bulk unit weight are moist unit weight of the soil so for a partially saturated soil mass specific gravity is defined G suffix m as γB by γW in case of say $G M$ dry that is mass specific gravity in dry state which is nothing but a γD by γW for dry soil $G M$ saturated which is nothing but a γS at by γW for saturated soil.

So $G M$ dry is nothing but a mass specific gravity in dry straight which is nothing but a ratio of γD by γw $G M$ sat which is nothing but a mass specific gravity in a saturated state.

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Submerged (Buoyant unit weight) γ'

= Weight of soil inside the water / Total volume

= $[(W_s + W_w) - V\gamma_w] / V$ Treating whole soil mass as one unit

= $(W_s + W_w) / V - \gamma_w$

= $\gamma_{sat} - \gamma_w$

$\gamma' = \gamma_{sat} - \gamma_w$

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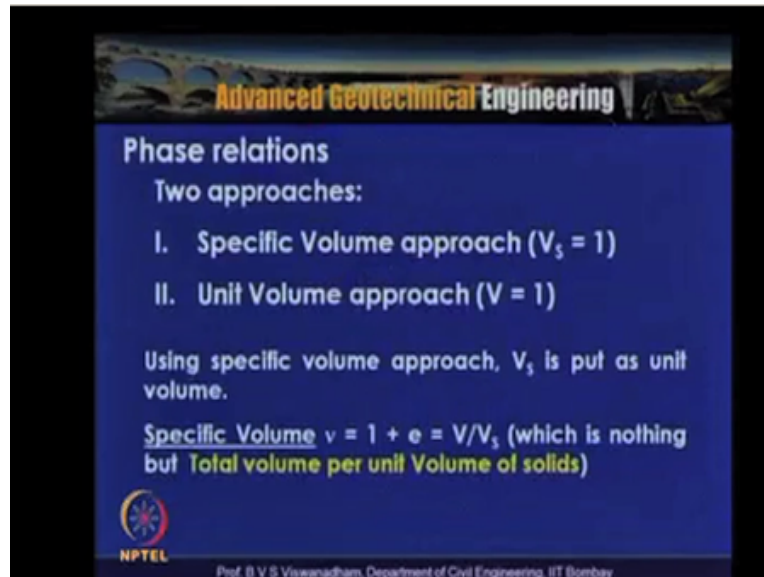
Then we knew that the soils when they are actually below the water table they are the Buoyant state so we define in order to calculate stresses at certain points below the water table and all you need to know the by on and unit weight or submerged unit weight of the soil so submerged unit weight or by another unit weight is indicated by γ sub or γ dash so γ dash is indicated nothing but submerged unit weight of the soil so how to you know get a relation between saturated unit weight of the soil and unit weight of water.

So further if you see the simple station here we have taken a ground surface this is the ground surface and this is the groundwater table and this is the soil mass which is completely saturated weight of water and weight of solids which is nothing but the weight of soil mass which is some much below the water table and volume of soil and volume of solids and volume of water on the left-hand side here so we can write the γ weight of soil inside the water divided by total volume so this can be written as very tough.

That is weight of solids + weight of water - $V \gamma_w$ so that much volume is displaced treating all soil mass as one unit we can write as $W_s + W_w - V \gamma_w$ by V so this can be written as $(W_s + W_w) / V - \gamma_w$ so which is nothing but γ_{sat} and we will get cancelled so $W_s + W_w$ by V is nothing but weight of saturated soil divided by volume which is nothing but a saturated unit weight of soil so we get relationship here γ sub is equal to $\gamma_{sat} - \gamma_w$.

So if you wanted to say compute the submerged unit weight of soil you need to take the unit weight of water subtract a unit of what from the saturated unit weight of the soil so γ dash is equal to γ sat $-\gamma$ W.

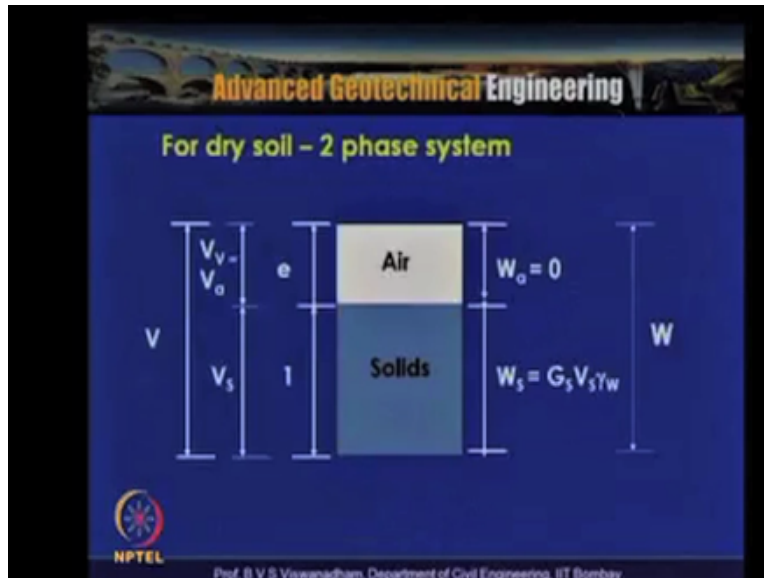
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So now if you wanted to determine the phase relationship phase relationships you can actually use different approaches like you can assume volume which is nothing but volume of wall voids volume of solids and volume of water volume of solids volume of water and volume of a in case of partially saturated soil and weight of water weight of solids and weight of air which is equal to zero so there are two approaches which are there one is called specific volume approach.

In this the volume of the solids is set as one volume of solids is set as actually one unit volume approach which is sometimes is also very convenient is set a assumed as $V = 1$ so this is assumed and these particular approaches are used if the volume is not known so using specific volume approach V_s is put as equivalent to unit volume so in the specific volume so you know here this is another which is actually used in which is nothing but total volume per unit volume of solids so $V = +E$ which is nothing but V_s, V by V_s that is nothing but V by V_s so total volume per unit weight of unit volume of solids is nothing but $V = V$ by V_s so here the specific volume is defined as $1 + e$.

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So for a dry soil in a two-phase system what you have here is weight of air is equal to zero so this is called two-phase system weight of solids is nothing but here we have set based on the unit volume approach here specific volume approach here V_s is equal to one we have set here and volume of why—it is equal to volume of air because the soil is dry so W_s is equal to $G_s v_s \gamma_w$ V_s as being one it is nothing but a $G_s \gamma_w$ and W_a is equal to 0. so this is taut weight of soil mass in case of a dry soil state two-phase system is nothing but W is equal to W_s .

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Soil is dry:

From the definition of void ratio $e = V_v/V_s$


$V_v = e$

$V_s = 1$

$G_s = \gamma_s/\gamma_w = W_s/V_s\gamma_w$

$W_s = G_s\gamma_w$

$\gamma_d = W_s/V = G_s\gamma_w/(1+e)$ **$\gamma_d = G_s\gamma_w/(1+e)$**


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So for the soil is dry from the definition of void ratio we can write V_v by V_s so V_v/V_s is nothing but e . V_s is nothing but 1 so $G_s = \gamma_s/\gamma_w$ based on that W_s by $V_s\gamma_w$ so we can write $W_s = G_s\gamma_w$ and γ_d are dry unit weight of the soil mass we can write as W_s/V which is nothing but $G_s\gamma_w/(1+e)$ so here for a dry soil we can write a relation like γ_d is equal to $G_s\gamma_w/(1+e)$ so this is very useful if you wanted to if you know the dry unit weight of the soil and the specific gravity of soil.

What will be the void ratio of the soil mass that means that e is equal to $G_s\gamma_w/\gamma_d - 1$ so if you rearrange the terms $\gamma_d = G_s\gamma_w/(1+e)$ we can use this relationship in case of basic relationship here for a saturated soil in this case weight of air is equal to nothing but $e\gamma_w$ because weight of air is 0 here weight of air is 0 and weight of water is $= e\gamma_w$ and weight of solids is equal to $G_s\gamma_w$ so total weight of the soil mass is equal to $G_s\gamma_w + e\gamma_w$ so from this if you look into the volume scale.

Total volume is nothing but $1+e$ so the saturated unit weight of soil mass γ_{sat} is nothing but W that is saturated unit weight of the entire soil mass divided by total volume so it is nothing but $G_s\gamma_w + e\gamma_w/(1+e)$ so like this by using these phase diagrams one can deduce the interrelationship interrelation between the soil properties .

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Soil is fully saturated:

From the definition of water content


$$W = W_w / W_s$$

$$= e \gamma_w / G_s \gamma_w$$

$e = W G_s \rightarrow$ For fully saturated case

$$\gamma_{sat} = W/V = (G_s \gamma_w + e \gamma_w) / (1 + e)$$

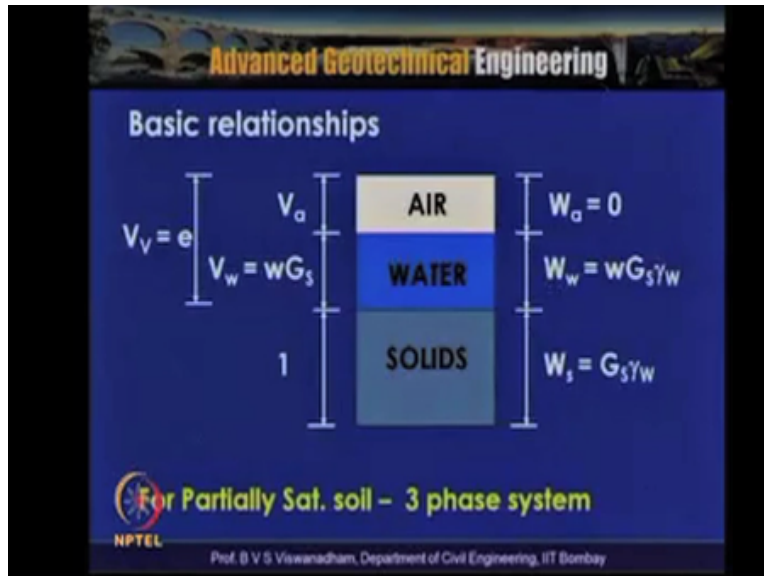
$$\gamma_{sat} = (G_s + e) \gamma_w / (1 + e)$$


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So for the soil is when the soil is fully saturated from the definition of water content we can write as W is equal to weight of water to weight of solids so weight of water in the previous slide we have seen as a γ_w weight of solids is nothing but $G_s \gamma_w$ because $V_s = 1$ we have set so with that if you look into this the relationship is that $E = W G_s$ so E the void ratio is nothing but water content times the specific gravity of the soil solids for a fully saturated case.

So for a fully saturated case γ_{sat} the saturated unit weight of the soil mass is nothing but W by V which is nothing but $G_s \gamma_w + e \gamma_w$ by $1 + e$ so the relationship with what we can write is that $\gamma_{sat} = G_s + e$ into γ_w divided by $1 + e$.

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Similarly now we have seen the dry state of the soil and saturated state of the soil but if you have got a partially saturated or a V we define this as a 3-phase system so in this case you have weight of water weight of solids and weight of a that is nothing but 0 so but here as a volume we have got volume of solids which we have set as 1 because of the specific volume approach and volume of voids which is nothing. But e but which is the summation of volume of air + volume of water so from the definition.

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From the definition of degree of saturation $S_r = V_w/V_v$

$$= wG_s/e$$

$e = wG_s/S_r$
 For $S_r = 1$, $e = wG_s$

$$\gamma_d = W_s/V$$

$$= G_s\gamma_w/(1+wG_s/S_r)$$

- Valid for a partially saturated soil

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Of degree of saturation of soil we can write S_r is = volume of water - volume of voids so volume of water which is nothing but here if you see weight of water is nothing but W G_s by $\gamma_w V$ V W_s is = $G_s \gamma_w$ from the water content definition we have written here weight of water is equal to WGS by γ_w so here by writing this what we get is that a relationship between for a partially saturated soil between void ratio water content specific gravity of the solids and degree of saturation.

Where E is equal to WGS by S_r for $S_r = 1 = WGS$ that we have deduced it previously for a saturated soil so here γ_d that is nothing w_s that is a dry state that is $G_s \gamma_w$ divided by V that is nothing but a volume which is nothing but $1 + e$ but E is nothing but $w G_s$ is R so for a partially saturated soil if you see the relationship is that $\gamma_d = G_s \gamma_w / (1 + w G_s / S_r)$ so this invalid for a partially saturated side.

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Relationship between γ_d , G_s , w and n_a

$$V = V_s + V_w + V_a$$

$$1 = V_s/V + V_w/V + n_a$$

$$1 - n_a = V_s/V + V_w/V$$

$$= \gamma_d/\gamma_w (w + 1/G_s)$$

$$\gamma_d = (1 - n_a) G_s \gamma_w / (1 + w G_s)$$

Using $V_s = W_s/G_s \gamma_w$
 $V_w = W_w/\gamma_w$
 & by writing $W_w = w W_s$

When soil becomes completely saturated $n_a = 0$

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Now let us try to derive some interrelationship between γ_d the dry unit weight of the soil specifically out of the solid G_s water content and n_a so we know that you know how to derive that let us say that we have total volume V is equal to $V_s + V_w + V_a$ okay so what we are doing is that by dividing both left-hand side and right-hand side by volume we can get 1 is equal to $V_s/V + V_w/V + n_a$ which is nothing but V_a/V we have written as n_a using V_s is equal to $W_s/G_s \gamma_w$ and V_w is equal to $w W_s/\gamma_w$ and by writing $W_w = w W_s$ we can get that n_a if you take it to the left hand side.

We get $1 - n_a = V_s/V + V_w/V$ by capital v and if you substitute these expressions here and simplify you will get $\gamma_d/\gamma_w (w + 1/G_s)$ so if you write this expression in terms of γ_d we can write $\gamma_d = (1 - n_a) G_s \gamma_w / (1 + w G_s)$ so for when the soil becomes completely saturated we know that $n_a = 0$ so in this case what will happen is that γ_d is $= G_s \gamma_w / (1 + w G_s)$ this is actually the expression what we derived for a saturated side.

So this is for a partially saturated soil if you have the relationship between γ_d , G_s , w , n_a can be given by $\gamma_d = (1 - n_a) G_s \gamma_w / (1 + w G_s)$ now having seen the specific volume approach let us see the unit volume approach also is sometimes very easy to deduce interrelationships or phase relationship phase relationships on the different phases of the soil for a partially saturated soil let us consider it and we have we say that it is a 3phase system and here.

What we did is that total volume is set as 1 so if you set the total volume as 1 then the portion which is the volume of voids is nothing but n because porosity is nothing but volume of voids

divided by total volume so V_v by V is equal to n nothing but this becomes small n that is porosity and this portion becomes this is this becomes $1 - n$ so the weight of the solids is given as G_s into $1 - n$ into γ_w G_s into $1 - n$ into γ_w based on.

The water content definition we can write W into G_s $1 - n$ γ_w by taking γ_w out we can write here the volume of water as w G_s into $1 - n$ the rest is volume of the a and $W a = 0$ here so for a partially saturated soil and if you want say the total weight of the soil mass which is nothing but G_s into $1 - n$ γ_w + $w G_s$ into $1 - n$ γ_w by total volume .

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For Partially Sat. soil – 3 phase system

$n = V_v/V$

With $V = 1$ $n = V_v$

$e = V_v/V_s = n / (1 - n)$

$\gamma_d = W_s/V = (1-n)G_s \gamma_w$

$\gamma_{bulk} = W/V = (1-n)G_s \gamma_w + wG_s (1-n)\gamma_w$

Percentage air voids $n_a = V_a/V$

$= (V_v - V_w)/V = [n - wG_s(1-n)]$

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So for a partially saturated soil three phase system we have $n = V_v/V$ by V with $v = 1$ n is equal to volume of whites so we can write E is equal to V_v/V_s that is nothing but n by 1 sm we have deduced earlier so we can write $\gamma_d = W_s/V$ that is nothing but $1 - n$ into $G_s \gamma_w$ that is we are expressing in terms of say porosity or that is $1 - n$ into $G_s \gamma_w$ γ_{bulk} from the previous slide we can say that W/V which is nothing but $1 - n$ into $G_s \gamma_w + w G_s$ into $1 - n$ γ_w .

Now percentage air voids n_a can be defined as V_a/V so which is nothing but $V_v - V_w$ by V which is which can be very simply it can be written as $n - w G_s$ into $1 - n$ because total volume = 1 for a completely saturated soil similarly we can actually set and use this approach.

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For a completely Saturated soil

$$\gamma_{sat} = G_s(1-n)\gamma_w + n\gamma_w$$

$$w = n\gamma_w / [G_s(1-n)\gamma_w]$$

$$= e/G_s$$

$$e = wG_s$$

(for $S_r = 1$)

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And we can write γ_{sat} is equal to $G_s(1-n)\gamma_w + n\gamma_w$ because for in case of completely $n\gamma_w$ saturated soil you have got $n\gamma_w$ that is the weight of water and weight of solids is nothing but $G_s(1-n)\gamma_w$ so here the total volume set is equal to set as 1 and n is the volume of water so water W is equal to we can write it as $n\gamma_w$ divided by $G_s(1-n)\gamma_w$ which is nothing but e de by G_s . So y equal to WGS is what actually we have got for a you know completely saturated soil.

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For a dry soil

$$\gamma_d = W_s/V =$$

$$= (1-n)G_s\gamma_w$$

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For a dry soil similarly so we have seen for partially saturated soil and the same for completely saturated soil and for a dry soil with the same approach we can write γ_D is equal to $W S$ by V which is nothing but $1 - n$ into G 's γW divided by total volume is equal to 1 because of this we can write this as $1 - n$ into $G S \gamma W$.

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Some additional phase properties and relations

- Porosity can be defined with respect to each of the phase of a soil
- Soil particle porosity $n_s = V_s/V$ (expressed in %)
- Water porosity $n_w = V_w/V \rightarrow$ (also referred as volumetric water content (w))
- Air porosity $n_a = V_a/V$

The water and air porosities represent their volumetric percentages in the soil: The soil particle porosity can be visualized as the percentage of total volume comprised of soil particles.

$n_s + n_w + n_a = 100\%$

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And let us also discuss some additional phase properties and relations particularly. Which are used in unsaturated soil mechanics and the porosity can be defined with respect to each of the phase of this side so we have defined normal the porosity as volume of voids in a total volume of soil but we have got different phases of soil particularly you have got a solid phase that is solids are there and water is there a or gas is there.

So soil particle porosity n_s is defined as volume of solids in total volume that is V_s by V this is expressed in percentage and water porosity which is nothing but n_w which is volume of water in total volume so this is also referred as volumetric water content θ_w so this is particularly used in soil science or in nowadays it is widely used in unsaturated soil mechanics air porosity and suffix $n_a = V_a$ by capital V so air porosity $n_a = V_a$ by capital v and water porosity $n_w = V_w$ divided by V .

And this is defined this is also referred as volumetric water content θ_w now the water and air porosity is represent their volumetric percentages in the soil so water and air porosity they represent their volumetric percentages in the soil that is volume of water in the total volume of a in total volume the soil particle velocity can be visualized as a percentage of the total volume compression of soil particles so the soil particle porosity is can be visualized as the percentage of total volume compression of soil particles.

So the soil particle porosity can be visualized as a percentage of total volume compression of soil particles but if you look into this n_s is nothing but volume of soil solids - volume of total volume + n is nothing but volume of whites in a total volume so the summation of $n_s + n = n_s + n_a + n_w$ should be equal to 200percent that means that here in is water the air porosity n_w that is nothing but water porosity n_w is nothing but a soil particle porosity the summation of $n_s + n = n_s + n_a + n_w$ has to be current to 100% some additional phase properties and relationships. If you derive volumetric water content or you know we have defined in the previous line as θ .

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Some additional phase properties and relations

Volumetric water content $\theta_w = V_w/V$

$\theta_w = S_r V_v / V = n S_r = e S_r / (1+e)$ Using $S_r = V_w / V_v$;
 $n = V_v / V$; $n = e / (1+e)$

→ The relationship between the gravimetric water contents, w and θ_w can be established by substituting the basic weight-volume relationship:

For partially saturated soils:

$\theta_w = w G_s S_r / (S_r + w G_s)$ Using $e = w G_s / S_r$

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As volume of water - capital V so using degree of saturation as the V W by V, V and n = V, V by V we can write and using any is equal n = e by 1 + e if you substitute here for a degree of saturation we can write it in terms of theta W in terms of SR V V by V how we have written is nothing but V W = SR into V V we have written divided by capital V, V, V by V is nothing but porosity so we can write the volumetric water content is n times porosity times degree of saturation so here by using n is equal to e + 1 e.

We can write e sr divided by 1 + e so if you wanted to get say the relationship between the gravimetric water content which is what we have defined conventionally the water content and θ_w the volumetric water content we can actually obtain for a partially saturated soil as by using E is equal to substituting G is equal to WGS by SR if you substitute here then what we get is that θ_w is equal to WGS SR by SR + w GS.

So if you have this one you will get a relationship between volumetric water content θ_w and the gravimetric water content W so $\theta_w = WGS sr + / sr + w GS$.

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Example problem 1

A 0.9 m³ soil specimen weighs 17 kN and has a moisture content of 9%. The specific gravity of the soil solids is 2.7. Using the phase relations calculate:

(i) γ , (ii) γ_d , (iii) e ,
 (iv) n , (v) V_w , and (vi) S_r

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So having seen the phase relationships among the soil properties let us take some example problems and try to solve them so in this example problem in this slide it is given as a point eight meter cube soil specimen weighs 17 kilo Newton and has a moisture content of 9% the specificity of the soil solids is 2.7 using the phase relationships so we need to calculate γ γ_d e porosity volume of water and degree of saturation.

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Solution to example problem 1

Given: $V = 0.9 \text{ m}^3$, $W = 17 \text{ kN}$, $w = 9\%$, and $G_s = 2.7$

From the definition of unit weight, $\gamma = W/V$

$$\gamma = 17/0.9 = 18.89 \text{ kN/m}^3$$

Using $\gamma_d = \gamma/(1+w) = 18.89/(1+0.09)$

$$= 17.33 \text{ kN/m}^3$$

$$W_s = \gamma_d V = 17.33 \times 0.9 = 15.59 \text{ kN}$$

$$W_w = W - W_s = 17 - 15.59 = 1.41 \text{ kN}$$

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So the solution runs like this what is given is that volume is given point 9 U and weight is given as 17 kilo Newton and water content is given as 9percent and specific gravity of the solids is

given as 2.7 so from the definition of unit weight we can write θ is equal to W/V so the θ the bulk weight it can be defined as 1770 divided by 0.9 we can write it as 18 point 89 can provide Q using θ Dis equal to γ by $1 + W$ which is nothing but γ is nothing but γ bulk divided by $1 + W$ we can calculate what is the dry unit weight of the soil.

So from this if you look into this w_s is equal to γDV which is nothing but 17 point 33 into 0.9 which is nothing but 15 point 59 kilo Newton sand W , W is nothing but $W - W_s$ nothing but weight of water in the soil mass is nothing but 1 point 41 kilo Newton's.

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Using $G_s = \gamma_s/\gamma_w = (W_s/V_s)/\gamma_w$

$V_s = W_s/(G_s \gamma_w) = 15.6 / (2.7 \times 9.81) = 0.5889 \text{ m}^3$

$V = V_v + V_s \rightarrow V_v = 0.9 - 0.5889 = 0.311 \text{ m}^3$

Void ratio $e = V_v/V_s = 0.311/0.5889 = 0.528$

Porosity $n = V_v/V = 0.311/0.9 = 0.346$
(expressed as $n = 34.6\%$)

Volume of water $V_w = W_w/\gamma_w = 1.4/9.81 = 0.143 \text{ m}^3$

Degree of saturation $S_r = V_w/V_v = 0.143/0.311 = 0.459$
(expressed as $S_r = 45.9\%$)

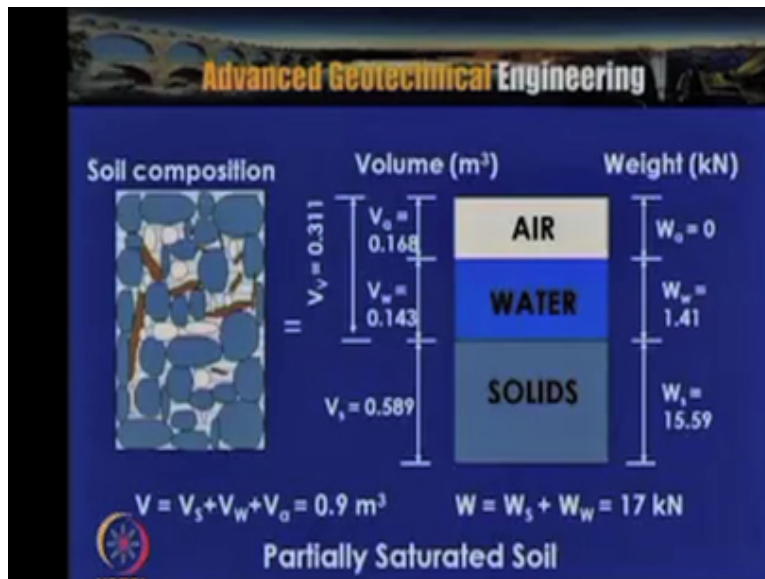
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Then using specific gravity is equal to γ_s by γW W_s by V_s γW we can determine volume of solids as W_s by $G_s \gamma W$ which is nothing but a point 5889 meter cube total volume is equal to $V_s + V_v$ and which is nothing but V V is obtained as point 311 meter cube so wide ratio is nothing but the volume of voids to volume of solids so in this case the soil mass has void ratio of 0.528 porosity is nothing but volume of voids are due to the total volume and total volume of the soil mass is given as point 9 so the porosity which is obtained as point 346 it has 34.6%

and the volume of water is nothing but weight of water divided by γ_w and γ_w is equal to taken has taken as nine point eight one kilo n per meter cube.

So with that volume of water has 0.143 meter cube so the degree of saturation is given as volume of water to tell total volume of whites so based on this the problem for a given problem the degree of saturation is obtained as about forty five point nine percent and if you express.

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This in what we calculated in the soil composition for a partially saturated soil total volume is 0.9 if you are portion this volume of solids is 0.5 eight nine meter cube and the volume of water is 0.14three volume of air is 1 0.168 so total put two there you will get 0.9 meter cube volume of voids is nothing but volume of water plus volume of a which is nothing but point three one meter cube and weight of water.

Which is one point four one and weight of solids is fifteen point five nine total weight of soil mass is $W_s + W_w = 17$ kilo meter cube and V is equal to $v_s + BW v_w + V_a$ which is equal to 0.9 meter cube so by using these phase diagrams one can actually deduce the properties of soil and interrelationship between weight and volume ratios so in this lecture what we understood is that

some phase relationships among soil properties and we have defined number of soil properties particularly like volumetric ratios and water content.

And we also discussed about the relationship between gravimetric water content and θ_w the volumetric water content θ_w so in the next lecture we are going to discuss about soil particle sizes and their arrangement and particularly soil particle sizes and arrangement their particle arrangement and then we will discuss about mineral ge clay ontology and then that leads to the discussion about the type of clay in terms present in the soil you.

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